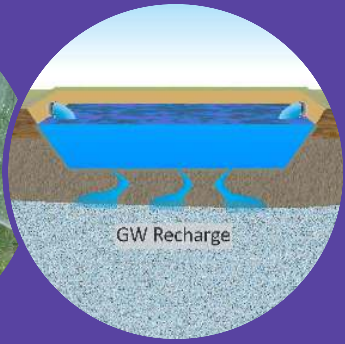
FINAL – September 2016

CASTAIC LAKE WATER AGENCY

Recycled Water Master Plan



NCWD

SCWD

VWC
Valencia Water Company

LACWD

SANITATION DISTRICTS OF LOS ANGELES COUNTY

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FINAL
Castaic Lake Water Agency Recycled Water Master Plan

September 2016

Prepared for

Castaic Lake Water Agency
27234 Bouquet Canyon Road
Santa Clarita, CA 91350

K/J Project No. 1544241.00

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List of Acronyms and Abbreviations

AF	acre-feet
AFY	acre-feet per year
AOP	Advanced Oxidation
AWWA	American Water Works Association
AWTF	Advanced Water Treatment Facility
BAC	Biologically activated carbon
CCR	California Code of Regulations
CDFW	California Department of Fish and Wildlife
CEQA	California Environmental Quality Act
CIP	Capital Improvement Program
CLWA	Castaic Lake Water Agency
CWA	Clean Water Act
COP	Certificates of Participation
CWSRF	Clean Water State Revolving Fund Loan
DDW	Division of Drinking Water
dia	Diameter
DOSH	California Division of Occupational Safety and Health
DPR	Direct Potable Reuse
DWR	California Department of Water Resources
EBC	Enhanced brine concentration
EIR	Environmental Impact Report
FAT	Full Advanced Treatment
Fps	Feet Per Second
GIS	Geographic Information System
GO	General Order
gpcd	gallons per capita daily
gpm	gallons per minute
GRR	Groundwater Replenishment Reuse
HDPE	High-Density Polyethylene
HGL	Hydraulic Grade Line
I&C	Instrumentation and Controls
IPR	Indirect Potable Reuse

IRWMP	Integrated Regional Water Management Plan
IX	Ion Exchange
LACDHS	Los Angeles County Department of Health Services
LACDRP	Los Angeles County Department of Regional Planning
LACFCD	Los Angeles County Flood Control District
LACSD	Sanitation Districts of Los Angeles County
LACWD	Los Angeles County Waterworks District
LF	Lineal Feet
M&I	Municipal and Industrial
MCL	Maximum Contaminant Level
MDD	maximum day demand
MF	Microfiltration
MG	million gallons
mgd	million gallons per day
mg/L	milligrams per liter
MWD	Metropolitan Water District of Southern California
NCWD	Newhall County Water District
NEPA	National Environmental Policy Act
NF	Nanofiltration
NMFS	National Marine Fisheries Service
NPDES	National Pollutant Discharge Elimination System
NPR	Non-Potable Reuse
NWP	Nationwide Permits
O&M	Operations and Maintenance
PEIR	Programmatic Environmental Impact Report
psi	pounds per square inch
PVC	Polyvinyl Chloride
RDX	Hexahydro-1,3,5-Trinitro-1,3,5-Triazine
RO	Reverse Osmosis
RW	Recycled Water
RWC	Recycled Wastewater Contribution
RWMP	Recycled Water Master Plan
RWQCB	Regional Water Quality Control Board

SAR	Sodium Absorption Ratio
SAT	Soil aquifer treatment
SCADA	Supervisory Control And Data Acquisition
SCWD	Santa Clarita Water Division
SCVSD	Santa Clarita Valley Sanitation District of Los Angeles County
SCWD	Santa Clarita Water Division
SWGP	Stormwater Grant Program
SNMP	Salt and Nutrient Management Plan
SWA	Surface Water Augmentation
SWP	State Water Project
SWRCB	State Water Resources Control Board
TDH	Total Dynamic Head
TDS	Total Dissolved Solids
TOC	Total Organic Carbon
USACE	U.S. Army Corps of Engineers
USBR	U.S. Bureau of Reclamation
USEPA	U.S. Environmental Protection Agency
USFWS	U.S. Fish and Wildlife Service
USGS	U.S. Geological Survey
UV	Ultraviolet
UWMP	Urban Water Management Plan
V/G/C	Virus, Giardia, or Cryptosporidium
VWC	Valencia Water Company
WRP	Water Reclamation Plant
WDD	winter day demand
WDR	Waste Discharge Requirements
WRRF	WateReuse Research Foundation
WRRDA	Water Resources Reform and Development Act

Executive Summary

ES.1 Introduction and Background

The Castaic Lake Water Agency (CLWA) is the public water wholesaler in Santa Clarita Valley; delivering imported State Water Project (SWP) water to four retail water purveyors: Los Angeles County Waterworks District 36 (LACWD36), Newhall County Water District (NCWD), Santa Clarita Water Division of CLWA (SCWD), and Valencia Water Company (VWC). Collectively, CLWA and the retail purveyors are the Santa Clarita Valley's 'water suppliers'. CLWA's water supply portfolio includes local groundwater, recycled water, imported supplies, and water from existing groundwater banking programs. Even with this diverse portfolio of water supplies, the extreme prolonged drought conditions have led CLWA and the retail purveyors to seek opportunities to expand their existing recycled water system to offset potable water demands and improve water supply reliability.

This Recycled Water Master Plan (RWMP) updates the 2002 Recycled Water Master Plan based on recent developments affecting recycled water sources, supply availability and demand, and explores opportunities to maximize the utilization of recycled water in the Santa Clarita Valley. This RWMP evaluates near-term, mid-term and long-term objectives as described in Figure ES-1.

Figure ES-1: Study Objectives

Near-Term Objective:

- Incorporate updates for Phase 2 Recycled Water System expansion.
- Support upcoming design work.
- Initiate exploration of groundwater recharge with recycled water.
- Assist in pursuit of currently available grants and loans.

Mid-Term Objective:

- Optimize expansion of the non-potable recycled water system.
- Further investigate or implement next steps for potable reuse.

Long-Term Objective:

- Continue exploration and/or implementation of potable reuse through surface water augmentation and/or direct potable reuse.

ES.2 Recycled Supplies

The Santa Clarita Valley Sanitation District of Los Angeles County (SCVSD) operates two water reclamation plants (WRPs) within the CLWA service area: 1) Saugus WRP and 2) Valencia WRP. The primary sources of wastewater to the Saugus and Valencia WRPs are residential and commercial flows. The two plants produce high-quality tertiary disinfected recycled water, which is distributed for non-potable reuse or discharged into the upper reaches of the Santa Clara River. The Valencia WRP has an average daily discharge of 13.8 million gallons per day (mgd), a current treatment capacity of 21.6 mgd, with plans to expand to 27.5 mgd over time. The Saugus WRP has an average daily flow of 5.5 mgd, a current maximum treatment capacity of 6.5 mgd, with no future expansions planned due to space limitations.

Some of the planned future developments in the Santa Clarita Valley, such as the Westside Communities and Vista Canyon developments, intend to construct water reclamation facilities to produce tertiary recycled water suitable for non-potable reuse to offset potable demands. Excess recycled water from these sources may be incorporated into the CLWA recycled water system or served directly to Santa Clarita Valley customers in the future. The Vista Canyon Water Factory is anticipated to come online in 2017 and treat flows from the planned Vista Canyon Development and produce 392,000 gallons per day (gpd) or 440 acre-feet per year (AFY) of tertiary disinfected recycled water for use within the development, with excess supply available for nearby existing SCWD customers. The proposed Newhall Ranch WRP is anticipated to produce 3.75 mgd (4,200 AFY) of recycled water based on anticipated flows from the Westside Communities development at buildout. The recycled water produced at the Newhall Ranch WRP would be available to meet a portion of the 7,100 AFY of the non-potable demands anticipated for the Westside Communities development.

A portion of the flows from the Valencia and Saugus WRPs are discharged to the Santa Clara River to meet instream flow requirements to protect biological resources in the river. SCVSD has prepared technical analyses that show that at least 13 mgd should be discharged from the Saugus and Valencia WRPs to the Santa Clara River in order to sustain biological resources (LACSD 2013). Recent trial court decisions have indicated that SCVSD's technical analyses regarding the discharge level of 13 mgd requires additional detail. Such studies may result in higher or lower quantities of water being required for discharge. However, based on discussions with the SCVSD, for the purpose of this study, it is assumed that 8.5 mgd of discharge must be maintained at the Valencia WRP outfall and 4.5 mgd at the Saugus WRP outfall. The amount of effluent available for recycled water use is based on the available supply less the required discharge.

Future production of recycled water is estimated based on the projected influent wastewater flow to the water reclamation facilities using SCVSD's planning level generation rate of 65 gallons per capita daily (gpcd) multiplied by the population within the CLWA service area projected overtime. Table ES-1 summarizes the existing and total projected supply of recycled water in the Santa Clarita Valley that could be available for reuse from the existing and proposed WRPs.

Table ES-1: Summary of Existing and Future Recycled Water Supplies

Existing Water Supply	Ave Annual (MGD)	Ave Annual (AFY)
Current Supply of RW from LACSD [1]		
Valencia WRP	13.8	15,500
Saugus WRP	5.5	6,100
Discharge to Minimize Environmental Impact [2]		
Valencia	8.5	9,520
Saugus	4.5	5,040
Current Available RW	**6.3**	**7,040**

Future Water Supply (2050)	Ave Annual (MGD)	Ave Annual (AFY)
Projected Supply of RW from LACSD [3]		
Valencia WRP[3]	18.7	20,900
Saugus WRP[4]	5.5	6,100
Proposed WRPs [5]		
Vista Canyon Water Factory	0.4	440
Newhall Ranch WRP	3.7	4,200
Discharge to Minimize Environmental Impact[2]		
Valencia	8.5	9,500
Saugus	4.5	5,000
Future Available RW	**15.3**	**17,140**

1. Based on historical data from LACSD for 2014
2. Per Email from Bryan Langpap with Sanitation Districts of LA County, dated 10/27/2015
3. Projected Valencia WRP based on a generation rate of 65 gpcd multiplied by the net projected population increase.
4. Assumes no increase in Saugus Production
5. Planned Schedule - VCWF Production by 2017 and Newhall Ranch WRP Production by 2023 (Cris Perez, 11/12/2015)

Based on historical effluent flows at the Saugus WRP for the last 10 years (4.2 mgd to 5.5 mgd), up to1 mgd of recycled water could be available beyond the minimum discharge requirement; however, in many years no recycled water would be available. Constructing new infrastructure for this small amount of available recycled water supply is more costly, less reliable and more operationally complex than using recycled water from the Valencia WRP. Thus the RWMP assumes that recycled water supplies from the Saugus WRP would not be utilized for expanding the recycled water system.

Flows from the Vista Canyon Water Factory would serve future customers in the planned Vista Canyon Development and nearby existing SCWD customers. Flows from the Newhall Ranch WRP would serve future customers in the planned Westside Communities Development.

ES.3 Recycled Water Market

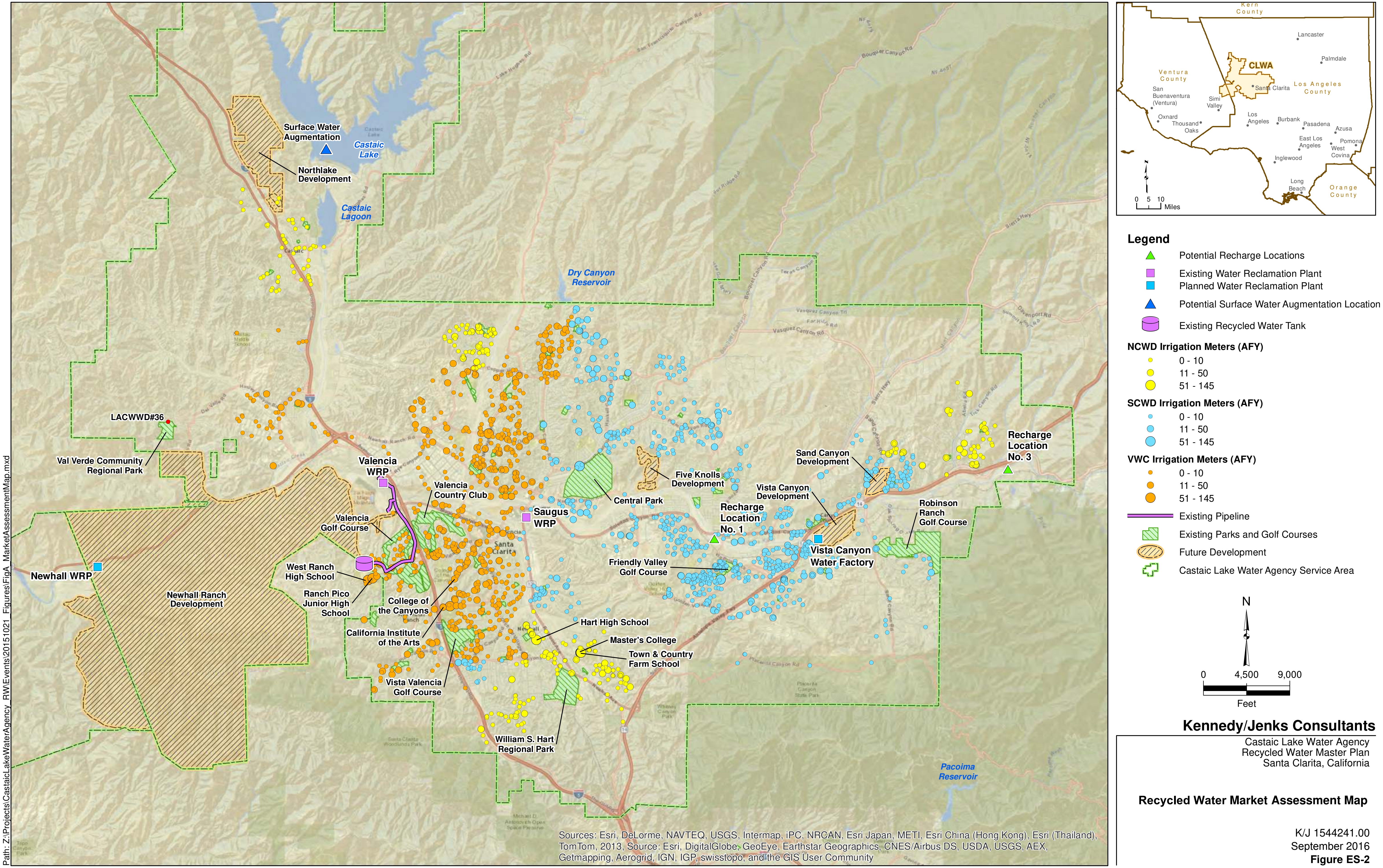
The production, discharge, distribution, and use of recycled water are subject to federal, state, and local regulations; the primary objectives of which are to protect public health. Table ES-2 summarizes the types of reuse that are explored as part of the recycled water market assessment.

Table ES-2: Summary of Types of Reuse

Type of Use	Description	Examples
Non-potable reuse	Typically tertiary treated municipal wastewater for specific purposes other than drinking	landscape irrigation, industrial uses, and agriculture or for environmental benefits
Potable reuse	Highly treated or purified municipal wastewater to augment a water supply that is used for drinking and all other purposes	Indirect Potable Reuse (IPR) and Direct Potable Reuse (DPR)
Indirect potable reuse (IPR)	The purposeful introduction of tertiary treated recycled water or highly purified recycled water into an untreated drinking water supply source.	Groundwater Replenishment Reuse (GRR) and Surface Water Augmentation (SWA)
Direct potable reuse (DPR)	The purposeful introduction of highly purified recycled water into a drinking water supply	Source water blending upstream of a drinking water treatment plant or directly into the potable water supply distribution system downstream of a water treatment plant.

Non-Potable Reuse Market Survey

Existing non-potable recycled water demands for the Santa Clarita Valley were estimated using 2013 irrigation meter data provided by CLWA and the purveyors. Figure ES-2 illustrates the location of existing irrigation meters, by purveyor, and relative demand (as indicated in the legend). The potential non-potable recycled water demands for planned future developments were estimated based on information provided by planning documents and discussions with the purveyors. Figure ES-3 illustrates the projected future (2050) monthly distribution of the total projected available recycled water supply in Santa Clarita Valley and potential demand for recycled water.



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Recycled Water Market Assessment Map

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Figure ES-2

Figure ES-3 Recycled Water Supply and Potential Demand in Santa Clarita Valley (2050)

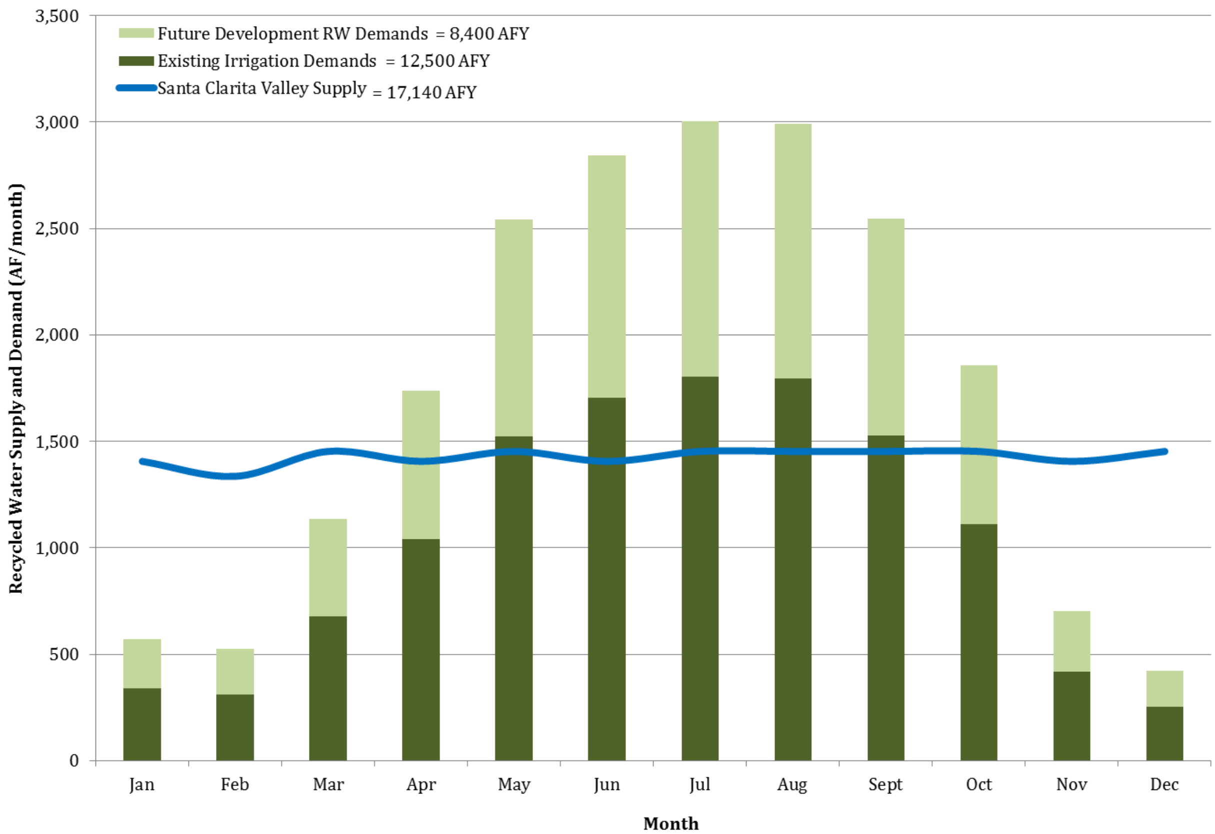

Figure ES-3 clearly shows that the Santa Clarita Valley is supply limited, both annually and during the peak irrigation months. In addition, the geographic distribution of the dedicated irrigation meters (shown in Figure ES-2) would make it cost prohibitive to serve many of these potential customers due to the significant amount of conveyance infrastructure that would be required. Thus the identification of potential customers, the appropriate source and quality of recycled water, necessary infrastructure and a proposed phasing plan to align supply and demand over time is evaluated as part of the Project Alternatives Analysis.

Potable Reuse Market Survey

The potable reuse concepts investigated include groundwater recharge, surface water augmentation and direct potable reuse. A market survey for potable reuse is not associated with meters; but rather a more holistic approach to assess opportunities to beneficially reuse the recycled water for potable uses indirectly or directly. Some of the potential benefits and challenges associated with potable reuse in Santa Clarita Valley are summarized in Table ES-3.

Table ES-3: Summary of Benefits and Challenges of Potable Reuse in Santa Clarita Valley

Potential Benefits of Potable Reuse:	Potential Challenges of Potable Reuse:
• Maximize use of local, drought-proof and sustainable water supply • Reduce reliance on imported water • Use of available recycled water in winter and off-peak irrigation months • Provide redundancy by keeping aquifers full and creating an available emergency supply in case of SWP interruption • Reduce discharges to the Santa Clara River (after meeting instream flow requirements) • Repurpose unused capacity in the SCVSD AWTF designed to remove chloride • Recharge groundwater basin(s) via surface spreading or injection to increase groundwater levels • Maintain lake levels (via surface water augmentation) • Provide an integrated approach solving multiple issues and bring together stakeholders in Santa Clarita Valley.	• High costs associated with advanced treatment and brine disposal • High costs associated with pumping and conveyance (for GRR and SWA projects) • Additional regulatory requirements • Public acceptance • Development of partnerships and agreements • Regulatory uncertainty related to SWA and DPR requirements

The following potable reuse opportunities were explored in the RWMP at a conceptual-level. More detailed studies would be required to confirm and refine the assumptions and approaches described herein:

Groundwater Replenishment via Surface Spreading: recycled water is discharged into spreading basins, where it percolates through the vadose (unsaturated) zone until it joins native groundwater and travels horizontally (saturated zone) towards extraction wells. Two recharge locations (Figure ES-2) were studied (1) Recharge Location #1 – an off-stream spreading site and (2) Recharge Location #3 – which includes in-stream (#3a) and off-stream (#3b) spreading options. For both sites it is assumed that groundwater recharge would be limited by the seasonal availability of recycled water (after all irrigation is served) and stormwater capture would be prioritized over recycled water (i.e. during heavier months of rainfall, spreading with recycled water would be limited in favor of stormwater). Native groundwater underflow would be relied upon as the dilution water to meet regulatory requirements and initial groundwater recharge volumes would be limited by the regulatory requirement to achieve a 20 percent recycled water contribution (RWC).

Groundwater Replenishment via Direct Injection: involves recycled water that has gone through a full advanced treatment (FAT) process and is directly injected into the saturated groundwater zone, bypassing soil aquifer treatment (SAT). To minimize additional costs, this concept assumes that the injection wells could be located in the vicinity of the Valencia WRP along with the advanced water treatment facility (AWTF), which would require brine disposal via truck hauling. Direct injection would not require dilution water to meet regulatory requirements and would also not be hindered by inclement weather as water can be injected into the ground regardless of the weather conditions. As such, all of the available recycled water could be utilized by a direct injection project.

Surface Water Augmentation (SWA): consists of conveying recycled water that has gone through FAT to augment Castaic Lake, shown in Figure ES-2, which is designated as a source of domestic water supply. Similar to direct injection, the SWA concept would require an AWTF to treat all of the available recycled water from the SCVSD and brine disposal would require truck hauling. The most recent draft SWA regulations require achieving a dilution requirement in the reservoir of 100:1 (or 10:1 with an additional treatment) and a retention time of at least six months (calculated as total volume divided by total outflow). The size of the Castaic Lake and the anticipated project flow is such that at least 10:1 dilution can likely be achieved in the reservoir and the drinking water treatment that is located on the downstream side of the reservoir would be used to meet the required incremental 1-log microbial pathogen treatment. However, because of the large withdrawals of reservoir water by Metropolitan Water District (MWD) from the State Water Project (SWP) water stored in Castaic Lake, the retention time in the reservoir is approximately two months and would not enable this project to qualify as a SWA project based on the criteria in the draft SWA regulation. Despite the regulatory uncertainty, a SWA is included in the RWMP alternatives.

Direct Potable Reuse (DPR): A DPR concept could potentially utilize all recycled water not already allocated for non-potable reuse, and would require FAT of the recycled water from SCVSD, brine disposal via truck hauling and only minimal conveyance requirements. The DPR alternative would treat 100 percent of the available recycled water from the SCVSD at an AWTF and the purified water would be blended with the raw water entering the existing Rio Vista Water Treatment Plant (WTP) for further treatment prior to distribution.

ES.4 Project Alternatives

Four alternatives are developed as part of the alternatives evaluation to address near-term, mid-term and long-term objectives,. Each alternative consists of a group of projects; some of which can be constructed independent of other projects, while others would build on previous phases and require upsizing of facilities to meet increased future flows. The non-potable reuse project alternatives are limited by the supply of recycled water in the summer months and can be implemented to meet near-term and mid-term objectives. The potable reuse project alternatives are limited by available flows from the Valencia WRP not destined for irrigation or discharge and present opportunities to meet mid-term and long-term objectives.

Alternative 1 - Non-Potable Reuse Expansion (Phase 2): looks at near-term opportunities to expand recycled water use for non-potable uses, with a focus on meeting irrigation demands (though commercial demands may also be served). Four projects planned to expand recycled water use within Santa Clarita Valley, which are collectively known as Phase 2, are depicted in Figure ES-4, and are currently in various stages of design. Phase 2A, 2C and 2D would use recycled water from the Valencia WRP and Phase 2B would use recycled water produced at the Vista Canyon Water Factory, which is being constructed to treat flows from the planned Vista Canyon Development.

Alternative 2 - Non-Potable Reuse Expansion (Future Phases): assesses mid-term opportunities to expand recycled water use for non-potable uses. This alternative considers future alignment extensions beyond Phase 2 for existing landscape irrigation, as well as service to the planned new development for the Westside Communities. An overview of potential conveyance facilities in the north (extending off Phase 2A) and in the south (extending off Phase 2C) are shown in Figure ES-4. Facilities associated with the planned Westside Communities would be designed by the developer and are not shown. The northern and southern extensions would utilize available recycled water from the Valencia WRP while the Westside Communities development would use recycled water from the planned Newhall Ranch WRP supplemented by Valencia WRP flows.

Alternative 3 - Groundwater Recharge (Surface Spreading): explores mid-term opportunities to expand recycled water use for non-potable uses while implementing a groundwater recharge project via surface spreading. Alternative 3 includes five projects that use recycled water to recharge groundwater via surface spreading at two locations, as shown in Figure ES-5. Each project would extend off the Phase 2A system, and require upsizing the pipeline capacity of most of the Phase 2A pipeline to maximize deliveries of recycled water for non-potable use and to one or more spreading basin(s). Utilizing recycled water from the Valencia WRP for surface spreading would require blending tertiary recycled water with the product water from the planned AWTF, currently being designed by the SCVSD as part of their Chloride Compliance Project, to provide additional treatment to meet the water quality objectives in the basin. Based on discussions with SCVSD, for the purpose of this evaluation, it is assumed that up to 5,000 AFY of Valencia Blend water (a 70/30 blend of tertiary effluent to RO permeate from the AWTF) may potentially be available to CLWA (at a higher cost than the tertiary recycled water) for surface spreading. The recharge limitation for both spreading locations is the amount of available recycled water after meeting discharge requirements and prioritizing stormwater recharge during rainy periods.

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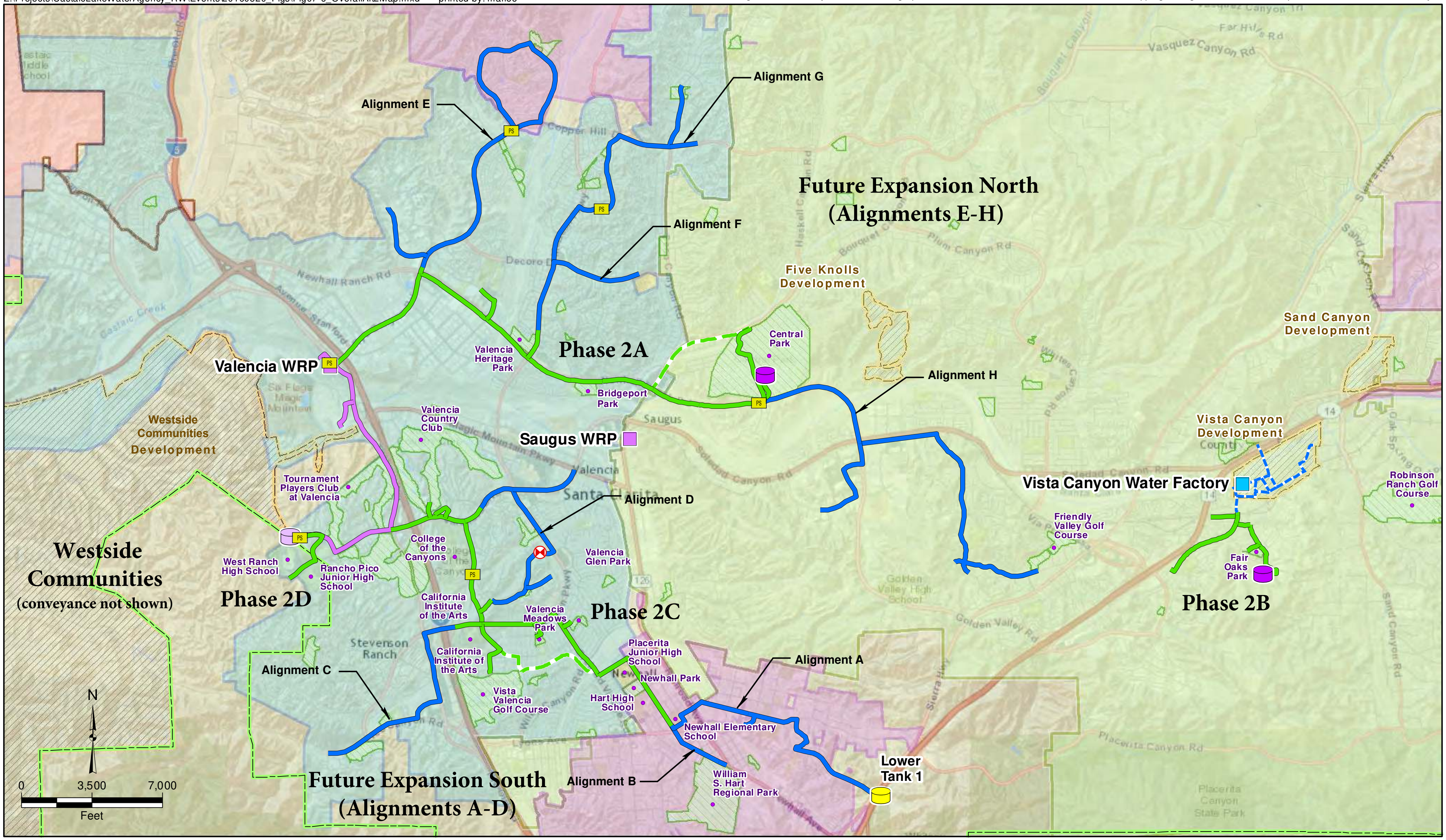

Legend

- Existing Water Reclamation Plant
- Planned Water Reclamation Plant
- Existing Recycled Water Tank
- Proposed Phase 2 Recycled Water Tank
- Proposed Phase 3 Recycled Water Tank
- Proposed Pump Station
- Proposed PRV
- Existing Phase 1 Pipeline
- Proposed Vista Canyon RW Pipeline
- Alternative 1 - NPR Phase 2
- Alterantive 2 - NPR Future Expansion
- Planned Developments
- Existing Parks and Golf Courses
- Castaic Lake Water Agency Service Area
- Newhall County Water District
- LACWWD36
- Santa Clarita Water Division
- Valencia Water Company

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Recycled Water Master Plan
Santa Clarita, California

Alternatives 1 & 2
Non-Potable Reuse Expansion

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Figure ES-4

Figure ES-5 Alternative 3 – GRR via Surface Spreading Alternatives

Alternative 4 - Advanced Treatment for Potable Reuse: considers long-term opportunities to implement a potable reuse project. This alternative considers both indirect and direct potable reuse projects that require advanced treatment to meet regulatory requirements, including:

1) Groundwater recharge via direct injection in the vicinity of the Valencia WRP and other viable locations with the Valley,
2) Surface water augmentation at Castaic Lake and
3) Direct potable reuse by blending with the raw water supply at the Rio Vista WTP.

Similar to Alternative 3, the amount of recycled water that can be advanced treated for potable reuse is limited by the available supply because irrigation demands for Phase 1, Phase 2 and future customers would use all available summer supplies that are not required for discharge. However, since these projects would not be limited by stormwater capture prioritization, the total volume of water available in winter and shoulder months could be utilized. Each Alternative 4 project would include AWTF, located at the Valencia WRP or near the potable WTP, and brine disposal via truck hauling. The AWTF is assumed to be similar to the SCVSD's Chloride Compliance Project treatment train, and would consist of MF, enhanced brine concentration (EBC), RO and UV for disinfection with advanced oxidation process (AOP). The DPR project would also include ozone and biologically activated carbon (BAC) pre-treatment to offer two new and different mechanisms to control the wide diversity of potential chemical and microbiological threats. An overview of the alignments and associated infrastructure for these projects is shown in Figure ES-6.

Figure ES-6 Alternative 4 – Advance Treatment for Potable Reuse Alternatives

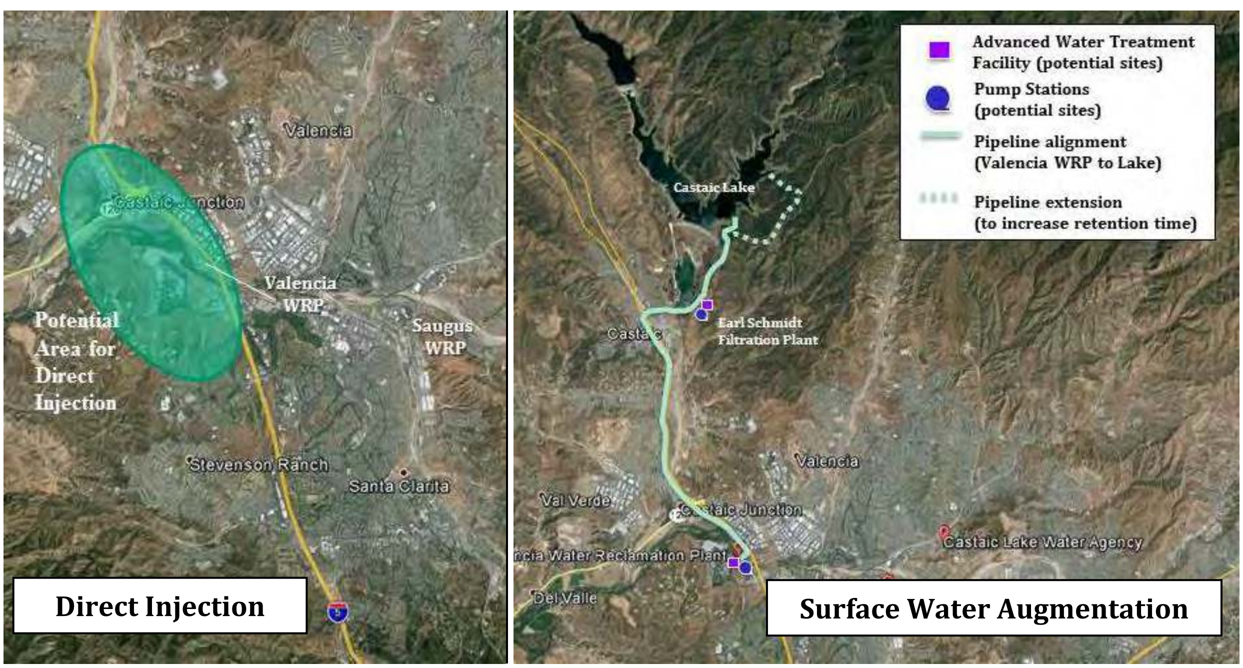


Figure ES-6: Alternative 4 – Advance Treatment for Potable Reuse Alternatives (con't)

Other considerations that were explored as part of the RWMP include repurposing existing infrastructure, seasonal storage and customer retrofits.

Repurposing Existing Infrastructure: CLWA and the purveyors have identified four existing assets that could potentially be repurposed for recycled water (1) a groundwater transmission main on Newhall Ranch Road, (2) the Honby Lateral that crosses the Santa Clara River at Golden Valley Road, (3) the Honby Pipeline that traverses Soledad Canyon Road and terminates at Sand Canyon Road and (4) the currently inactive Honby Pump Station. Rehabilitating an existing asset has the potential to reduce conveyance costs and environmental impacts of construction. With all of these facilities, additional investigations and studies are required to ascertain the viability of repurposing them for use with the future recycled water system. For the purpose of the RWMP and associated programmatic EIR, the alternatives presented in the prior sections assume construction of new facilities (with the exception of the last two projects in Alternative 3).

Seasonal Storage: To maximize unused water supply in the winter months when irrigation demand is low, water can be stored for use in the summer months when irrigation demand is high. Based on the projected monthly supply of recycled water less discharge to the Santa Clara River and when irrigation demands utilize all available summer supply, there would be approximately 5,500 AFY of recycled water available to store seasonally in the year 2050 to allow for further expansion of recycled water for irrigation. Note that this is the same volume considered to be available for potable reuse in Alternatives 3 and 4. Seasonal storage is very expensive and the feasibility would depend on availability of land, construction costs for reservoir, pipelines and pump stations to fill the reservoir, conveyance costs to serve new customers, permitting and environmental mitigation costs, water quality requirements, public acceptance, and ability to finance. Due to these challenges, this RWMP does not include an alternative to construct new above ground seasonal storage; however, the GRR alternatives utilize below ground seasonal storage and SWA alternative uses an existing reservoir for seasonal storage to similarly maximize reuse in Santa Clarita Valley during the winter and shoulder months.

Customer Retrofits: Serving existing irrigation sites with recycled water requires on-site evaluations and identification of retrofit requirements, which must comply with local guidelines and permit/code requirements. Most of the landscape irrigation systems in the Santa Clarita Valley are metered separately from the potable system and could be easily retrofitted to receive recycled water by following the guidelines in Title 17 of the California Code of Regulations (CCR). Mixed meters that serve both the irrigation and potable system are more complex to retrofit; however for larger users such as schools or commercial/industrial areas with significant landscaping demands, retrofits can still be cost effective. Existing buildings that have not been constructed with dual-plumbing systems can be complex and expensive to retrofit , and therefore, such sites would only be considered potential customers if they have a high demand use such as a cooling tower that can be easily separated from the potable water system. For the purpose of the alternatives analysis, unit costs for retrofits were developed using a cost equation based on the irrigated area in square feet and added as a one-time capital cost.

An engineer's opinion of probable cost was developed for each alternative project based on a conceptual level estimate of capital and operating costs. Planning-level opinions of capital, operations and maintenance (O&M), and lifecycle costs were developed to facilitate an economic comparison of the projects. Table ES-4 summarizes the four alternatives, their associated demands and costs.

Table ES-4: Summary of Alternative Demands

Alternative	Project	Description	Ave Annual Demand (AFY)	Estimated Construction Cost ($mil)	Annualized Unit Construction Cost ($/AF)	Annual O&M Cost ($/AF)	Total Annual Cost ($/AF)
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)	Phase 2A[1]	Bouquet Canyon Road	482	$20	$2,400	$490	$2,890
		Central Park South w/o Tank	560	$24	$2,400	$480	$2,880
		Central Park South w/ Tank	560	$25	$2,600	$560	$3,160
	Phase 2B	Combined SCWD + Vista Canyon	300	$7	$1,300	$260	$1,560
	Phase 2C	VWC + NCWD Extensions	1,374	$24	$1,000	$270	$1,270
	Phase 2D	VWC Extension	186	$3	$1,000	$660	$1,660
Alternative 2 - Non-Potable Reuse Expansion (Future Phases) [2]	Phase 2A + Future Expansion North	Includes Phase 2A and Future Alignments E-H to expand reuse North of the Santa Clara River	1,904	$77	$2,300	$600	$2,900
	Phase 2C + Future Expansion South	Includes Phase 2C and Future Alignments A-D to expand reuse South of the Santa Clara River	2,391	$71	$1,700	$490	$2,190
	Westside Communities[3]	Includes non-potable demands for proposed developments at buildout served as an independent system from Phase 1 & 2	7,184	$123	not included	$300	$300
Alternative 3 - Groundwater Recharge (Surface Spreading)[4]	Phase 2A + Spreading Site #1	Includes Phase 2A costs and maximizes deliveries to Off-Stream Spreading Site #1	3,410	$76	$1,300	$500	$1,800
	Phase 2A + Spreading Site #3a	Includes Phase 2A costs and maximizes deliveries to In-Stream Spreading Site #3a	3,010	$95	$1,800	$700	$2,500
	Phase 2A + Spreading Site #3b	Includes Phase 2A costs and maximizes deliveries to Off-Stream Spreading Site #3b	3,010	$108	$2,100	$700	$2,800
	Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	Includes Phase 2A costs and reuses Honby lateral and Honby pipeline to deliver to In-Stream Spreading Site #3b, which limits the amount of flow delivered.	1,660	$62	$2,100	$900	$3,000
	Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)	Includes Phase 2A costs, splits deliveries between Spreading Sites #1 & #3b, and reuses Honby lateral and Honby pipeline	3,410	$98	$1,700	$700	$2,400
Alternative 4 - Advanced Treatment for Potable Reuse[5]	Direct Injection	Direct injection of advance-treated water near Valencia WRP	4,250	$279	$3,800	$1,400	$5,200
	Surface Water Augmentation	Augment Castaic Lake with advance-treated water	4,250	$262	$3,600	$1,500	$5,100
	Direct Potable Reuse + Phase 2A	Augment raw water to Rio Vista WTP with of advance-treated water (includes Phase 2A)	4,810	$283	$3,900	$1,400	$5,300

[1] Three variations are shown for Phase 2A; only one Phase 2A project would be selected
[2] Due to limited supply of recycled water in the summer months, not all of the Alternative 2 projects could be implemented.
[3] Capital construction costs for the Westside Communities (estimated at $123 million) are assumed to be paid for by the developer and are therefore not included in the annualized total cost.
[4] Since spreading would occur primarily in winter and shoulder months, an Alternative 2 project and an Alternative 3 project could both be implemented; however only one Alternative 3 project would be selected.
[5] An Alternative 4 project would utilize all water not used for irrigation and could be implemented instead of an Alternative 3 project; only one Alternative 4 project would be selected.

ES.5 Alternatives Evaluation

The alternatives were evaluated based on the following considerations to identify a recommended project, or set of projects for the RWMP: cost comparison, water supply availability, readiness to proceed, permittability, required agency coordination/collaboration, ease of implementation and environmental considerations. A summary of the findings for each alternative is provided below:

- **Alternative 1 - Phase 2B, 2C and 2D projects** (1) are the lowest cost projects that serve existing irrigation customers, (2) have sufficient recycled water supply available, (3) have initiated design and environmental work and are in-line for funding, (4) are currently permittable and would be similar in operation to the existing Phase 1 system, (5) are the easiest to implement in terms of regulatory requirements, public acceptance and infrastructure requirements. The **Alternative 1 - Phase 2A** project has similar circumstances to the other Phase 2 projects; however, since the sizing of the transmission pipeline is dependent on the feasibility of GRR and future non-potable expansion decisions, this project would proceed once the status of those projects is more defined.

- Similar to Alternative 1, the **Alternative 2 projects** are (1) currently permittable, (2) the easiest to implement in terms of regulatory requirements, public acceptance and infrastructure requirements and (3) would be similar in operational requirements to the existing Phase 2 system. However, due to limited supply of recycled water in the summer months, not all of the Alternative 2 projects can be implemented based on assumed supply, demands and river discharge requirements.

    - **Alternative 2 Westside Communities** is the most cost effective project to expand beyond Phase 2; however, the benefit of the developer funding the capital infrastructure is tempered by the challenge of having less control over the schedule for development. Thus, uncertainty of the readiness of this development to proceed may defer this project and could potentially result in a decision to pursue one of the other Alternative 2 Projects.

    - **Alternative 2 – Phase 2C + Future Expansion South** has a lower annualized unit cost than **Alternative 2 – Phase 2A + Future Expansion North** in part because of the higher volume of recycled water delivered. If additional supplies become available, then the Future Expansion Projects North and/or South could be potentially implemented. Since it is likely that Phase 2C will be constructed and in operation before a decision is made about whether to expand beyond Phase 2; extending services to the north (off Phase 2A) would allow for more time to determine the appropriate conveyance facility requirements to accommodate the additional expansion north.

- The **Alternative 3 GRR projects** offer a unique opportunity to create a multi-beneficial project and utilize excess recycled water available in the winter and shoulder months when irrigation demands are lower. These projects provide the added benefit of comingling recycled water and stormwater to recharge areas of the groundwater aquifer with a local and drought proof supply. Due to the unique nature of these projects, additional evaluation is needed to assess the feasibility, permittability and public acceptability of groundwater replenishment in the Santa Clarita Valley. A **GRR Feasibility Study** is needed to confirm the viability of this alternative through additional modeling, coordination with the Los Angeles County Flood Control District (LACFCD) and initial discussions with the regulators, landowners and the public.

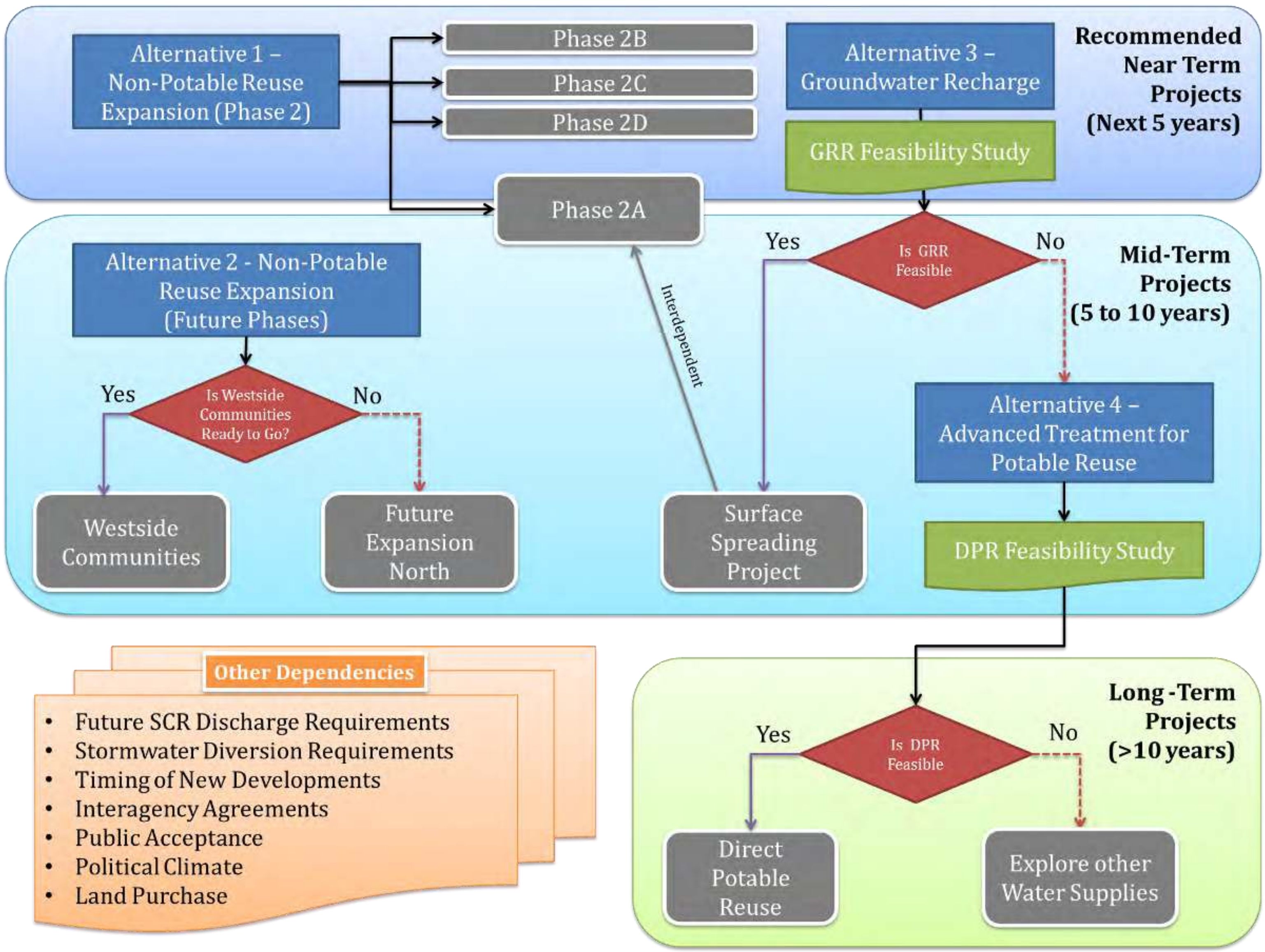
- The **Alternative 4 Advanced Treated Reuse Projects** would be the most expensive due to the need for additional treatment. Unlike the other alternatives, these projects require construction of an AWTF and brine disposal, which would likely have similar challenges to the SCVSD Chloride Compliance Project. These projects are subject to more regulatory uncertainty and the public acceptance of potable reuse is uncertain at this time. Based on the assumptions in this RWMP related to irrigation demands and discharge requirements, there would only be sufficient supplies for an Alternative 4 project if Alternative 3 is found to be infeasible. An **Alternative 4 – Surface Water Augmentation** project would require coordination with the Metropolitan Water District given the shared use of Castaic Lake for water supply and may not meet the anticipated regulatory requirements for a SWA project due to the limited retention time. The viability of **Alterative 4 – Direct Potable Reuse** is dependent on future regulations and the progress of other DPR projects in California; both of which should be tracked in the long-term.

ES.6 Recommended Project

There are many projects that provide viable opportunities to expand the use of recycled water in Santa Clarita Valley in the near-, mid- and long-term. The decision to pursue one project over another may, in some cases, depend on external factors, such as the progress of private developments, future discharge requirements, the availability of land, political climate, agreements with other agencies and the permittability and public acceptance of potable reuse.

For the purpose of this RWMP the Recommended Project is defined as a course of action in the near-term to expand recycled water in Santa Clarita Valley to offset potable demands and maximize the use of "local" water supplies. Based on the evaluation of alternatives, a decision flow process is presented in Figure ES-7 to illustrate the Recommended Projects to pursue in the near-term, and the decision points to guide the future expansion of the CLWA recycled water program in the mid- and long-term.

Figure ES-7: Recycled Water Program – Decision Flow Chart



The Recommended Project includes the following activities:

(1) **<u>Implement Alternative 1 - Non-Potable Reuse Expansion Projects - Phase 2B, 2C and 2D.</u>** The Phase 2 projects, shown in Figure ES-8, are currently in various stages of design and environmental work and are progressing thought the efforts of CLWA and/or the lead purveyor assigned to each project. These projects are already in-line for Proposition 1 funding and may be competitive for other funding opportunities. Together, these three projects will increase the recycled water delivery in Santa Clarita Valley from 450 AFY to 2,310 AFY.

(2) **<u>Complete preliminary design and environmental work for Alternative 1 Non-Potable Reuse Expansion Project - Phase 2A</u>**. Given the interdependency of the Phase 2A transmission pipeline with other potential future expansion opportunities, it is recommended that the backbone pipeline be sized with a 24-inch diameter pipeline to meet potential future demands for Alternative 2 – Future Expansion North, Alternative 3 - GRR or Alternative 4 – DPR. Final design for Phase 2A should be deferred until the feasibility of GRR is determined.

(3) <u>Initiate a GRR Feasibility Study to evaluate the viability of Alternative 3 GRR projects.</u> The feasibility study would include additional hydrogeologic, hydrologic and operations modeling to confirm assumptions, coordination with LACFCD regarding implementing a cooperative recycled water-stormwater recharge project, discussions with DDW and the RWQCB regarding permitting, communication with land owners to confirm the availability of the spreading sites and outreach to the public about indirect potable reuse. The study would include evaluation of the alternatives for surface spreading of tertiary recycled water, confirmation of sources and availability of diluent water, and could explore opportunities for direct injection of advanced treated recycled water.

Z:\Projects\CastaicLakeWaterAgency_RW\Events\20160415_Figs\Fig07-1_OverallAlt1Map.mxd printed by: marioo

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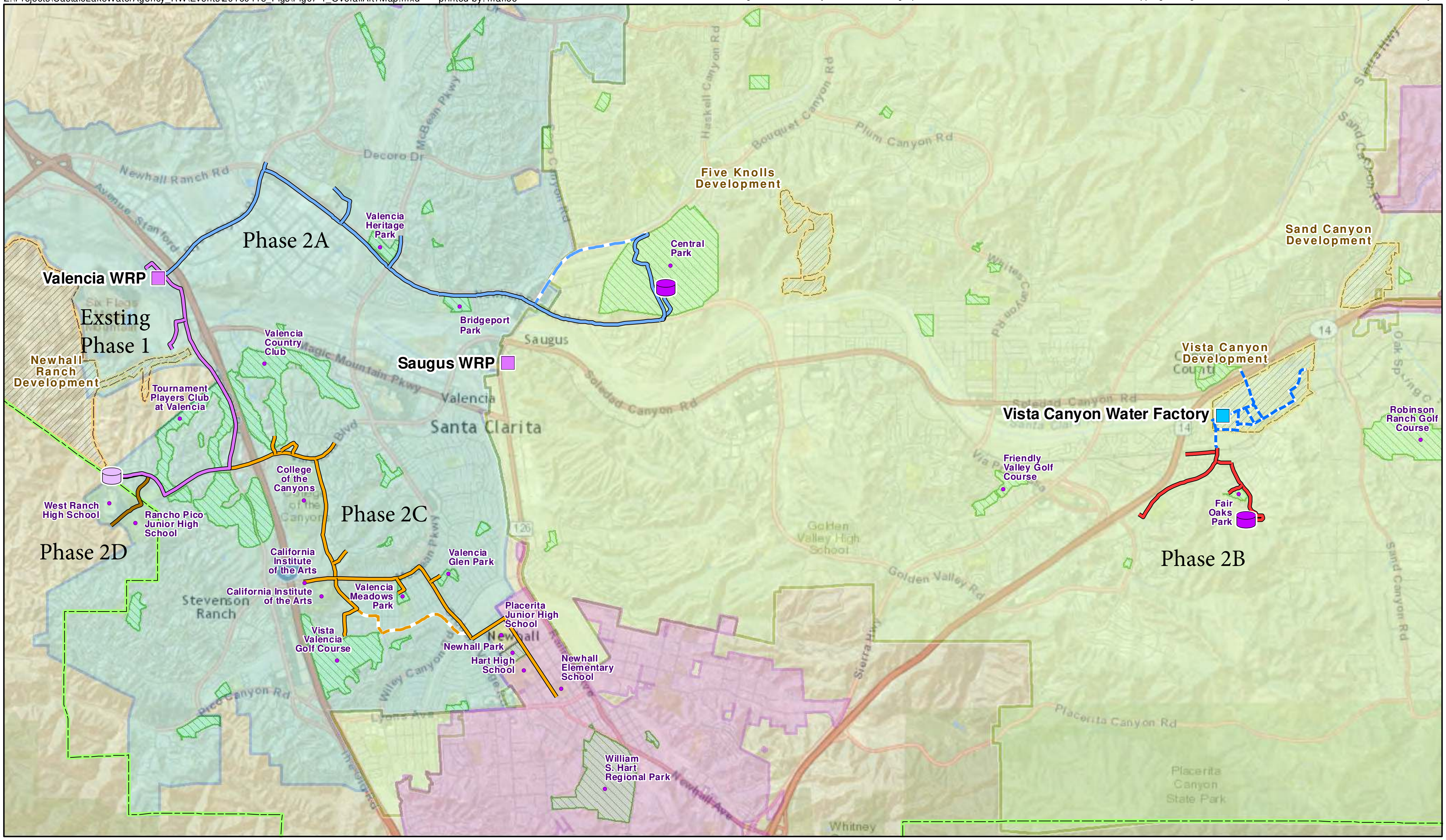


Legend

- Existing Water Reclamation Plant
- Planned Water Reclamation Plant
- Existing Recycled Water Tank
- Proposed Recycled Water Tank
- Existing Phase 1 Pipeline
- Proposed Vista Canyon RW Pipeline
- Planned Phase 2A Pipeline
- Planned Phase 2B Pipeline
- Planned Phase 2C Pipeline
- Planned Phase 2D Pipeline
- Castaic Lake Water Agency Service Area
- Newhall County Water District
- Santa Clarita Water Division
- Valencia Water Company
- Planned Developments
- Existing Parks and Golf Courses

N

0 3,000 6,000
Feet

Kennedy/Jenks Consultants

Castaic Lake Water Agency
Recycled Water Master Plan
Santa Clarita, California

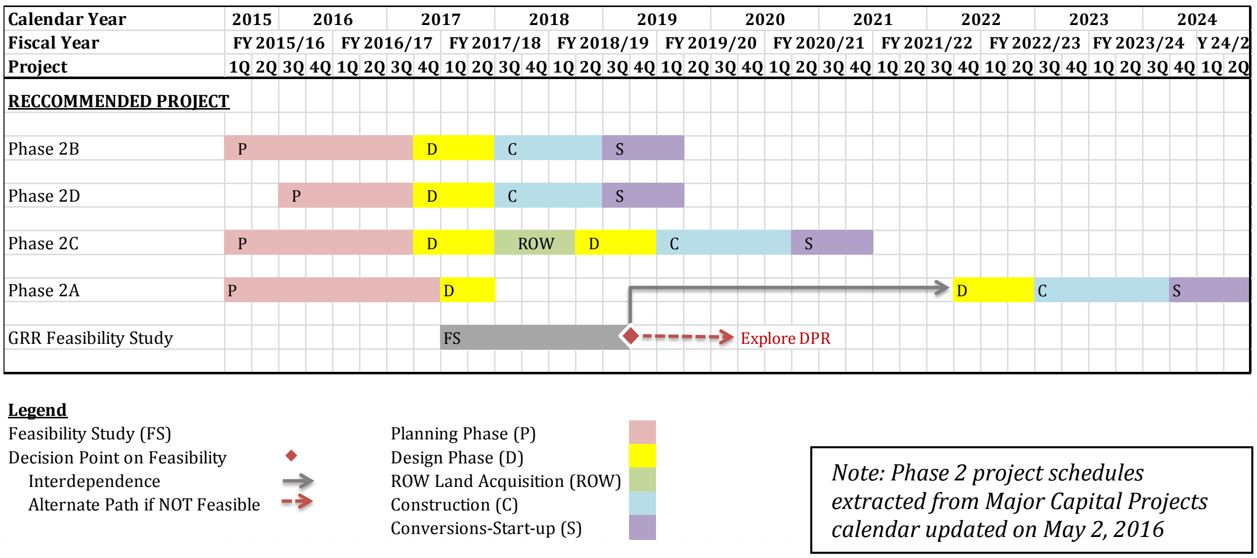
Reccommended Phase 2 Projects

K/J 1544241.00
September 2016

Figure ES-8

A potential phasing plan for the Recommended Projects is presented in Figure ES-9 based on the considerations discussed earlier in this section and the decision flow process presented in Figure ES-8.

Figure ES-9: Potential Phasing Plan for the Recommended Projects



ES.7 Next Steps

CLWA and the purveyors recognize that recycled water is a critical component of their water supply portfolio and expanding the recycled water system in the Santa Clarita Valley provides a reliable source of water year round that can help offset reliance on imported water and local groundwater. The following list summarizes key activities to implement the Recommended Projects as well as mid-term and long-term activities to support the continued expansion of reuse.

1. **Development of Agreements:** Memorandums of Understanding (MOUs) between CLWA and each purveyor were developed to establish a framework to guide the implementation of the recycled water program[1]. Project specific agreements will need to be developed for each project to define roles, cost sharing commitments, funding mechanisms, ownership and operations and maintenance responsibilities over the life of the project.

2. **Implementation Plan for Alternative 1 – Phase 2 Projects:** Common implementation elements to all Phase 2 Projects include (1) developing the customer base, (2) performing preliminary design and engineering feasibility studies, (3) obtaining regulatory approval, (4) completing final design and construction, and (5) providing appropriate training and

[1] The executed MOU for each purveyor is included in Tab 14 of the SCV Rules and Regs Handbook (Kennedy/Jenks 2016b).

guidance for site supervisors. Some of the Phase 2 projects have already completed some of these elements and/or work is currently being performed under separate contracts.

3. **Implementation Plan for GRR Feasibility Study:** This study would include exploration of surface spreading (Alternative 3) and direct injection (Alternative 4) to identify a GRR Project that is implementable, acceptable by the CLWA Board and staff, supported by the regulators and stakeholders, and affordable. The GRR Feasibility Study could be led by CLWA or a project proponent and ideally would engage the LACFCD to be a project partner. The GRR Feasibly Study may include, but not be limited to, the following: (1) groundwater modeling, (2) hydrologic evaluation for stormwater recharge, (3) coordination with SCVSD, LACFCD, and the appropriate regulators, (4) confirmation of the availability of land for the spreading sites, (5) investigation of re-purposing existing infrastructure, (6) development of a public outreach and communication strategy, (7) environmental analysis and (8) update project costs and recognition of non-quantifiable benefits for the different GRR projects. If GRR is determined to be feasible, the study would identify a preferred project and the next steps for implementation.

4. Beyond the Recommended Project, **activities conducted in the mid-term** should be focused on optimizing expansion of the recycled water system beyond Phase 2, and could include: (1) tracking recycled water deliveries from the Phase 1 and 2 projects to understand peak irrigation demands and to improve operational efficiency of the recycled water system, (2) following SCVSD's efforts related to the Chloride Compliance Project and instream flow requirements and (3) monitoring the status of the Westside Communities development. A key decision point may arise if the Westside Communities development is only partially built or put on hold indefinitely, at which point CLWA and the purveyors would have the opportunity to pursue other Alternative 2 projects. The selection of the next best project(s) would likely be influenced by a combination of the outcome of the GRR Feasibility Study, climatic conditions, water supply availability, imported water rates, and political influences.

5. The Alternative 4 projects represent **long-term opportunities** to maximize reuse in the Santa Clarita Valley that would require an AWTF and brine disposal, at a high capital and operating cost. SWA and DPR projects would only be pursued if GRR is not selected for implementation. A DPR project represents the most cost effective Alternative 4 project; however the viability of DPR is contingent on regulatory and legislative progress and public acceptance. CLWA and the purveyors should continue to track DPR developments to understand the possibilities, benefits and limitations for implementing a project in Santa Clarita Valley in the future.

Section 1: Introduction

Due to the ongoing drought in California and the resulting fluctuations in water supply, the Castaic Lake Water Agency (CLWA) is seeking opportunities to determine the most appropriate way to expand their existing recycled water system to offset potable water demands and improve water supply reliability. This Recycled Water Master Plan (RWMP) updates the 2002 Recycled Water Master Plan based on recent developments affecting recycled water sources, supplies, uses and demands and explores opportunities to maximize the utilization of recycled water in the Santa Clarita Valley.

1.1 Background

CLWA is the public water wholesaler in Santa Clarita Valley; delivering water to four local water purveyors: Los Angeles County Waterworks District 36 (LACWD36), Newhall County Water District (NCWD), Santa Clarita Water Division of CLWA (SCWD), and Valencia Water Company (VWC). Collectively, CLWA and the retail purveyors are the Santa Clarita Valley's 'water suppliers'.

CLWA is one of 29 State Water Project (SWP) contractors and receives water imported from northern California via the California Aqueduct. CLWA also receives imported water acquired from the Buena Vista Water Storage District in Kern County and Yuba County Water Agency, has access to "flexible storage" in Castaic Lake and has entered into four groundwater banking and water exchange programs. Even with this diverse portfolio of water supplies, the extreme prolonged drought conditions have required CLWA to focus on conservation and recycled water use in order to maximize water supplies.

Since 2003, CLWA has been receiving tertiary treated water from the Santa Clarita Valley Sanitation District's (SCVSD) Valencia Water Reclamation Plant (WRP), and wholesaling the recycled water through VWC within its territory for sale to retail customers for appropriate uses. The existing recycled water system (Phase 1) includes: the Valencia WRP Recycled Water Pump Station, a recycled water tank in the Westridge community, and approximately 15,600 feet of recycled water pipelines. Annual recycled water usage has averaged 370 acre-feet per year (AFY) for the last 10 years. Ninety percent of water use is between May and October.

1.2 Drivers and Objectives

In normal years, approximately 55 percent of the municipal and industrial demands within CLWA's service area are met with imported water. However, the reliability of the SWP is subject to the availability of the water (i.e., precipitation and snowpack of the present and past years) and deliveries can be curtailed. When sufficient water supply is not available, the balance is met with local groundwater provided by the purveyors. It is anticipated that water demands will continue to increase with increasing population. Accordingly, additional reliable sources of water may be necessary to reliably meet projected water demands. CLWA recognizes that local recycled water is an important and reliable source of additional water. Recycled water enhances reliability in that it provides an additional source of supply and allows for more efficient utilization of local groundwater and imported water supplies. By increasing the use of recycled water, CLWA and the local purveyors can conserve potable drinking water and increase the reliability of water supplies in the Santa Clarita Valley.

The primary objective of this RWMP is to update the 2002 Recycled Water Master Plan based on recent developments affecting recycled water sources, supplies, uses and demands. It is the intent of the CLWA and the purveyors in Santa Clarita Valley to make recycled water available and encourage its use where authorized and economically feasible.

This RWMP evaluates near-term, mid-term and long-term objectives as follows:

1. **Near-Term Objective:** Incorporate updates to the next phase of recycled water projects (Phase 2) to support master planning, upcoming design work and the pursuit of currently available grants and loans for non-potable recycled water projects. Initiate exploration of opportunities to expand reuse in the Santa Clarita Valley, including groundwater recharge with recycled water.

2. **Mid-Term Objective:** Optimize expansion of the non-potable recycled water system based on updated demand projections associated with planned new developments and available supplies. Further investigate or implement next steps for potable reuse.

3. **Long-Term Objective:** Further explore opportunities for potable reuse through surface water augmentation and/or direct potable reuse.

1.3 Previous Studies

The following reports evaluated potential opportunities for recycled water use in the CLWA service area.

Recycled Water Master Plans -1993 & 2002

An initial Reclaimed Water System Master Plan (RWMP) was completed for CLWA in 1993 (Kennedy/Jenks 1993) and an update to the 1993 RWMP was completed in 2002 (Kennedy/Jenks 2002) to address the changes in the area that had occurred in the last preceding decade. The information developed in the 2002 RWMP was largely drawn from the 1993 RWMP supplemented with new information from CLWA, Sanitation Districts of Los Angeles County (LACSD), local water purveyors, the City of Santa Clarita, the County of Los Angeles, oil company representatives, and potential water users. Additional analysis and computer modeling were performed as part of the 2002 Master Plan update. Water demand characteristics were also updated through discussions with potential users. The updated data and computer modeling were used to develop a revised cost-effective recycled water system. Construction costs and a construction schedule are included in the update.

The 2002 RWMP recognized that current WRP production is not anticipated to be adequate to meet the total demands of the CLWA service area. However, as potable water demands increase, recycled water production would similarly increase, thereby becoming more available to support non-potable uses in lieu of imported potable water or groundwater. The implementation plan outlined in the 2002 RWMP was phased to utilize the increases in plant production. Implementation phases were prioritized based on the status of the users (existing or future), the anticipated construction schedule of future users, and the proximity of the users to the recycled water source.

Water Resources Reconnaissance Study (2015)

CLWA and the purveyors commissioned a Water Resources Reconnaissance Study (Recon Study) to evaluate alternatives for expanding local supplies to offset future periodic occurrences of significant shortfalls in imported water supplies (Carollo 2015). The Recon Study provided an initial assessment of groundwater recharge with recycled water through surface spreading into the alluvial aquifer and aquifer storage and recovery via groundwater injection into the deeper Saugus formation. Groundwater recharge with recycled water through surface spreading has been further reviewed and refined as part of this RWMP.

Salt and Nutrient Management Plan (2016)

A Salt and Nutrient Management Plan (SNMP) for the Santa Clara River Valley East Subbasin is being developed in accordance with the State Water Resources Control Board's (State Water Board's) Recycled Water Policy (Policy) and is anticipated to be completed and adopted by the end of 2016. A Salt and Nutrient Task Force, facilitated by CLWA, is preparing the SNMP to determine the current (ambient) water quality conditions in the East Subbasin and ensure that all water management practices, including the use of recycled water, are consistent with water quality objectives. The SNMP is intended to provide the framework for water management practices to ensure protection of beneficial uses, and allow for the sustainability of groundwater resources consistent with the Basin Plan (Geoscience 2015).

The SNMP recognizes the benefits of increased recycled water reuse in the East Subbasin. Furthermore, the SNMP demonstrates that implementation of proposed recycled water projects represent a "maximum benefit" to the people of the State by providing beneficial uses for recycled water decreasing the use of assimilative capacity as compared to not adding planned projects to the East Subbasin (Geoscience 2015).

1.4 Master Plan Organization

The report is organized to align with the State Water Resources Control Board (SWRCB) Water Recycling Funding Program Guidelines - Division of Financial Assistance, Appendix B - Recommended Planning Outline for Water Recycling Projects, which will facilitate future applications for funding through the State Revolving Fund (SRF) program. The organization of this RWMP will also serve to meet the Proposition 84 requirements, which provided partial grant funds for this study. The RWMP is organized as follows:

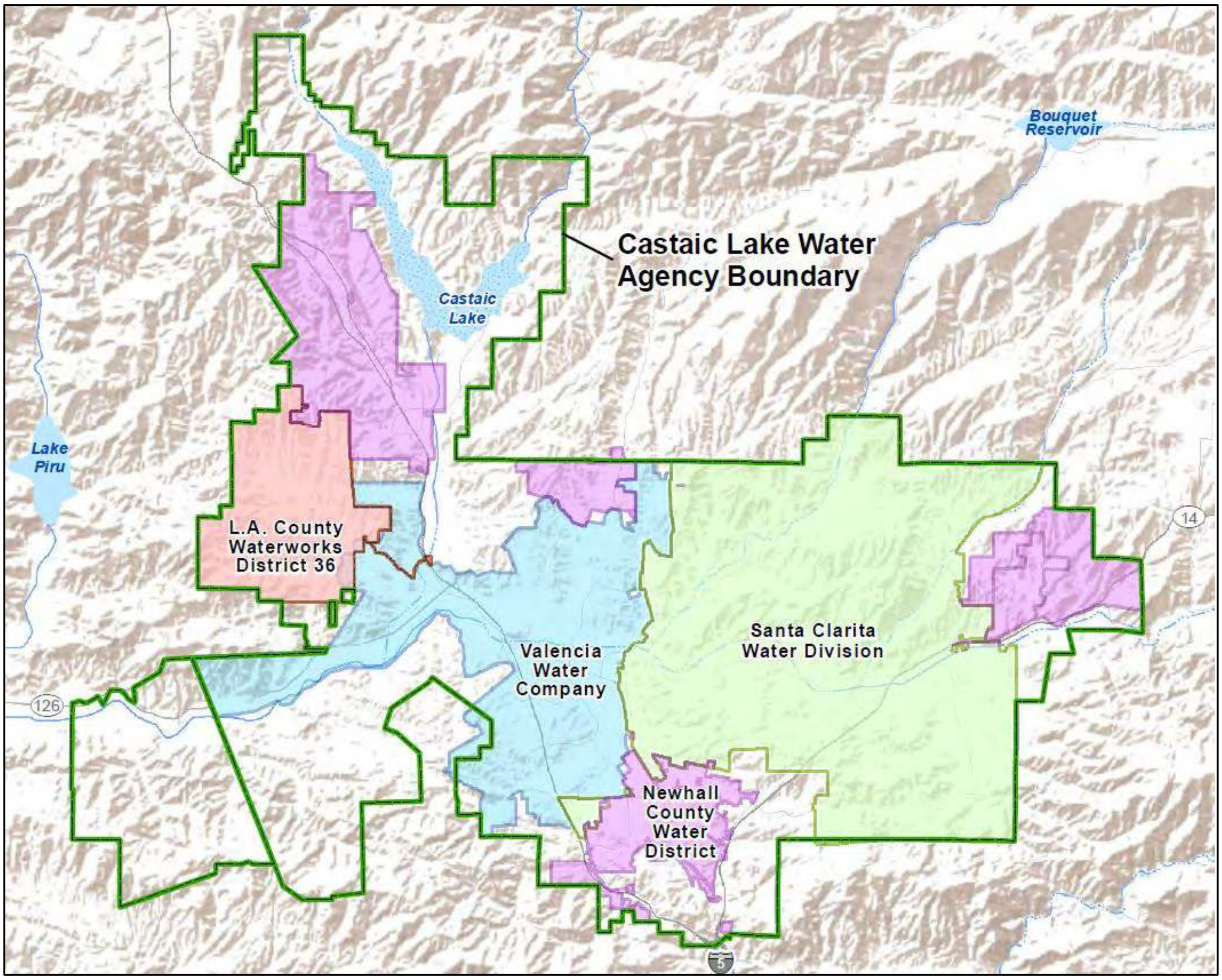
- **Section 1: Introduction** – summarizes the background and objectives of the Recycled Water Master Plan as well as addresses previous studies.
- **Section 2: Study Area Characteristics** – describes the study area, major hydrologic features, water quality, land use, population projections and beneficial uses of receiving waters in the Santa Clarita Valley.
- **Section 3: Water Supply Characteristics and Facilities –** describes wholesale and retail entities, water supplies and usage, water supply reliability and future sources of additional demand.
- **Section 4: Wastewater Characteristics and Facilities –** presents an overview of water recycling facilities, effluent flows and recycled water quality in the Santa Clarita Valley.
- **Section 5: Treatment Requirements –** discusses regulations guiding recycled water production, discharge, distribution, and use to protect public health, including the most recent regulatory landscape for potable reuse.
- **Section 6: Recycled Water Market –** identifies potential recycled water users within the CLWA service area and estimates annual and peak demands.
- **Section 7: Project Alternatives Analysis –** describes the four alternatives considered and the planning and design criteria used to evaluate each alternative along with other considerations for expanding and implementing recycled water. Capital, operations and maintenance (O&M) and annualized unit costs are provided for each alternative.
- **Section 8: Recommended Project –** discusses the selection considerations for identifying a recommended project including costs, water supply availability, readiness to proceed, permittability, required coordination, ease of implementation and environmental considerations. Presents decision points and phased activities to implement near-term, mid-term and long-term projects.
- **Section 9: Construction Financing Plan and Revenue Program –** presents funding and financing options for the proposed recommended project. Discusses potential pricing policies, funding opportunities, avoided costs and lost revenues to provide a more comprehensive view of the true cost and benefit of expanding the recycled water program.

Section 2: Study Area Characteristics

2.1 Study Area

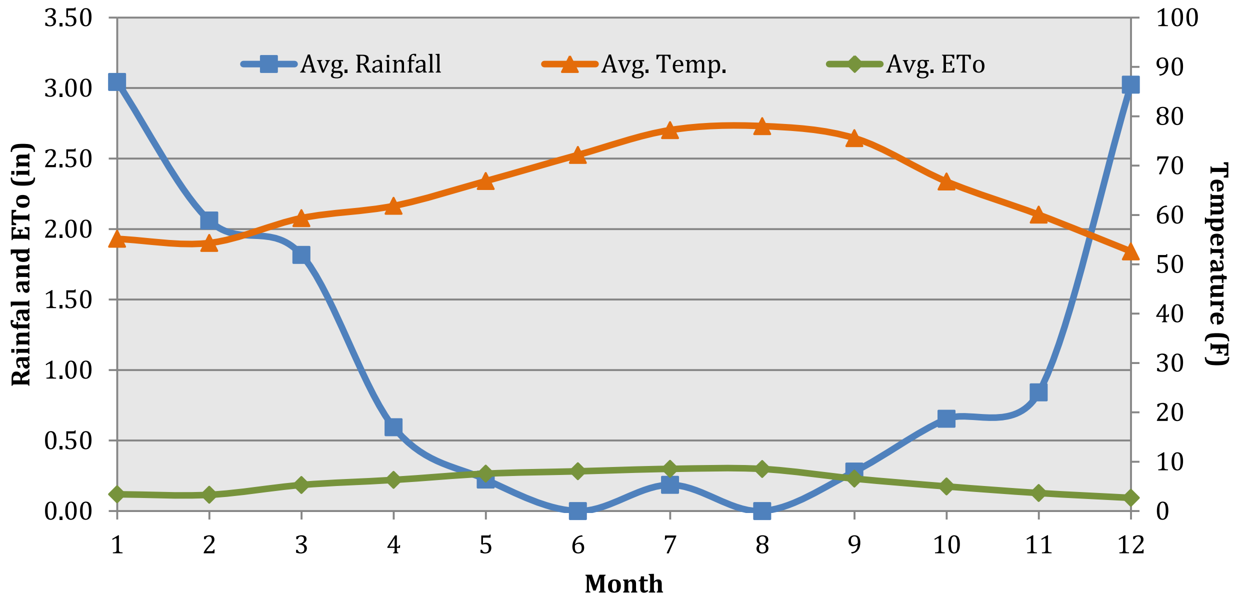
The Study area for the RWMP includes the CLWA Service Area, as shown in Figure 2-1.

Figure 2-1: Study Area



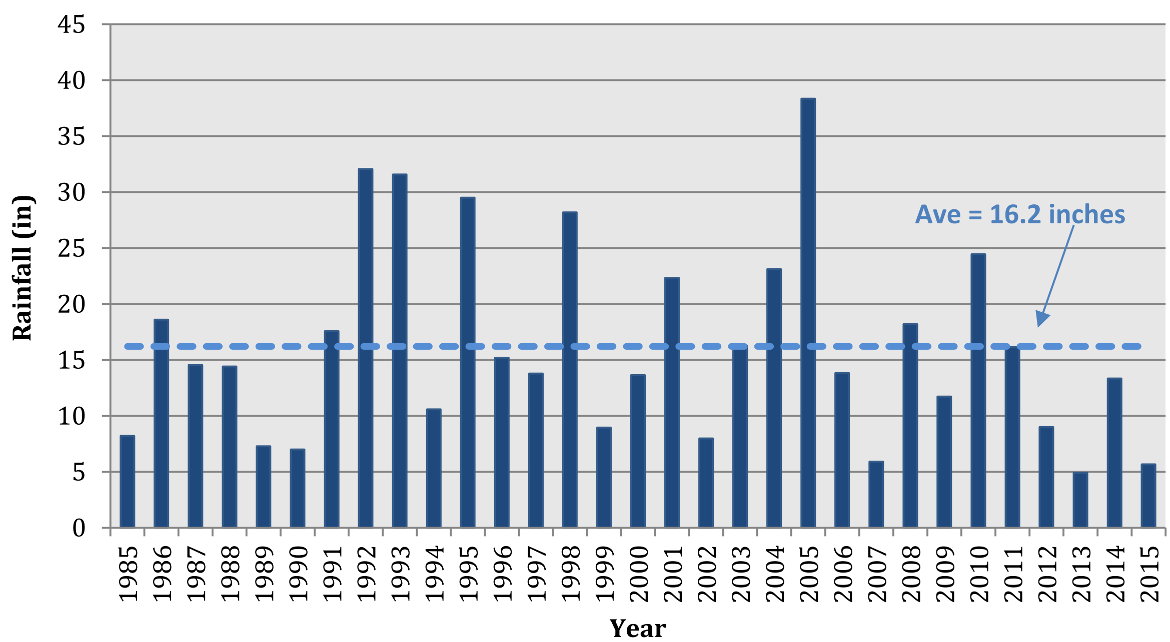
The climate in CLWA's service area is generally semi-arid and warm. Summers are dry with temperatures as high as 110°F. Winters are somewhat cool with temperatures as low as 20°F. Average rainfall since 1985 is approximately 16.2 inches per year in the flat areas and about 25 to 30 inches in the mountains. The region is subject to wide variations in annual precipitation and also experiences periodic wildfires. The region's average climate conditions are presented in Figure 2-2 and Figure 2-3.

Figure 2-2: Average Temperature, Evapotranspiration (ETo) and Rainfall



Source: Temp and ETo: CMIS Station #204 (2007-2015), Precipitation: Los Angeles County Department of Public Works data for Site32Z (Newhall-Fire Station 73) (2007-2015)

Figure 2-3: Historical Average Annual Rainfall



Source: Precipitation: Los Angeles County Department of Public Works data for Site32Z (Newhall-Fire Station 73)

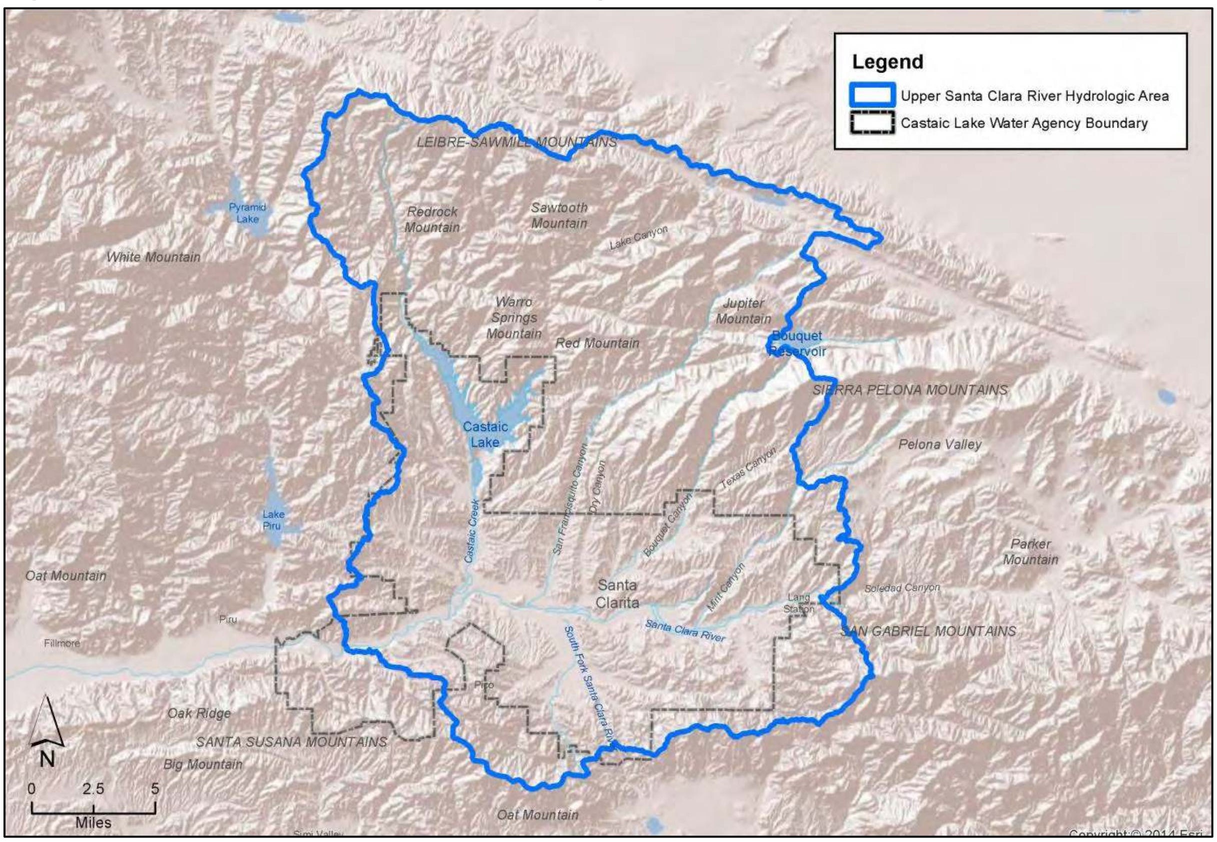
2.2 Major Hydrologic Features

Surface Water

The Santa Clara River is the Santa Clarita Valley's primary drainage course, which flows westward from Soledad Canyon through the CLWA service area to the Pacific Ocean. Major hydrologic features in the Upper Santa Clara River Hydrologic Area are depicted in Figure 2-4. All surface water flows into the Santa Clara River through year-round and ephemeral tributaries and intermittent mountain streams. Streamflow in the Santa Clara River consists of stormflow and base flow. Base flow consists of groundwater, effluent from the water reclamation plants (WRPs), reservoir releases, other point sources, bank seepage, and nonpoint discharge from agricultural and urban runoff (USGS 2003).

Castaic Lake, a man-made impoundment, is the largest surface water body within the hydrologic area, with a maximum storage capacity of 323,700 acre-fee (AF). Castaic Lake is fed State Water Project (SWP) water by the California Aqueduct and also stores flood flows.

Figure 2-4: Upper Santa Clara River Hydrologic Area

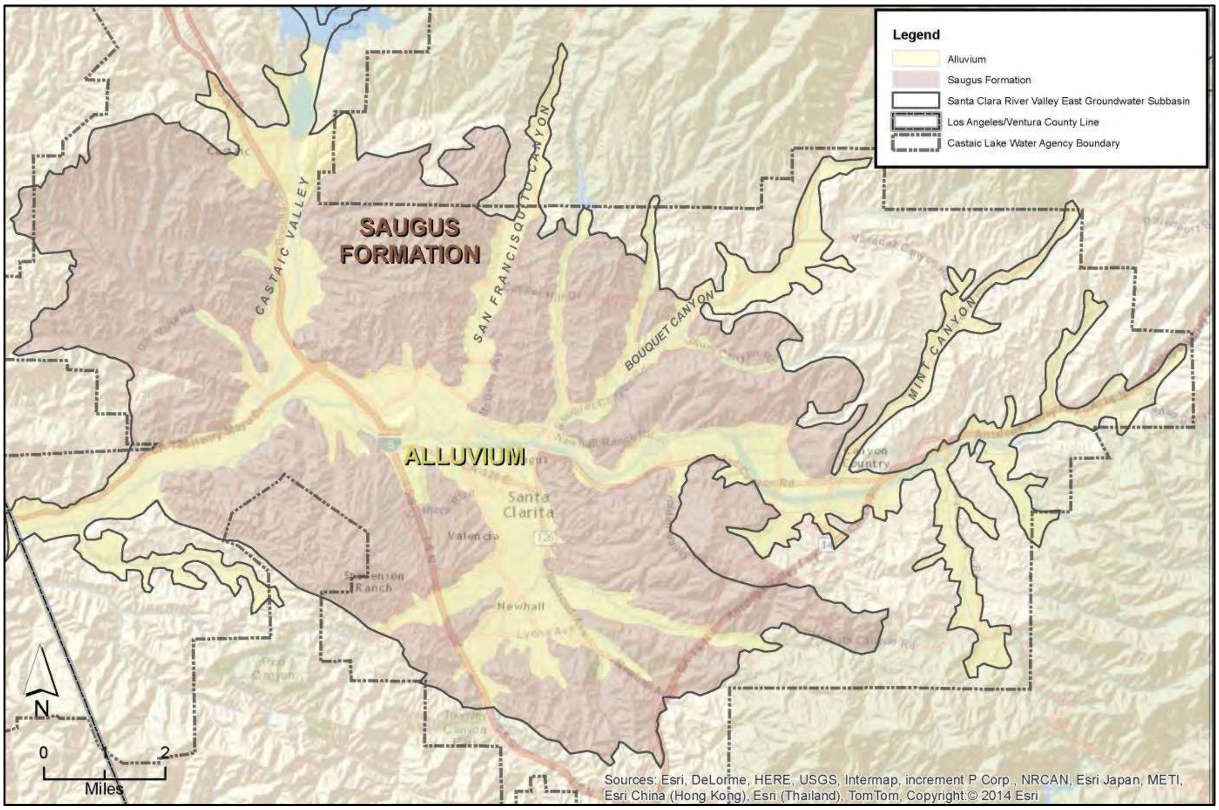


Source: 2014 Santa Clarita Valley Water Report (Luhdorff & Scalmanini, 2014)

Groundwater

The sole source of local groundwater for urban water supply in the Santa Clarita Valley is the groundwater basin identified in the DWR Bulletin 118, 2003 Update as the Santa Clara River Valley Groundwater Basin, East Subbasin (Basin) (Basin No. 4-4.07). The Basin is comprised of two aquifer systems; (1) the Alluvium and (2) the Saugus Formation, shown in Figure 2-5.

Figure 2-5: Alluvium and Saugus Formation



Source: 2014 Santa Clarita Valley Water Report (Luhdorff & Scalmanini, 2014)

The Alluvium generally underlies the Santa Clara River and its several tributaries, to maximum depths of about 200 feet; and the Saugus Formation underlies practically the entire Upper Santa Clara River area, to depths of at least 2,000 feet. There are also some scattered outcrops of Terrace deposits in the Basin that likely contain limited amounts of groundwater. However, since these deposits are located in limited areas situated at elevations above the regional water table and are also of limited thickness, they are of no practical significance as aquifers for municipal water supply; consequently they have not been developed for any significant water supply in the Basin.

2.3 Water Quality

Surface Water Quality

The Santa Clara River, shown in Figure 2-4, provides most of the annual groundwater recharge to the groundwater system and has been identified as an impaired water body; it is listed in the Clean Water Act Section 303(d) list published by the US Environmental Protection Agency (EPA). The quality of the surface water in the Santa Clara River is the product of numerous factors, such as native surface water quality entering the East Subbasin, urban and natural storm flows, discharge of treated wastewater, effluent discharges from the groundwater system (Geoscience 2015).

The State of California has determined that high levels of chloride (salt) harm salt-sensitive avocado and strawberry crops along Highway 126, downstream from the Valencia and Saugus WRPs and has ordered the SCVSD to reduce the chloride levels in the Valleys treated wastewater to below the strict legal limit of 100 milligrams per liter (mg/L), in certain portions of the river. The SCVSD has spent many years seeking the most effective solution to meeting State mandates related to the chloride levels allowed in the Valley's wastewater that is discharged to the Santa Clara River (LACSD 2013) and is currently developing advanced water treatment and disinfection facilities to comply with the revised Total Maximum Daily Load (TMDL) for Chloride in the Upper Santa Clara River, Resolution No. R4-2014-010, by July 1, 2019.

Groundwater Quality

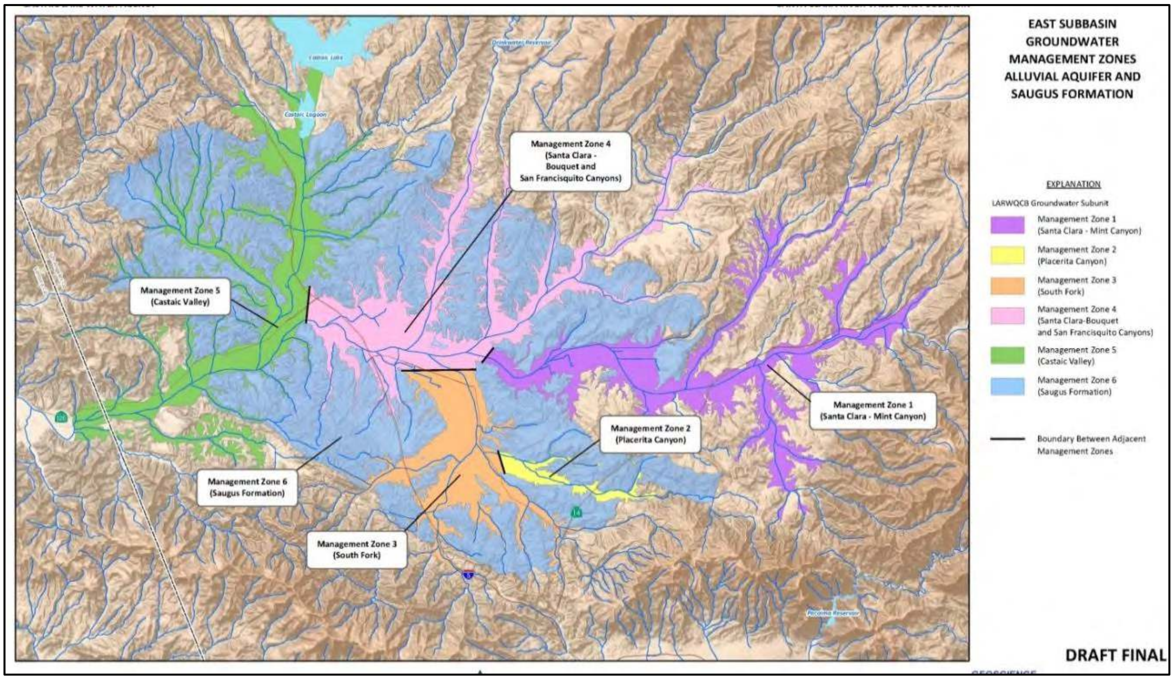
The groundwater basin has two sources of groundwater, the Alluvial Aquifer and the Saugus Formation, which is a much deeper aquifer (Figure 2-5). Local groundwater does not have microbial water quality problems. Local groundwater has very little total organic carbons (TOC) and generally has very low concentrations of bromide, minimizing potential for disinfection by-product (DPB) formation. Taste and odor problems from algae are not an issue with groundwater.

The groundwater is very "hard," and it has high concentrations of naturally occurring calcium and magnesium (approximately 250 to 600 mg/L total hardness as $CaCO3$). Groundwater may also contain higher concentrations of nitrates and chlorides when compared to SWP water for example.

Salt and Nutrient Management Plan

The Salt and Nutrient Management Plan for CLWA and the purveyors in accordance with the SWRCB's Recycled Water Policy, assesses ambient concentrations and assimilative capacities for Total Dissolved Solids (TDS), chloride, nitrate, and sulfate for six management zones shown in Figure 2-6:. The ambient groundwater concentrations and Basin Objectives for each management zone are listed in Table 2-1. The Plan has been submitted to the Regional Board for consideration and final completion is anticipated by the end of 2016.

Figure 2-6: SNMP East Subbasin Groundwater Management Zones



Source: SNMP (Geoscience, 2015)

Table 2-1: Ambient Groundwater Concentrations and Basin Objectives Management Zone

Zone	Groundwater Subunit	Water Quality (WQ) Status Comparison	TDS [mg/L]	Chloride [mg/L]	Nitrate [mg/L]	Sulfate [mg/L]
1a	Santa Clara-Mint Canyon	WQ Objective	800	150	45	150
		Ambient WQ	*728*	*89*	*20*	*138*
1b	Santa Clara-Mint Canyon	WQ Objective	800	150	45	150
		Ambient WQ	*833*	*72*	*21*	*269*
2	Placerita Canyon[1]	WQ Objective	700	100	45	150
		Ambient WQ	*NA*	*NA*	*NA*	*NA*
3	South Fork[1]	WQ Objective	700	100	45	200
		Ambient WQ	*NA*	*NA*	*NA*	*NA*
4	Santa Clara-Bouquet and San Francisquito Canyons	WQ Objective	700	100	45	250
		Ambient WQ	*710*	*77*	*16*	*189*
5	Castaic Valley	WQ Objective	1,000	150	45	350
		Ambient WQ	*727*	*77*	*8*	*246*
6	Saugus Formation[2]	WQ Objective	700	100	45	NA
		Secondary Water Quality Objective	500	250	100	250
		Ambient WQ	*636*	*28*	*14*	*235*

[1] Insufficient data to establish trend

[2] Water Quality Objectives (WQOs) have not been established for the Saugus Formation. Therefore, at the recommendation of the Los Angeles Regional Water Quality Control Board (RWQCB), the most conservative of the alluvial management zone WQOs of the alluvial management zone were used for comparison for TDS, chloride and nitrate. For information purposes, the secondary MCL is provided.

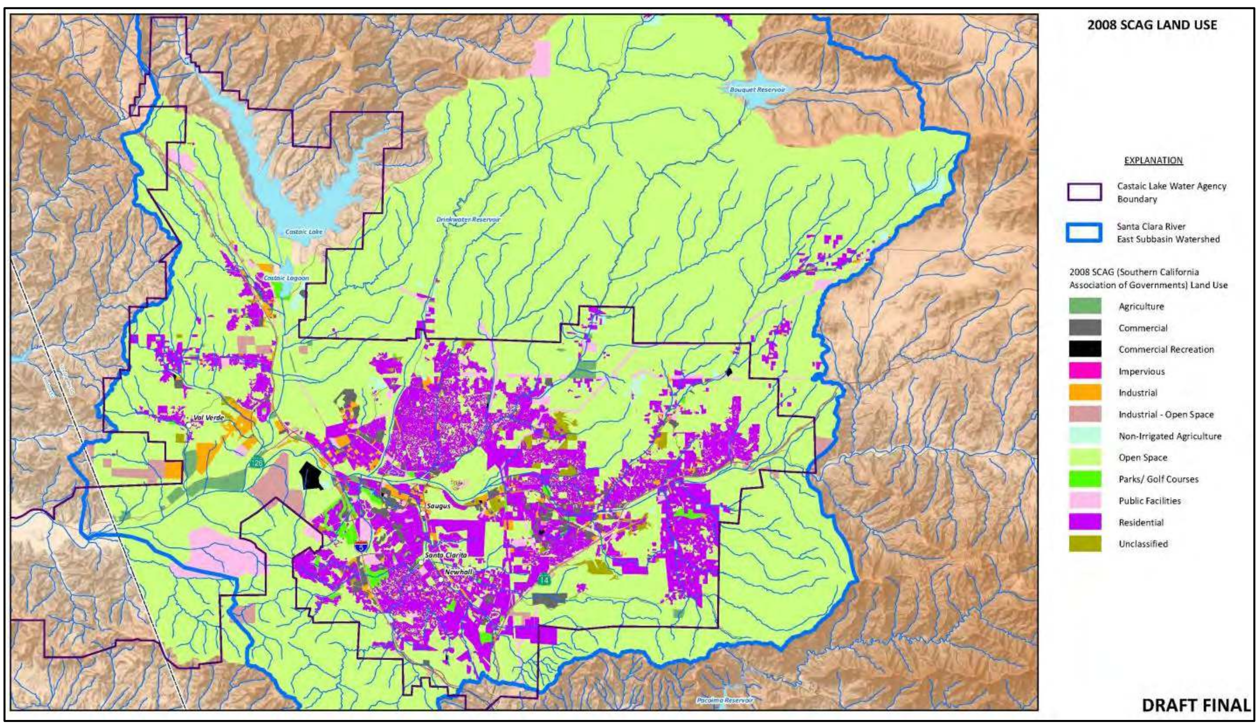
Note: red values indicate exceedance of WQOs.

2.4 Land Use

Rapid development of portions of the valley floor and canyons has occurred due to growth influences from the Los Angeles metropolitan area and the presence of three major highways (U.S. Interstate 5/the Golden State Freeway, State Highway 14/the Antelope Valley Freeway, and State Highway 126). The Santa Clarita Valley specifically now includes a variety of residential, commercial, industrial, institutional, agricultural, open space, and parks/golf course areas as shown in Figure 2-7. Although a large portion of the valley is not suitable for development due to steep terrain, flooding potential, or federal jurisdiction (Angeles National Forest), many of these existing areas allow for the utilization of recycled water through irrigation or other methods of water use.

There are also a number of future development projects underway that are seeking approval in the Santa Clarita Valley. Many of these developments intend to use recycled water to offset potable water demand and reduce waste discharge.

Figure 2-7: Land Use Map

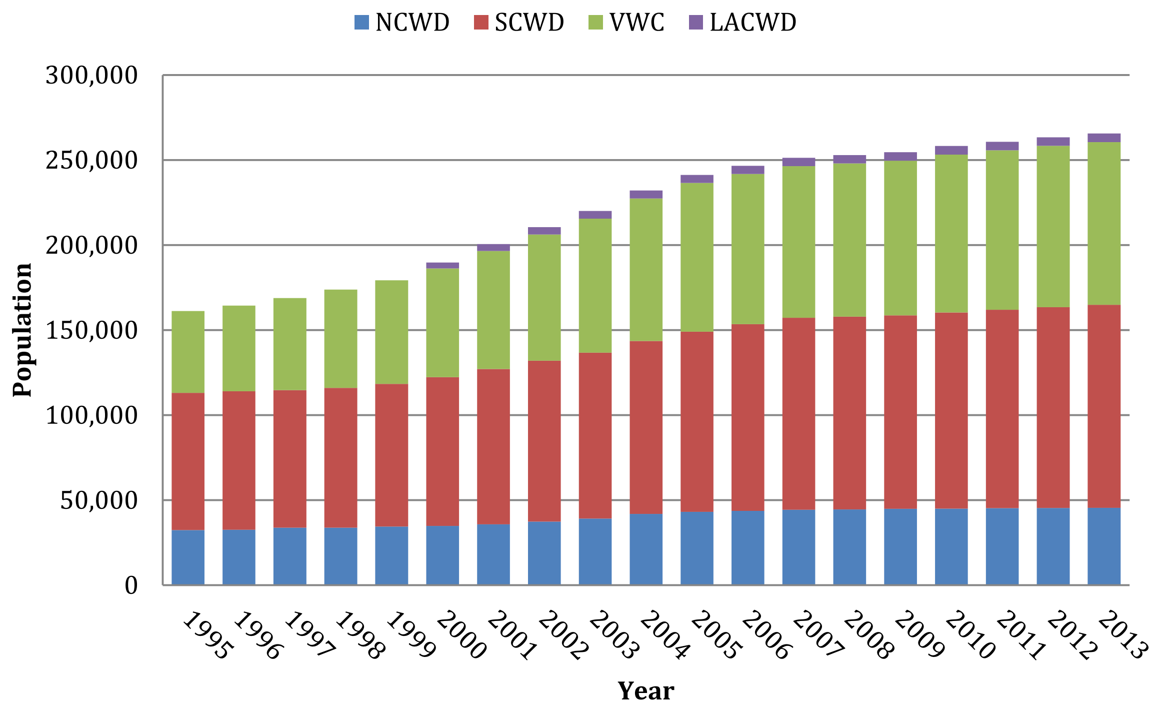


Source: SNMP (Geoscience 2015)

2.5 Population Projections

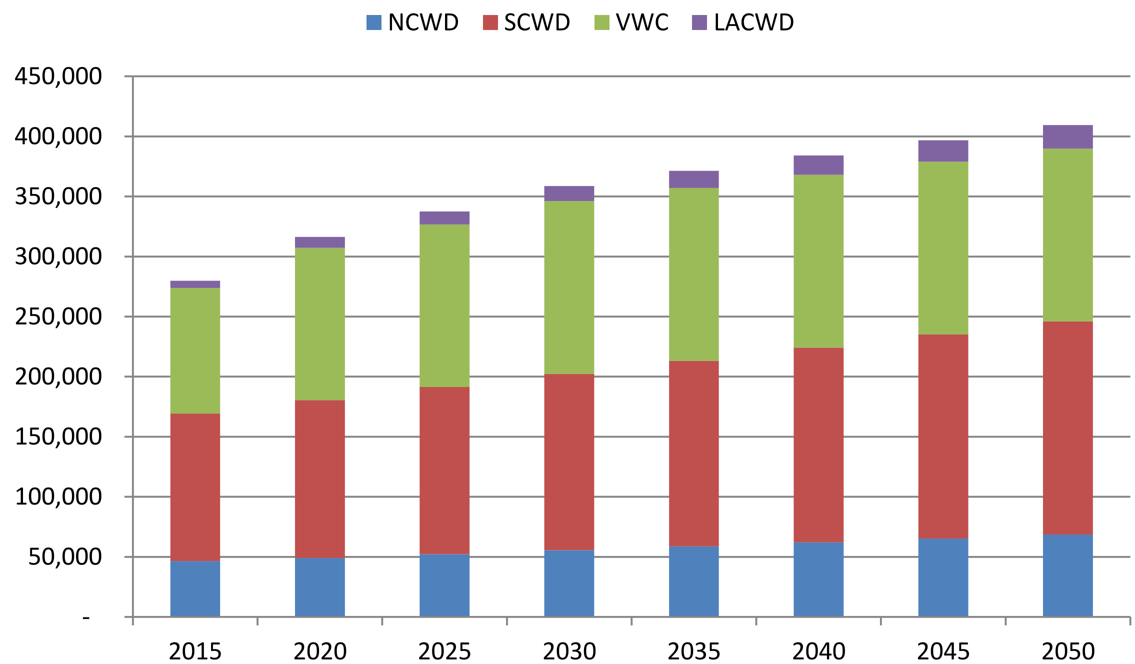
Historical population data categorized by purveyor is presented in Figure 2-8 for years 1995 through 2013 (Maddaus 2015). Most customers reside within the VWC, SCWD, and NCWD service areas and populations in Santa Clarita Valley have increased by approximately 70 percent within the last 20-year period.

Figure 2-8: Historical CLWA Population by Purveyor



CLWA recently conducted a demand projection analysis to forecast predicted population and demand increases for in Santa Clarita Valley in order to comply with the 2015 Urban Water Management Plan (UWMP) Act (Maddaus 2015). The projected population estimates from that study, presented in Figure 2-9, are based on a land use analysis supplemented by information from the purveyors on planned future developments. The population in the Santa Clarita Valley is expected to continually grow; even with increased conservation efforts water demands and wastewater flows are projected to increase over time. This subsequently increases the supply of recycled water coming from local water reclamation plants, while also increasing the demand for recycled water for a variety of uses (as discussed in the following section).

Figure 2-9: CLWA Population Projections by Purveyor



2.6 Beneficial Uses of Receiving Waters

The tertiary disinfected recycled water produced at the Valencia and Saugus WRPs is suitable for a wide variety of reuse applications. Within the recycled water service area, specific reuse applications were identified by the Water Quality Control Plan-Los Angeles Region: Basin Plan for the Coastal Watersheds of Los Angeles and Ventura Counties, including the following:

- Industrial service/process supply
- Agricultural water supply groundwater recharge
- Freshwater replenishment
- Water contact recreation
- Non-contact water recreation
- Warm freshwater and wildlife habitat
- Preservation of rare and endangered species
- Wetland habitat

In 2015, 450 AF of recycled water was utilized for landscape irrigation, with the remainder of recycled water (approximately 19.5 mgd) discharged to the Santa Clara River. Based on the Chloride Compliance Facilities Plan and associated Final Environmental Impact Report (FEIR) prepared by the SCVSD as part of the Chloride Compliance Project; 13 MGD of recycled water would be required to discharge into the River, while some or all of the remaining supply would be made available to CLWA for reasonable and beneficial non-potable use in accordance with State law and policy to maximize the use of recycled water (Kennedy/Jenks 2016a). Upon review of the FEIR, one of the March 9, 2016 rulings by the trial court stated that SCVSD's analysis did not contain sufficient detail to justify that the 13 mgd discharge amount from the WRPs would not result in a significant impact to protected species.

On March 23, 2016, the SCVSD Board recertified the 2013 EIR as augmented by the Final Supplemental EIR and approved the modified chloride compliance project. The SCVSD has also indicated that in order to avoid delays in meeting the deadline of July 2019 for the chloride compliance project to be fully operational, the recycled water reuse component is not part of the modified chloride compliance project, and that the recycled water component will be separately considered by the SCVWD Board after further environmental and public review in a separate environmental document (Kennedy/Jenks 2016a).

On June 2, 2016 the Superior Court issued a subsequent ruling that the SCVSD cannot take further action on its modified chloride compliance project until it completes the additional environmental review that the court required in its ruling dated March 9, 2016 (Kennedy/Jenks 2016a).

It is assumed that the SCVSD will be required to maintain a defined minimum discharge to the Santa Clara River to sustain the Santa Clara River biological resources (LACSD 2013). Based on discussions with the SCVSD and available information at the time of this RWMP, for the purpose of this study it is assumed that 8.5 mgd of discharge must be maintained at the Valencia WRP and 4.5 mgd at the Saugus WRP. Furthermore, it is assumed that recycled water supplies that are not required for discharge will be available for reuse within Santa Clarita Valley.

Section 3: Water Supply Characteristics and Facilities

3.1 Water Supplies and Usage

The existing water resources in CLWA's service area include local groundwater, recycled water, imported supplies, and water from existing groundwater banking programs. Local and imported water resources in the Santa Clarita Valley are managed cooperatively between CLWA and the purveyors. Table 3-1 lists available water supplies in the Santa Clarita Valley based on 2015 data reported in the UWMP (Kennedy/Jenks 2016a). Note these values indicate the supplies available in 2015, not necessarily the actual water use by source in 2015.

Table 3-1: Summary of 2015 Existing Water Resources (Average/Normal Year)

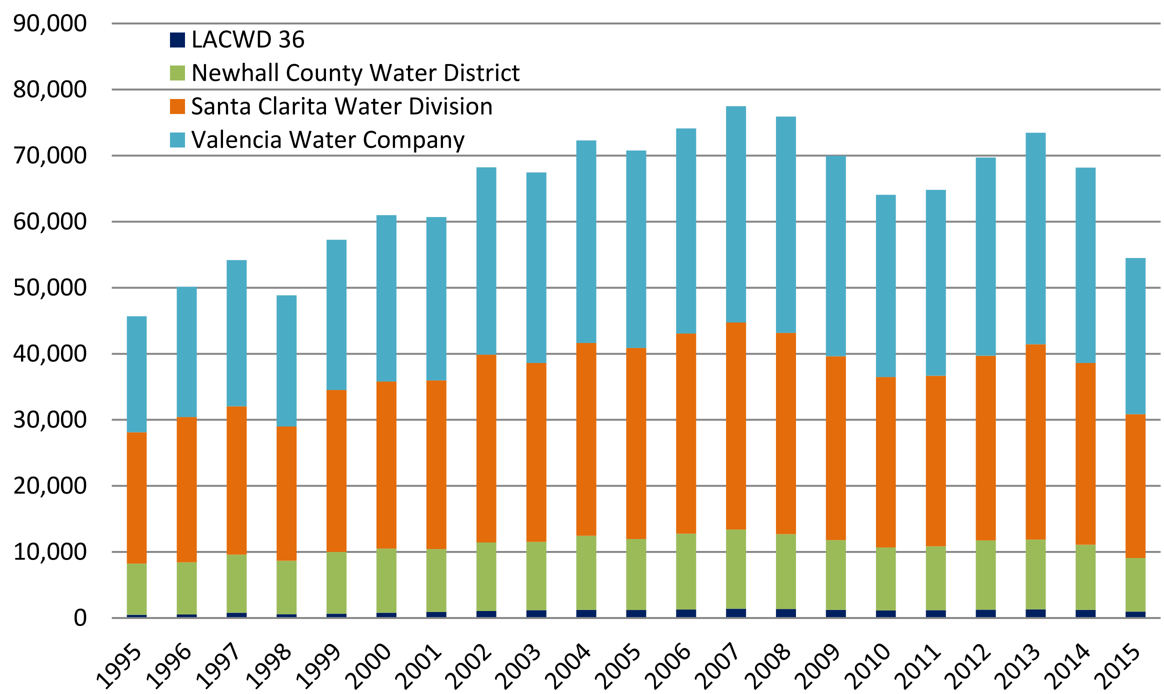
Description of Supply	Supply AF
Local Groundwater[a]	
Alluvial Aquifer	24,100
Saugus Formation	7,445
Total Groundwater	**31,545**
Recycled Water [b]	**450**
Imported Water	
State Water Project[c]	59,000
Flexible Storage Accounts[d]	6,060
Buena Vista-Rosedale	11,000
Nickel Water - Newhall Land	1,607
Yuba Accord Water	1,000
Total Imported	**78,667**
Existing Banking Programs	
Rosedale Rio-Bravo [e]	3,000
Semitropic Bank[e]	5,000
Semitropic - Newhall Land Bank[e][f]	4,950
Rosedale Rio-Bravo Exchange[g]	9,500
West Kern Exchange[g]	500
Total Banking	**22,950**
Total Existing Water Resources	**133,612**

Notes:

(a) Local groundwater represents the quantity of groundwater pumped with existing wells in average/normal years.
(b) Represents recycled water delivered in 2015.
(c) SWP supplies are based on average deliveries assumed in the Department of Water Resources "*2015 Delivery Capability Report (DCR)*."
(d) Includes both CLWA and Ventura County entities flexible storage accounts.
(e) Supplies shown are annual amounts that can be withdrawn using existing firm withdrawal capacity and would typically be used only during dry years.
(f) Existing Newhall Land supply. Assumed to be transferred to CLWA or VWC during Newhall Ranch development, with firm withdrawal capacity made available to CLWA prior to that.
(g) Supplies shown are totals recoverable under the exchange and would typically be recovered only during dry years.

The water purveyors in the Santa Clarita Valley primarily serve residential, commercial, and industrial customers. Approximately 50 percent of the M&I demand within CLWA's service area is met with imported water. VWC and SCWD are the two larger purveyors, responsible for 43 percent and 40 percent, respectively, of the total water usage in 2015. NCWD accounts for 15 percent and LACWD No. 36 accounts for the remaining 2 percent of the 2015 water usage in CLWA, as illustrated in Figure 3-1.

Figure 3-1: CLWA Historical Water Usage



Source: 2014 Santa Clarita Valley Water Report (June 2015) and 2015 data provided by each retail water purveyor.

3.2 Water Supply Reliability

The reliability of the imported supply is subject to availability, which is a function of present and past years' precipitation and snowpack, the total amount requested and used by SWP contractors and more recently regulatory cutbacks. Imported water deliveries can be curtailed during dry periods. When sufficient imported water is not available, the balance of demand is met with water previously stored in water banking programs as well as with local groundwater supplies provided by the purveyors. However, local groundwater may also be limited in some areas, highlighting the need for additional reliable sources of water to meet current and future demands under all hydrologic conditions. Implementing and expanding the recycled water system in the CLWA service area provides a reliable source of water year round that can help offset reliance on imported water and local groundwater.

3.3 Future Sources of Additional Supplies

CLWA and the purveyors recognize that recycled water is a critical component of their water supply portfolio along with new groundwater production and additional banking programs. Implementing and expanding the recycled water system in the Santa Clarita Valley provides a reliable source of water year round that can help offset reliance on imported water and local groundwater. Transfers, exchanges, and water banking are options available to CLWA for stabilizing SWP and groundwater supply. Previous evaluations of desalinated water have concluded it to be impractical or economically infeasible. Recycled water is another source of water that is available at a more constant rate throughout the year and may be banked during winter months for use in summer months. This water source adds diversity to Santa Clarita Valley's water portfolio and mitigates risk of low SWP water allocations. CLWA, NCWD, SCWD, VWC and LACWD #36 are committed to working together to increase recycled water use in the Santa Clarita Valley.

The 2015 UWMP (Kennedy/Jenks 2016a) provides additional information about the projected sources and distribution of water supplies in the Santa Clarita Valley.

Section 4: Wastewater Characteristics and Facilities

4.1 Existing Water Reclamation Facilities

The Sanitation Districts of Los Angeles County (LACSD) are a confederation of independent special districts that serve the wastewater and solid waste management needs of approximately 5.4 million people in Los Angeles County. LACSD operates ten water reclamation plants (WRPs) and one ocean discharge facility (Joint Water Pollution Control Plant), which treats approximately 394 million gallons per day (mgd); 135 mgd of which are available for reuse.

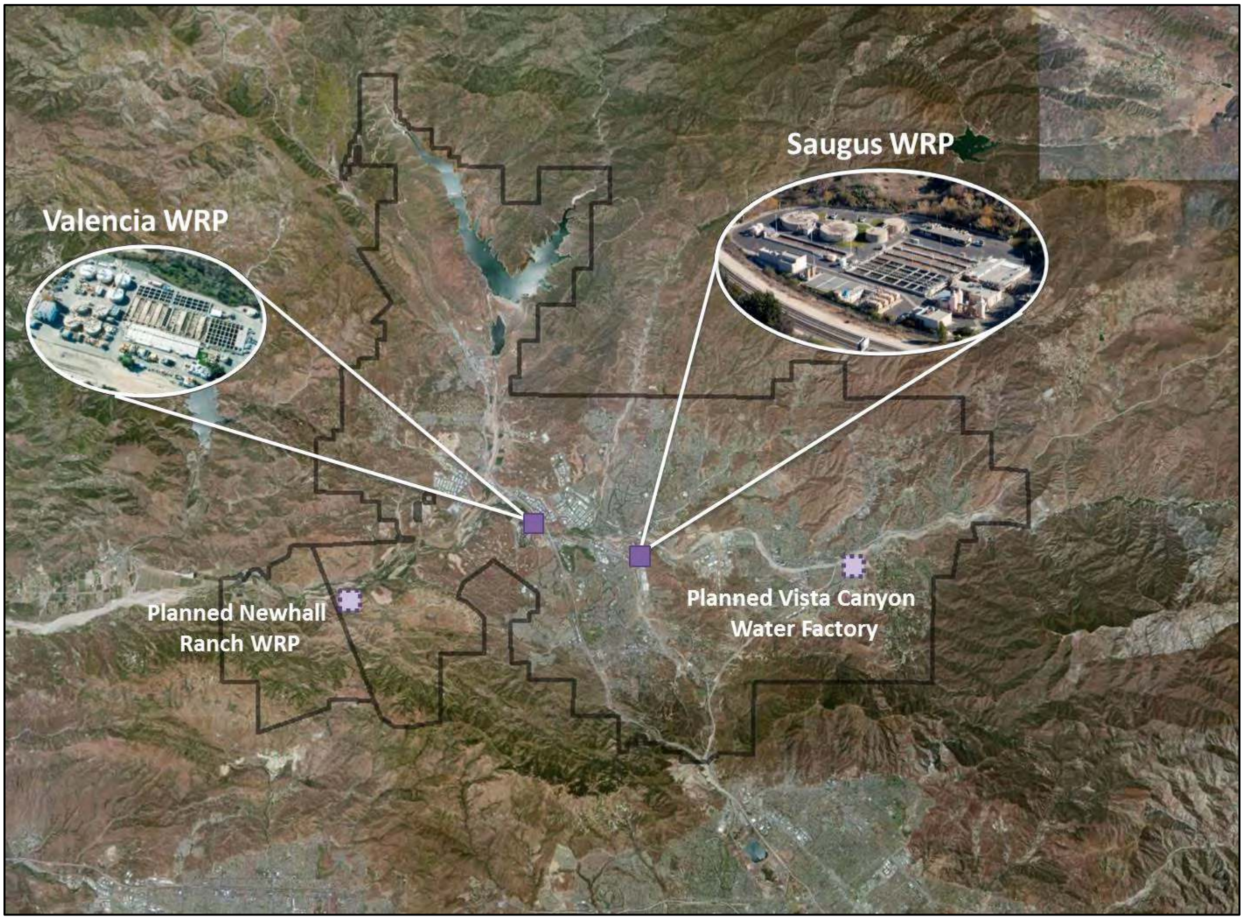
The Santa Clarita Valley Sanitation District of Los Angeles County (SCVSD) was formed through a consolidation of Sanitation District Nos. 26 and 32 to provide wastewater management services to the Santa Clarita Valley. The SCVSD operates two WRPs within the CLWA service area: 1) Saugus WRP and 2) Valencia WRP, as shown on Figure 4-1. The primary sources of wastewater to the Saugus and Valencia WRPs are residential and commercial flows. The two plants produce high-quality tertiary disinfected recycled water, which is distributed for non-potable reuse or discharged into the upper reaches of the Santa Clara River (under NPDES Order No. R4-2005-0031 and Order No. R4-2005-0032 respectively).

- The **Valencia WRP**, completed in 1967, is located on The Old Road near Six Flags Magic Mountain Amusement Park. The Valencia WRP has a current treatment capacity of 21.6 mgd, developed over time in stages. In 2015, the Valencia WRP produced an average of 13.3 mgd of tertiary recycled water. Use of recycled water from the Valencia WRP is permitted under Los Angeles RWQCB Order Nos. 87-48.

- The **Saugus WRP,** completed in 1962, is located southeast of the intersection of Bouquet Canyon Road and Soledad Canyon Road. The Saugus WRP has a current maximum treatment capacity of 6.5 mgd and no future expansions are possible at the plant due to space limitations at the site. In 2015, the Saugus WRP produced an average of 5.1 mgd of tertiary recycled water. Use of recycled water from this facility is permitted under Los Angeles RWQCB Order Nos. 87-49.

The Saugus and Valencia WRPs operated independently until 1980, at which time the two plants were linked by a bypass interceptor. The interceptor was installed to transfer a portion of flows received at the Saugus WRP to the Valencia WRP. Together, the Valencia and Saugus WRPs have a design capacity of 28.1 mgd.

Figure 4-1: Existing and Proposed Water Reclamation Plants



4.2 Planned Water Reclamation Facilities

Planned future developments in the Santa Clarita Valley, such as the Westside Communities and Vista Canyon developments, are also planning to construct water reclamation facilities to produce tertiary recycled water suitable for non-potable reuse to offset potable demands (Figure 4-1). Excess recycled water from these sources may be incorporated into the CLWA recycled water system or served directly to Santa Clarita Valley customers in the future.

Vista Canyon Water Factory

The proposed Vista Canyon Water Factory would be located near Highway 14, just south of the Santa Clara River. The Vista Canyon Water Factory would be constructed as a "turn-key" facility. At this time it is assumed that this facility would be owned and operated by the City of Santa Clarita. The facility is designed to be a scalping plant with no solids treated on-site and waste activated sludge treatment at the SCVSD's existing facilities downstream. The treatment process begins with pumping to the plant, screening, flow equalization, extended aeration activated sludge, disc filtration, and UV disinfection (Dexter Wilson 2015a).

The Vista Canyon Water Factory is anticipated to come online in 2017 and is projected to treat an average flow of 392,000 gpd (approximately 440 AFY) of wastewater, consisting of flows from Vista Canyon (approximately 214,000 gpd) and wastewater extracted from LACSD's sewer line. Solids generated would be discharged to the existing sewer and treated at the Valencia WRP. Title 22 tertiary disinfected recycled water would be produced at full design capacity from the start (392,000 gpd or 440 AFY), taking wastewater from an existing sewer interceptor that serves existing development upstream of the project site (Impact Sciences 2010).

Newhall Ranch WRP

The proposed Newhall Ranch WRP would be located near the western edge of the development project along the south side of State Route 126. The Newhall Ranch WRP would serve the Newhall Ranch Specific Plan and a new County Sanitation District has been created to operate and maintain the Newhall Ranch WRP. The Newhall Ranch WRP is anticipated to produce 3.75 mgd (4,200 AFY) of recycled water, which would be available to meet a portion of the 7,100 AFY of the non-potable demands anticipated for the development (Newhall Ranch, Entrada South, Entrada North, Legacy Village, and the buildout of Valencia Commerce Center) at buildout (GSI 2015). Recycled water from the Valencia WRP would be used to meet the remainder of the non-potable demands.

4.3 Discharge Requirements

Historically, the effluent from the two WRPs has been discharged to the Santa Clara River. The Saugus WRP effluent outfall is located approximately 400 feet downstream (west) of Bouquet Canyon Road. Effluent from the Valencia WRP is discharged to the Santa Clara River at a point approximately 2,000 feet downstream (west) of The Old Road Bridge.

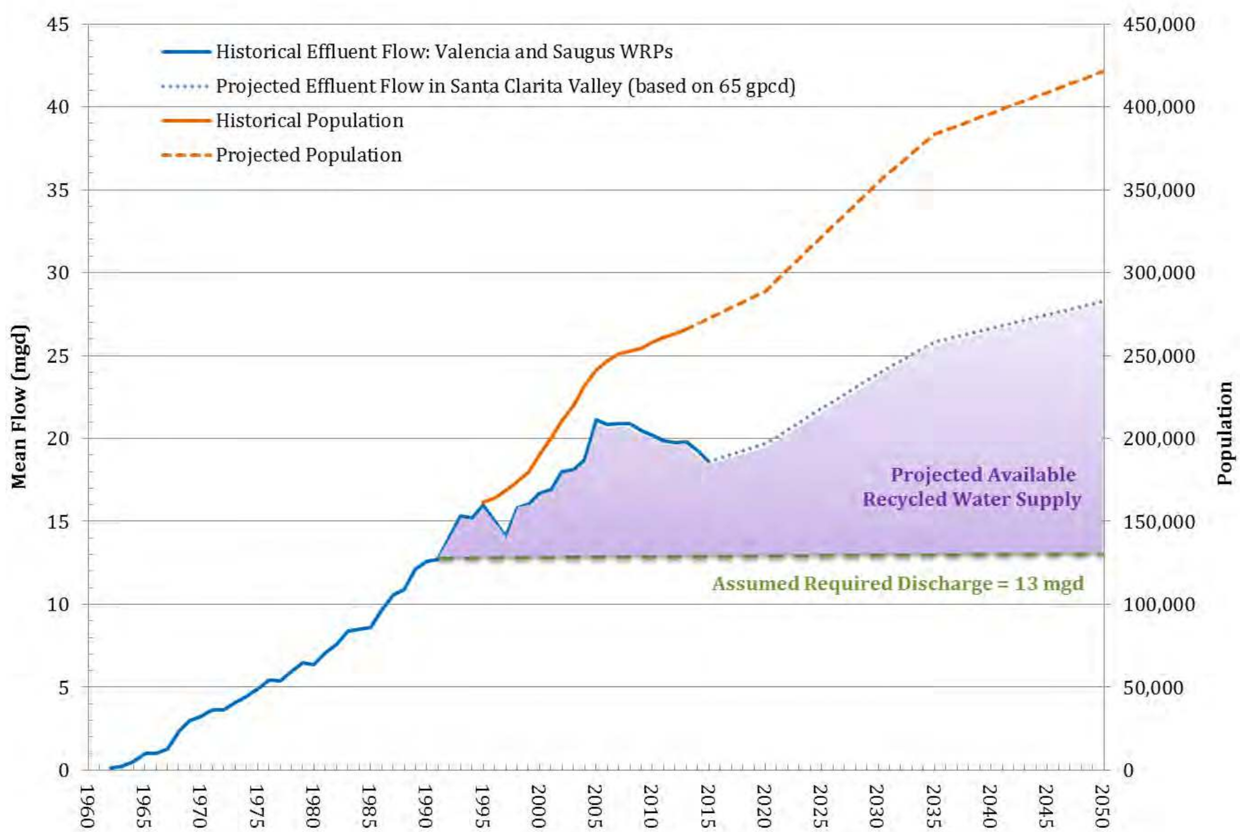
As discussed in Section 2.6, it is assumed that the SCVSD will be required to maintain a minimum discharge to the Santa Clara River to sustain biological resources (LACSD 2013). The Superior Court ruling on June 2, 2016 states that the SCVSD cannot take further action on its modified chloride compliance project until it completes the additional environmental review that the court required in its ruling dated March 9, 2016 (Kennedy/Jenks 2016a). Based on discussions with the SCVSD and available information at the time of this RWMP, for the purpose of this study it is assumed that 8.5 mgd of discharge must be maintained at the Valencia WRP and 4.5 mgd at the Saugus WRP.. Furthermore, it is assumed that recycled water supplies that are not required for discharge will be available for reuse within Santa Clarita Valley.

4.4 Projected Supply of Recycled Water

The future production of recycled water is estimated based on the projected influent wastewater flow into water reclamation facilities using a generation factor multiplied by the net projected population increase within the CLWA service area (discussed in Section 2.5). A generation rate of 65 gallons per capita daily (gpcd) was recommended by SCVSD based on observed gpcd rates for the last 5 years. The projected supply of recycled water is calculated as the production minus the anticipated required discharge (Section 4.3). Figure 4-2 illustrates the total projected supply of recycled water in the Santa Clarita Valley that could be available for reuse from the existing and proposed WRPs. Appendix A, Table A-1 presents the annual flow calculations.

One limitation to utilizing all of the available annual recycled water produced is the seasonal irrigation demand for recycled water, which peaks in summer months and is low in the winter and shoulder months. In addition, the Saugus WRP has limited flow available after meeting the anticipated discharge requirement, which makes it a less reliable source for recycled water use.

Figure 4-2: Projected Available Recycled Water Supply



4.5 Recycled Water Rules and Regulations

The regulations governing the wholesale use of recycled water from the Valencia and Saugus WRPs are set forth in the *Joint Outfall System and Santa Clarita Valley Sanitation District - Recycled Water Users Handbook* prepared by the Sanitation Districts of Los Angeles County (LACSD) – July 1, 2008 (herein referred to as Sanitation Districts' Handbook[2]), which describe the rules and regulations for the safe use of tertiary recycled water in compliance with applicable Federal, State and local statutes, ordinances, regulations, orders and other requirements.

[2] http://www.lacsd.org/waterreuse/recycledresources.asp

As the producer of recycled water, the SCVSD oversees the production and use of recycled water pursuant to permits issued by the RWQCB. The water reclamation requirements for the Saugus and Valencia WRPs are described in the Los Angeles RWQCB Orders No. 87-49 and 87-48.

In 2007, an ordinance was enacted that provides the Sanitation Districts with enforcement powers over the use of recycled water in the Santa Clarita Valley. This ordinance, known as the *Santa Clarita Valley Sanitation District Recycled Water Ordinance*[3], applies to wholesalers, purveyors and users in the SCVSD receiving recycled water directly or through an intermediate party, including purveyors. Authorized sites must file an application and execute a User Agreement with the SCVSD, or through the purveyor.

The water purveyors in Santa Clarita Valley may retail recycled water purchased through CLWA to water customers in their service area. VWC has been serving recycled water through the existing system for the last decade. SCWD, VWC and NCWD are currently working collaboratively with CLWA and each other to expand the existing system to reach more of their customers. The Sanitation Districts' Guidelines also include regulations governing retail provision of recycled water.

The regulations governing the wholesale use of recycled water from water reclamation facilities not owned and operated by the Sanitation Districts, such as the planned Vista Canyon and Newhall Ranch WRPs, have not been formally adopted at this time. It is anticipated that the requirements would be similar to those established in the Sanitation Districts Guidelines.

Existing agreements that guide the sale and use of recycled water are summarized in the *Santa Clarita Valley Recycled Water Rules and Regulations Handbook* (herein referred to as the SCV Rules and Regs Handbook) (Kennedy/Jenks 2016b).

4.6 Rights to Recycled Water

A determination of rights to treated wastewater is required prior to long-term project expenditures. Ownership of the rights to wastewater is addressed in three separate state laws or codes:

- Clean Water and Water Bond Law of 1978
- California Department of Fish and Game Code, Section 1600
- Water Code, Sections 1210, 1211, and 1702

The Clean Water and Water Bond Law of 1978 established that treated wastewater was the property of the treatment facility that produced it and that the producer could sell or transfer its rights. In addition, the rights of the treatment facility allowed the treated wastewater to be used for beneficial purposes regardless of the detriment to downstream users. However, the advice of legal

[3] "Ordinance Providing for the Establishment and Enforcement of Regulations Pursuant to Water Recycling Requirements for Recycled Water Users" February 2007. DMS - #781170

counsel for individual determinations and the development of the most equitable and least detrimental projects for all affected parties are recommended.

The California Department of Fish and Wildlife Code Section 1600 requires that "any project which will divert, obstruct or change the natural flow or bed, channel or bank of any river, stream or lake designated by the department in which there is at any time an existing fish or wildlife resource or from which these resources derive benefit" be modified sufficiently "for the protection and continuance of the fish or wildlife resources." On the Santa Clara River, there are users of river water downstream of both Saugus and Valencia WRPs, as well as Significant Environmental Areas that support endangered species. Potential impacts to these users and the habitat should be addressed in the environmental documents to be prepared for this proposed recycled water project.

Water Code section 1210 states that between the owner of the wastewater treatment plant and the entities contributing the wastewater into the collection system, the owner of the treatment plant has exclusive rights to the treated wastewater. This does not mean that the treatment plant owner has exclusive rights to effluent. Water rights may accrue after discharge. The discharged water may also support instream or riparian habitat. Therefore, downstream water rights or environmental conditions may supersede the rights of the owner of the treatment plant to the use of the treated effluent. In terms of the rights to treated wastewater, DDW would determine to what extent treated wastewater would need to remain instream to satisfy downstream water rights. In general, "if the water is imported from another watershed (for example), the legal user of water may be considered to have recaptured the water rather than abandoning it to the stream system. In such a case the local water users may not be able to claim interference with their water rights for projects involving re-use of foreign water"[4].

Water Code section 1211 requires the SWRCB to review a proposed change in point of discharge, place of use, or purpose of use of treated wastewater in the same manner as the SWRCB would review a proposed change to an appropriative water right. As both sections 1210 and 1211 make clear, however, the Legislature did not intend to affect any rights that downstream users may have to the treated wastewater discharge under the common law. Therefore, Water Code section 1702 provides that before granting permission to make a change, the SWRCB must find "that the change will not operate to the injury of any legal user of the water involved." The statutory "no injury" rule set forth in Water Code section 1702 codifies that common law no injury rule and therefore should be interpreted consistently with case law that interprets and applies the common law rule. Generally, the common law no injury rule precludes a change in the exercise of a water right if, among other things, the change would alter the pattern or rate of return flow to the detriment of downstream water right holders (*Scott v. Fruit Growers' Supply Co, 1972).*

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[4] Source: DDW website related to Wastewater Change Petition
http://www.swrcb.ca.gov/waterrights/water_issues/programs/applications/wastewaterchange/index.shtml

The first contract between SCVSD and CLWA for the use of recycled water was executed in 1996 and has since been amended to provide for temporary allotment increases to support construction activities. The existing contract (CSD Contract #3425 signed on July 24, 1996) is the basis for wholesaling recycled water in Santa Clarita Valley and makes 1,600 AFY of recycled water from the Valencia WRP available to CLWA for purchase. Contract #3118266 (signed on Oct 20, 2014) and Contract #3322936 (signed on July 23, 2015) served to temporarily increase the allotment for fiscal year 2014/15 and 2015/16, respectively, to 2,200 AFY. This increase was attributed to the need for recycled water to be used for dust control for Westside Communities development construction activities. Future contracts, allotment increases and/or amendments to the wholesaling contract with the SCVSD should be approved prior to the expansion of the recycled water system beyond 1,600 AFY.

4.7 Recycled Water Quality

Effluent quality from the Valencia and Saugus WRPs is regulated by the RWQCB. Discharge permits specifying the wastewater quality requirements for effluent discharged to the Santa Clara River have been issued for each plant. Each plant also has a reclamation permit specifying recycled water quality requirements. The quality of effluent from the Valencia and Saugus WRPs has consistently been in compliance with the recycled water requirements specified in their reclamation permits.

Depending on the place and purpose of the recycled water use, the necessary treatment processes and the maximum allowable concentrations vary. These variations are addressed in the reclamation permits and recycled water uses are limited to those identified in the permits. The tertiary-treated wastewater from the Valencia and Saugus WRPs is "adequately disinfected, oxidized, coagulated, clarified, filtered wastewater" as specified for use of recycled water in non-restricted recreational impoundments, the use subject to the most stringent requirements in the permits.

Average concentrations of effluent constituents measured from 2012-2014 for each plant and associated regulatory requirements are provided in Appendix C, Table 3. Related regulatory requirements are also listed including: (1) all Maximum Contaminant Levels (MCL) in the Title 22 California Code of Regulations, (2) Limits described in the SNMP (discussed in Section 0) for several constituents including chloride, total dissolved salts, and sulfate and (3) the Santa Clarita Valley east groundwater basin objectives. SCVSD is currently developing advanced water treatment facilities at the VWRP to meet compliance with the USCR chloride TMDL. The advanced water treatment facilities are scheduled to be in operation in July 2019. Advanced treated water, which is low in chloride and other constituents, may be available for reuse, should SCVSD have excess treatment capacity not needed for compliance.

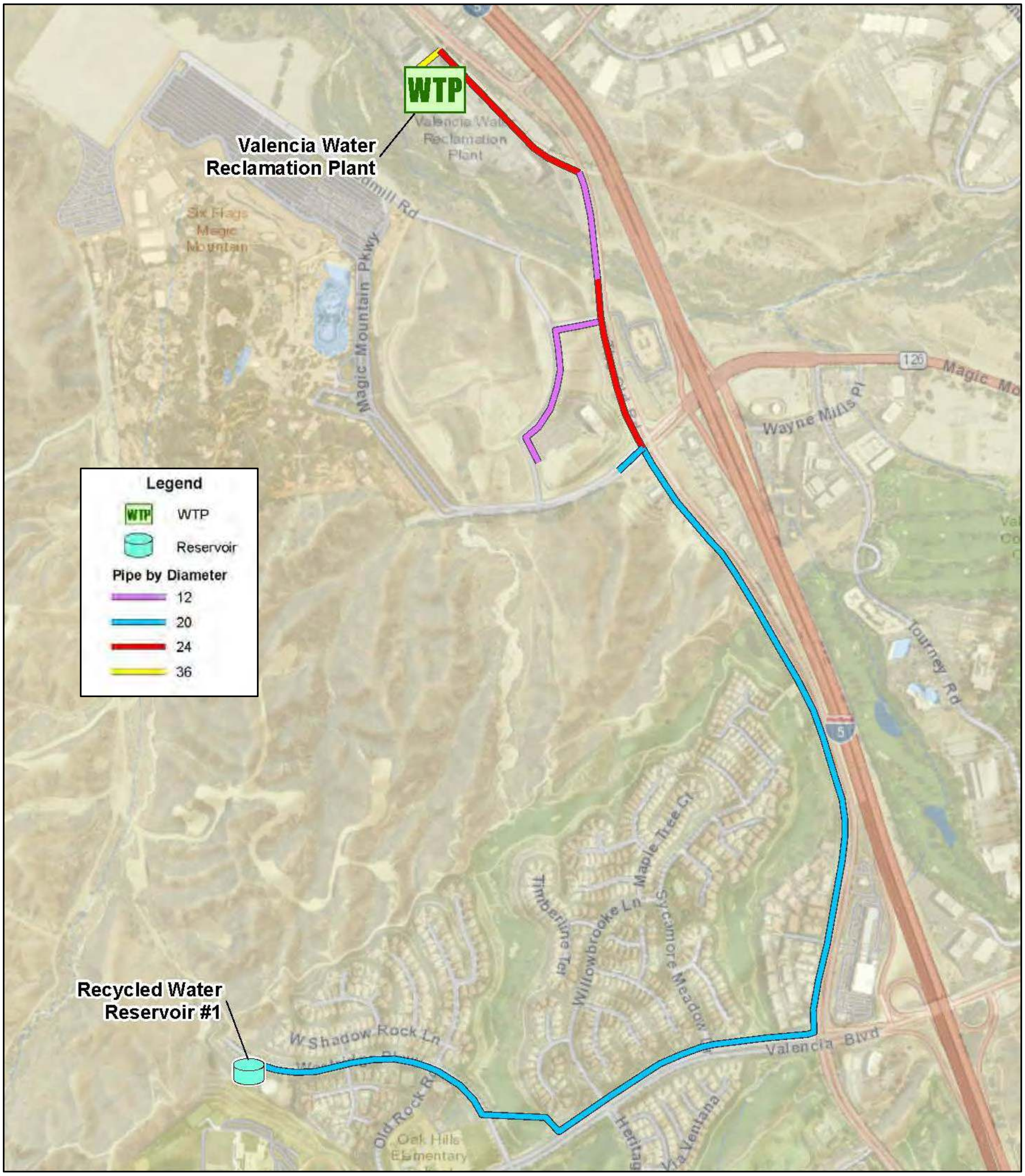
4.8 Existing Recycled Water System

CLWA currently serves recycled water to VWC through the Recycled Water System Phase 1 facilities which include: a Recycled Water Pump Station at the Valencia WRP; a 1.5 million gallon Recycled Water Tank in the Westridge development; and approximately 15,600 feet of recycled water pipelines ranging in diameter from 12-inches to 36-inches, as shown in Figure 4-3. Average annual recycled water usage has averaged 415 acre-feet per year (AFY) for the last 10 years. In 2015, 450 AFY of recycled water was used. Appendix A, Table A-3 lists historical monthly recycled water deliveries through the existing system. Ninety percent of water use is between May and October, during the irrigation season.

The existing recycled water system was modeled and calibrated using meter data provided by CLWA. The model and calibration results are described in Appendix D. Initial results identified the following deficiencies:

1. The 12-inch pipeline across the bridge in The Old Road has a velocity reaching as high as 5.4 fps, which is acceptable for the current demands but will become higher as demands increase
2. The pressures near the Recycled Water Tank are low and it may be difficult to serve new customers in this area using the existing storage tank.

Figure 4-3: Existing Recycled Water System Configuration



Section 5: Treatment Requirements

5.1 Recycled Water Regulations

The production, discharge, distribution, and use of recycled water are subject to federal, state, and local regulations; the primary objectives of which are to protect public health. Regulatory requirements apply for non-potable and potable uses of recycled water.

- **Non-potable reuse** refers to the use of tertiary treated municipal wastewater for specific purposes other than drinking; such as landscape irrigation, industrial uses, and agriculture or for environmental benefits. Non-potable reuse usually requires an independent "purple pipe" distribution system for conveying recycled water to customers separate from the potable supply. In California, non-potable reuse is ongoing throughout the nation for the last century and regulations for non-potable reuse have been in place since the 1970s.

- **Potable reuse** refers to the intended use of highly treated or purified municipal wastewater to augment a water supply that is used for drinking and all other purposes. Unplanned potable reuse, where one community draws raw water supplies downstream from discharges from wastewater treatment plants, is regulated by federal discharge requirements. Planned potable reuse involves a more formal public process and regulatory consultation program to implement and the regulations in California for the indirect and direct use of recycled water are at varying stages of development.

    - **Indirect potable reuse (IPR)** is the purposeful introduction of tertiary treated recycled water or highly purified recycled water into an untreated drinking water supply source, such as groundwater in an aquifer or surface water in a large reservoir. The recycled water may require blending with a diluent water, at a specified blending ratio for groundwater replenishment, and purified water must be added to a specified volume of surface water during reservoir augmentation. A minimum of 6 to 12 months travel time between the point of addition and eventual extraction is clearly specified for groundwater replenishment with recycled water. In addition, reservoir augmentation requires retreatment at a drinking water treatment plant. Regulations for groundwater replenishment using recycled water became effective on June 18, 2014 and the adoption of water recycling criteria for surface water reservoir augmentation are anticipated by December 31, 2016.

    - **Direct potable reuse (DPR)** is the purposeful introduction of highly purified recycled water into a drinking water supply; immediately upstream of a drinking water treatment plant or directly into the potable water supply distribution system downstream of a water treatment plant. Currently, DPR is not yet included as an allowable use in California, though a report on the feasibility of developing uniform water recycling criteria for direct potable reuse is anticipated by December 31, 2016.

Meeting regulatory requirements is an integral part of implementing any non-potable or potable recycled water project. Appendix B summarizes the regulatory requirements and their administration, with an emphasis on regulations relating distribution and use of recycled water in California. Appendix C provides additional details about current and anticipated regulatory requirements for potable reuse.

Use of recycled water from the Valencia and Saugus WRPs is permitted under Los Angeles RWQCB Order Nos. 87-48 and 87-49, respectively. Copies of these recycled water permits along with SCVSD Ordinances and Requirements for Recycled Water Users in Santa Clarita Valley and Los Angeles County Department of Public Health (LACDPH) guidelines and inspection requirements are provided in the Santa Clarita Valley Rules and Regs Handbook (Kennedy/Jenks 2016b).

5.2 Non-Potable Customer Requirements

Recycled water quality requirements for a given project depend on the regulatory requirements, which set a minimum standard plus any additional customer requirements for the end uses. For example, though removal of total dissolved solids (TDS, a measure of salinity) is not required for recycled water by regulations, it may be desirable depending on the end use and the concentration of TDS in the source water.

Irrigation Requirements

Table 5-1 provides a summary of broadly accepted general water quality guidelines available for use of recycled water for landscape and agricultural irrigation. These guidelines are not plant-specific and therefore may be too restrictive for some plants and not restrictive enough for more sensitive plants. However, these guidelines are considered to be conservative (Tchobanoglous et al. 2004; Ayers and Westcot 1985; Tanji et al. 2007).

Table 5-1: Recycled Water Quality Guidelines for Irrigation

Constituent or Parameter	Issue of Concern	Units	Degree of Restriction on Use[(a)]			Valencia WRP Effluent
			None	Slight to Moderate	Severe	
Boron	Toxicity to Plants	mg/L	< 0.7	0.7 to 3.0	> 3.0	0.53
Chloride	Ion toxicity, Spray (Overhead) Irrigation	mg/L	< 100	>100		126
	Surface irrigation	mg/L	< 140	140 to 350	> 350	
pH	Misc. Effects	--	Normal Range 6.5 to 8.4			7.43
Residual chlorine	Leaf Burn from Spray (Overhead) Irrigation	mg/L	< 1	1 to 5	> 5	3
Salinity as TDS	Plant Response	mg/L	< 450	450 to 2,000	>2,000	690

Notes: TDS = total dissolved solids;
Source: Water quality objectives from 2004 *Wastewater Engineering: Treatment and Reuse* (Tchobanoglous et al.), based on Ayers and Westcot (1985) with additional information from Tanji et al. (2007).
(a) *None* – Suitable water quality as is; *Slight to Moderate* – Manageable with proper irrigation scheduling, amendments, and/or plant selection; *Severe* – Problematic, may need partial removal of the constituent.

Tertiary disinfected recycled water produced at the Valencia WRP has been used by CLWA for irrigation for the past decade and is assumed to be suitable for irrigation without further treatment. Irrigation Management strategies that can address some common irrigation issues, should they arise, include: 1) applying excess water to maintain salt balance in the root zone (flush salts), 2) maintaining adequate soil drainage, 3) avoiding spray wetting of salt-sensitive plant foliage, 4) adding water and soil amendments, and 5) using salt-tolerate plants in landscaping.

Non-Irrigation Requirements

Non-irrigation uses, such as toilet and urinal flushing and cooling towers that are dual-plumbed with an internal purple pipe system to separate potable water from recycled water (non-potable) may have water quality objectives beyond meeting Title 22 objectives.

For aesthetic reasons, it is preferable that recycled water used for toilet and urinal flushing is odorless and colorless. This is generally recommended by professionals in the water reuse industry. Organic and inorganic compounds in recycled water can cause discoloration and odor. Oxidizing agents such as chlorine, ozone, and hydrogen peroxide can be used for removal of color and odor, and UV light may also contribute to the removal of color. Hydrogen peroxide (H_2O_2) is an oxidant commonly used in water treatment and wastewater reclamation, including for eliminating color and odor, though it is less effective than ozone but easier to implement. Chlorine is less effective for odor and color removal compared to ozone and hydrogen peroxide and so is not specifically used for this purpose.

Cooling towers prefer receiving a water source with a consistent water quality to achieve specific water quality requirements that align with operational and maintenance practices. Variable water quality can be a challenge as it impacts the number of cycles and chemical requirements; additionally, ammonia concentration is of greatest concern due to the potential for corrosion. Removal of salinity and ammonia may be desirable to meet cooling tower water quality objectives. It is not uncommon for cooling towers to have small package RO plants to manage water quality from potable water sources. Thus, if cooling towers are selected as a future customer it would be important to work closely with their operators to understand their current practices and needs.

5.3 Potable Reuse Requirements

Appendix C – Potable Reuse Technology Assessment provides a detailed assessment of treatment requirements and potential treatment trains to meet existing and anticipated regulations for (1) groundwater replenishment (surface spreading and direct injection), (2) surface water augmentation (at Castaic Lake), and (3) direct potable reuse. A summary is provided herein.

Groundwater Replenishment Treatment Requirements

Groundwater replenishment requirements are described in terms of (1) surface spreading and (2) direct injection. Both of these groundwater replenishment options are governed by the Groundwater Replenishment Reuse (GRR) Regulations, which were promulgated on June 18, 2014. Table 5-2 summarizes the GRR Regulations for spreading and direct injection.

Table 5-2: Summary of Groundwater Replenishment Reuse Regulations

Water Quality Limits for Recycled Water	Treatment and Diluent Requirements
≥ 12-log virus reduction ≥ 10-log *Giardia* cyst reduction ≥ 10-log *Cryptosporidium* oocyst reduction Drinking water MCLs (except for nitrogen) ≤ 10 mg/L total nitrogen Action levels for lead and copper TOC ≤ 0.5/RWC	**<u>Surface Spreading with Tertiary and Diluent Water</u>** - Oxidation, Filtration, Disinfection, Soil Aquifer Treatment - Diluent Water (based on TOC of recycled water) **<u>Surface Spreading with FAT*</u>** - Oxidation, Reverse Osmosis (RO), Advanced Oxidation Process (AOP) - Diluent Water (based on TOC of recycled water) **<u>Direct Injection with FAT*</u>** - Oxidation, RO, AOP - No Diluent water required

Other Selected Requirements
• Treatment train shall consist of at least 3 separate treatment processes to achieve the pathogenic (microorganism) control • For each pathogen (i.e., virus, *Giardia*, or *Cryptosporidium (V/G/C)*), a separate treatment process may be credited with no more than 6-log reduction, with at least 3 processes each being credited with no less than 1.0-log reduction • ≥ 2-month retention (response) time underground • Initial maximum RWC ≤ 20% for spreading tertiary treated water (depending on TOC of recycled water) or up to 100% for Injection with FAT. Over time the RWC can be increased if certain requirements are met. • For spreading, or Injection with FAT, 1-log virus reduction credit automatically given per month of subsurface retention • For spreading, 10-log *Giardia* reduction and 10-log *Cryptosporidium* reduction credit given to disinfected tertiary effluents with at least 6 months retention time underground

Notes: MCL = maximum contaminant level, TOC = Total Organic Carbon
RWC = recycled water contribution (the quantity of recycled water applied at the recharge site divided by the sum of the quantity of recycled water applied at the site and diluent water)
FAT = Full Advanced Treatment
* The treatment technologies listed do not include the full range of advanced treatment processes available to achieve FAT (i.e. Microfiltration (MF), ozone, decarbonation, etc.). Also, an alternative treatment approach to meeting the GRR Regulations may be approved if the project can demonstrate to DDW that the proposed alternative can reliably meet all water quality objectives and assures at least the same level of protection of public health.

Surface Spreading Treatment Requirements

In **surface spreading**, recycled water is discharged into spreading basins, where it percolates through the vadose (unsaturated) zone until it joins native groundwater and travels horizontally (saturated zone) towards extraction wells. Physical (filtration), chemical, and biological processes treat water through the vadose and saturated zones. This geopurification system is known as soil aquifer treatment (SAT). Per the GRR Regulations, the wastewater needs to be treated to meet the criteria for Title-22 RW unrestricted use (e.g. disinfected tertiary recycled water). Implementation of any surface spreading project requires blending recycled water with a diluent water such as surface water, stormwater, native groundwater or imported water. The potential sources of diluent water are discussed in Section 6.

Both Valencia and Saugus WRPs have the appropriate level of treatment to meet the GRR Regulations for surface spreading and further treatment is therefore not explicitly required. However, the inclusion of additional treatment could be required to meet specific regulatory limits or to allow more water to be spread, as discussed further in Appendix C.

The SCVSD, as part of their chloride compliance project, is currently designing an Advanced Water Treatment Facility (AWTF) at the Valencia WRP that includes membrane filtration (MF), enhanced brine concentration (EBC), reverse osmosis (RO), and ultraviolet (UV) light for disinfection. The EBC process is designed to pretreat the water prior to RO to reduce certain target constituents that commonly foul RO membranes including calcium, magnesium, and other salts while allowing chloride to pass through to be removed by the RO. The EBC process consists of nanofiltration (NF), ion exchange (IX) and pH control. The brine from the reverse osmosis process will be trucked to the LACSD's Joint Water Pollution Control Plant in Carson for disposal. SCVSD intends to blend tertiary recycled water with the advanced treated water (herein referred to as "Valencia Blend") to meet the chloride requirements for river discharge. When there is excess capacity in the AWTF, the Valencia Blend water may potentially be used for groundwater recharge. Based on discussions with SCVSD, for the purpose of this evaluation, it is assumed that up to 5,000 AFY of Valencia Blend water may potentially be available to CLWA (at a higher cost than the tertiary recycled water) for surface spreading. The potential quantity of Valencia Blend water for a surface spreading project is discussed in the alternative analysis in Section 7.

Direct Injection Treatment Requirements

In **direct injection**, recycled water that has gone through a full advanced treatment (FAT) process is directly injected into the saturated groundwater zone, bypassing SAT. The implementation of FAT (i.e. MF, RO and an advanced oxidation process (AOP)) allows for the use of up to 100% recycled water (e.g. no dilution requirement) and as little as a 2-month minimum retention time, if the 12-10-10 microbial requirements are met.

The GRR Regulations have specific requirements for the RO and AOP technologies in the FAT train. The RO membranes must achieve a minimum and average sodium chloride rejection of 99.0% and 99.2%, respectively. The initial RO permeate TOC must be less than 0.25 mg/L and not exceed 0.5 mg/L over the long term, based on a 20-week running average of all TOC results and the average of the last four TOC results. Any advanced treatment train constructed as part of a direct injection GRR project will undergo the same set of challenges regarding brine disposal as those faced by SCVSD. As a result, a modified version of SCVSD's AWTF process is assumed for implementation of a direct injection project (as described in Appendix C).

Alternative treatment process trains are considered by the DDW if all water quality objectives can be reliably met and comparable protection of public health can be proven.

Surface Water Augmentation Treatment Requirements

A **surface water augmentation** (SWA) project is defined as a project that plans to use recycled municipal wastewater for the purpose of augmenting a reservoir that is designated as a source of domestic water supply. In the most recent draft SWA regulations, the requirements include achieving:

(1) a dilution requirement in the reservoir of 100:1 (or 10:1 with an additional 1-log microbial pathogen treatment) and
(2) a retention time of at least six months (calculated as total volume divided by total outflow).

Currently no alternative permitting process is included in the draft SWA regulations, thus if both of these requirements cannot be met then the project would be considered a direct potable reuse project.

The anticipated treatment requirements for SWA look very similar to the GRR Regulations, particularly with regard to pathogenic microorganism control. The draft SWA regulations require that any 24-hour input of recycled water into the reservoir must be mixed such that water withdrawn for use as drinking water never contains more than 1% (or 10% with an additional 1-log treatment) recycled water. Because of the high withdrawals of reservoir water by Metropolitan Water District (MWD) from the SWP stored in Castaic Lake, the retention time in the reservoir is approximately 2 months. A simplified analysis using complete mixing of the reservoir indicates that the dilution factor would be 60:1, representing 2 months or 60 days of retention over a 24-hour or 1-day time period. This dilution factor is above the 1% dilution, but below the 10% dilution requirements, thus, the pathogenic microorganism control requirement for a SWA project in Castaic Lake would likely to require additional treatment to achieve 13/11/11 log removal requirement for virus, *Giardia*, or *Cryptosporidium (V/G/C)* (for further information see Appendix C – Section 3).

Where treatment credits are concerned, the principal difference between groundwater recharge and reservoir augmentation is the availability of treatment credit in the conventional drinking water treatment plant. The proposed treatment system concept for SWA at Castaic Lake would be to achieve the required 12/10/10 log removal requirement for V/G/C through an AWTF and rely on drinking water treatment that is located on the downstream side of the reservoir storage to meet the incremental increase to 13/11/11 log removal requirement for V/G/C. For this application, a similar FAT train is suggested as for the direct injection approach.

The ability to achieve the six month retention time requirement is independent of treatment and is discussed in Section 6.

Direct Potable Reuse Treatment Requirements

A **direct potable reuse (DPR)** project is defined as the planned introduction of recycled water either directly into a public water system or into a raw water supply immediately upstream of a water treatment plant. Thus, DPR has a spectrum of alternatives with significant differences in the 'directness' they seek. A reservoir that is too small to comply with the SWA criteria would be considered a DPR project that introduces recycled water into the raw water supply. SB918 has as its final requirement that DDW assess the feasibility of developing regulations for DPR by the end of 2016. It is important to note that SB 918 does not require the development of regulations, but only an assessment of whether or not it is feasible to do so. There is no mandated timeline for the state to develop a formal DPR regulatory framework.

The concept of DPR is fairly new and untested in California. As a result, there is very little data on DPR design, performance, and safety. The WateReuse Research Foundation (WRRF) has created a keystone project that seeks to tie together many of the findings from the last six years of potable reuse research. This project is WRRF 14-12, entitled "Demonstrating Redundancy and Monitoring to Achieve Reliable Potable Reuse". This project utilized a 1.6-MGD demonstration project at the City of San Diego's North City Water Reclamation Plant. WRRF 14-12 has developed a DPR conceptual process train that further augments both the treatment protection and the monitoring to provide continuous and demonstrable performance of a DPR train. The treatment train used in WRRF 14-12 was modified to mirror the SCVSD Chloride Compliance Project with the addition of ozone and biologically activated carbon (BAC) as pretreatment (for further information see Appendix C).

Section 6: Recycled Water Market

6.1 Non-Potable Reuse Market Survey

Title 22 of the California Code of Regulations identifies approved recycled water uses and treatment requirements for non-potable applications (Appendix B illustrates the water recycling criteria for each category of use). Based on discussions with CLWA and the purveyors, this RWMP focused on landscape irrigation and golf courses in Santa Clarita Valley primarily due to the ease and lower cost of converting irrigation only meters to recycled water. Other uses discussed include toilet and urinal flushing in dual-plumbed facilities, cooling (commercial and industrial) and construction activities (dust control, consolidation, etc.).

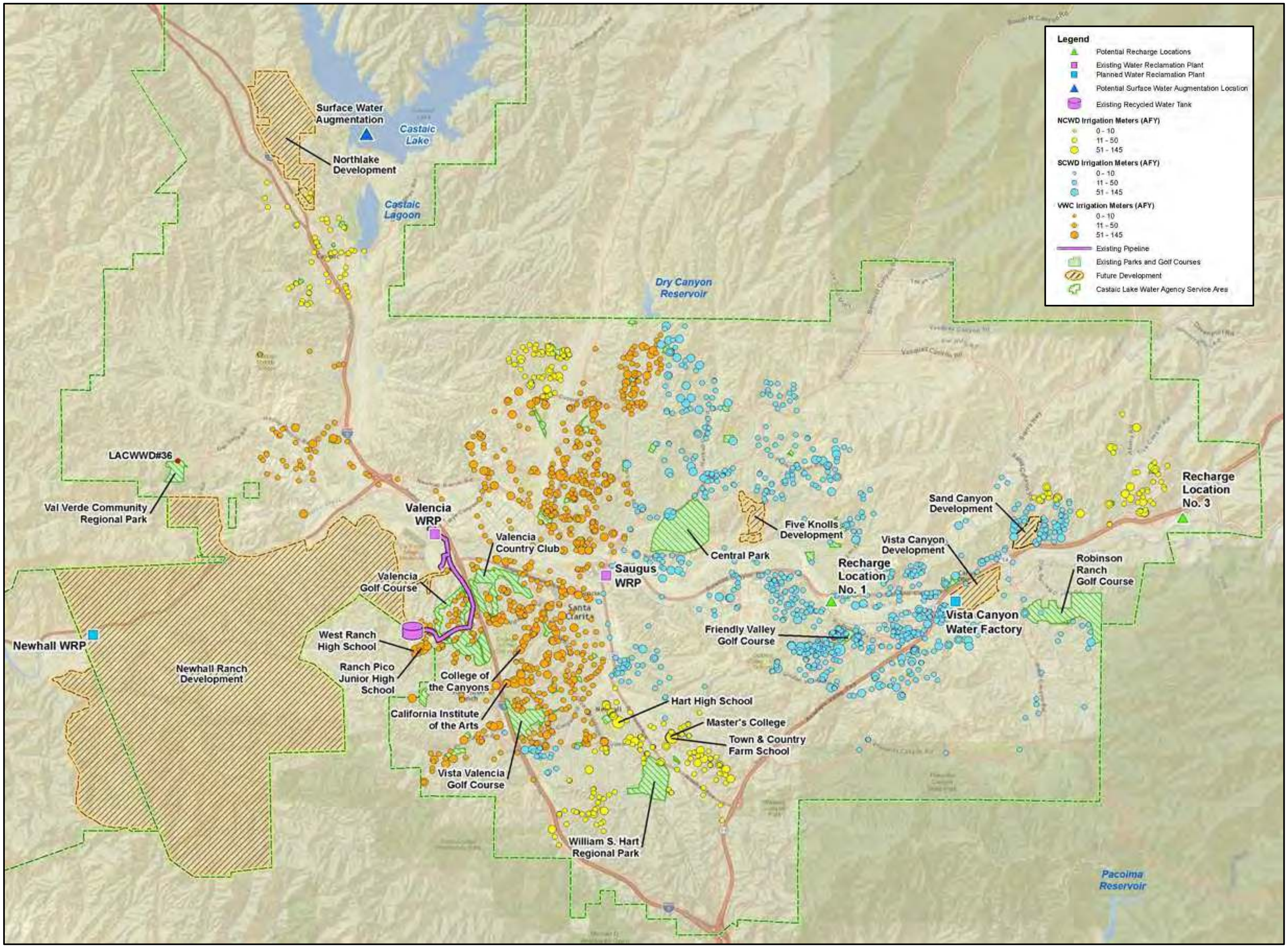
Existing recycled water demands for the Santa Clarita Valley were estimated using 2013 meter data provided by CLWA and the purveyors. In most cases, dedicated irrigation meter data was used to estimate demands. Mixed use demands were estimated based on percent irrigable land and average annual uses for the identified land use. Golf course irrigation demands were based on annual usage estimates provided by VWC. Golf course irrigation demand for Valencia Golf Course is based on deliveries from a shared well with VWC and demand for Vista Valencia Golf Course is estimated based on non-potable VWC well used exclusively by Vista Valencia that is not connected to the VWC water system. Table 6-1 summarizes the potential recycled water demands associated with landscape irrigation in Santa Clarita Valley. The market survey map shown in Figure 6-1 illustrates the location of existing irrigation meters, by purveyor, and relative demand (as indicated in the legend).

Table 6-1: Potential Recycled Water Demand for Existing Irrigation

Purveyor	Irrigation Demands (AFY)
VWC	6,070
SCWD	4,444
NCWD	1,942
LACWD36	50
Total Existing Demand	**12,506**

Specific meters that would be served by potential future recycled water alignments are identified in Section 7 - Project Alternatives Analysis and listed in Appendix A.

Figure 6-1 Recycled Water Market Survey



The potential recycled water demands for planned future developments were estimated based on information provided by planning documents and discussions with the purveyors. The majority of assumed reuse at planned developments is intended to meet irrigation demands. Some indoor use is assumed at proposed dual-plumbed public restrooms in the planned Vista Canyon development. Table 6-2 lists estimated recycled water demands associated with proposed future developments in Santa Clarita Valley included anticipated implementation dates. The location of these developments is also shown on Figure 6-1.

Table 6-2: Potential Recycled Water Demands for Future Developments

Planned Development (Purveyor)	Estimated Demands (AFY)	Projected Implementation Date
Vista Canyon (SCWD)	137	Projected Use by 2017
Five Knolls (SCWD)	152	Unknown
Sand Canyon (SCWD)	95	Unknown
Westside Communities (VWC)	265	Projected Use by 2020
	2,471	Additional Use by 2025*
	2,474	Additional Use by 2030*
	1,974	Additional Use by 2034*
North Lake (NCWD)	800	Unknown
Val Verde Community Regional Park	50	Projected Use by 2030
Total Future Demand	**8,418**	

* Demand increases based schedule for implementation for each neighborhood and population projections to estimate full occupancy.

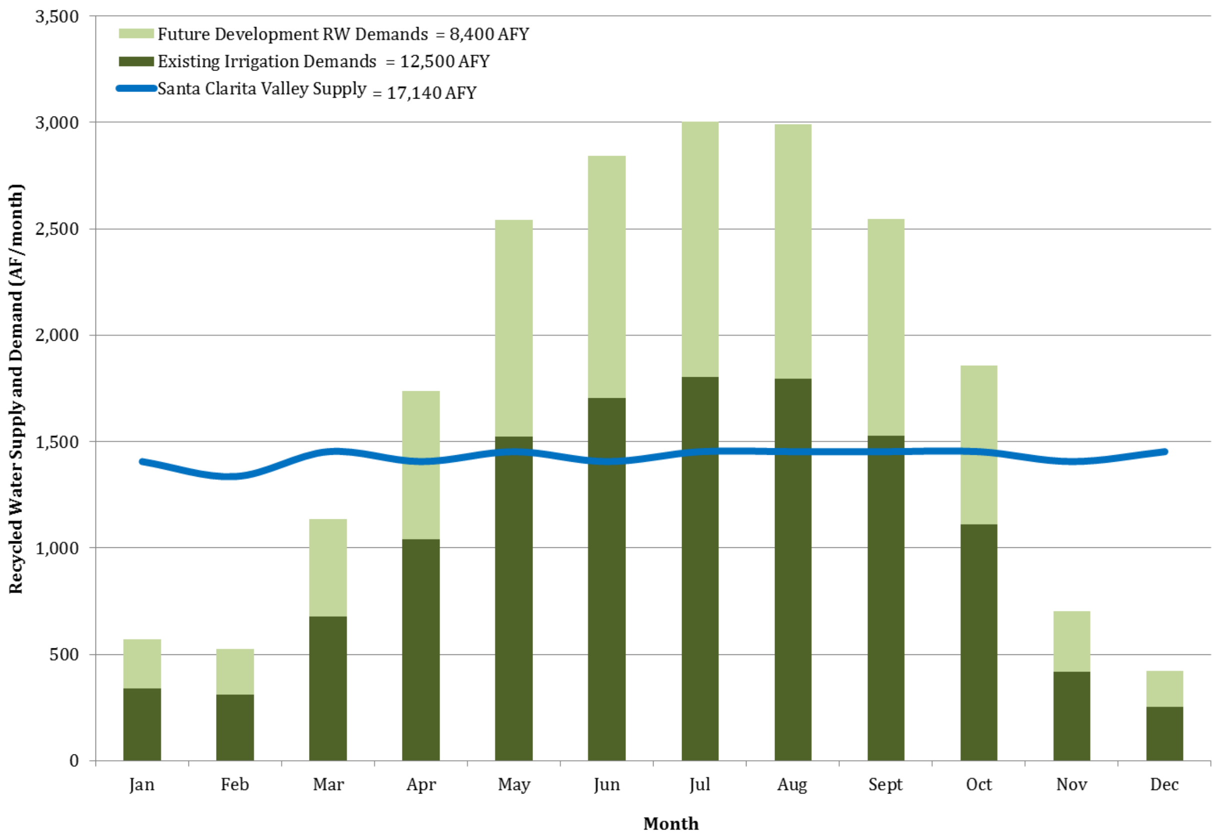
The projected available supply of recycled water in Santa Clarita Valley, previously discussed in Section 4.4, would remain relatively constant year-round while irrigation demands peak in the summer months. Table 6-3 and Figure 6-2 provides a comparison of the total projected available recycled water supply in Santa Clarita Valley and potential demand for recycled water.

Table 6-3: Comparison of Available Supply and Potential Demand in Peak Summer Months

	SCV Available Supply[1,2]	Potential Existing RW Demands[3]	Potential Future RW Demand	Potential Supply Shortfall[4]
Current Supply and Demand (2015)				
Annual (AFY)	6,300	12,506		-6,206
Peak Summer Month[3] (AFM)	525	1,751		-1,226
Projected Future Supply and Demand (2050)				
Annual (AFY)	17,140	12,506	8,418	-3,784
Peak Summer Month[3] (AFM)	1,425	1,751	1,179	-1,504

[1] Includes projected recycled water produced at the Valencia WRP, Saugus WRP, planned Newhall WRP and planned Vista Canyon Water Factory less required discharge to the Santa Clara River
[2] Assumes relatively constant year-round production of recycled water.
[3] Annual demands based on meter data for existing irrigation meters (as shown in Table 6-1). Peak summer demand based on historical monthly demand distribution for Phase 1 system (14% of demand occurs in July).
[4] Calculated as supply minus demand.

Figure 6-2 Recycled Water Supply and Potential Demand in Santa Clarita Valley (2050)



As shown in Table 6-3, the Santa Clarita Valley is supply limited, both annually and during the peak irrigation months now and in the future. Figure 6-2 clearly illustrates that only half of the summertime demand for existing and future irrigation needs could be met. In addition, the geographic distribution of the dedicated irrigation meters, shown in Figure 6-1 , would make it cost prohibitive to serve many of these potential customers due to the significant amount of conveyance infrastructure that would be required. Identification of potential customers, the appropriate source of recycled water, infrastructure and a proposed phasing plan to align supply and demand over time is evaluated in Section 7 – Project Alternatives Analysis.

6.2 Potable Reuse Market Survey

The potable reuse concepts investigated within the Santa Clarita Valley for this study include groundwater recharge, surface water augmentation and direct potable reuse. A market survey for potable reuse is not associated with meters; but rather a more holistic approach to assess opportunities to beneficially reuse the recycled water for potable uses directly or indirectly. Some of the potential benefits and challenges associated with potable reuse in Santa Clarita Valley are summarized below:

Potential Benefits of Potable Reuse in Santa Clarita Valley:

- Develop a local, drought-proof and sustainable water supply
- Reduce reliance on imported water
- Use of available recycled water flows in the winter and off-peak irrigation months
- Reduce discharges to the Santa Clarita River (after meeting instream flow requirements)
- Repurpose unused capacity in the SCVSD AWTF designed to remove chloride
- Recharge groundwater basin(s) (via groundwater recharge)
- Maintain lake levels (via surface water augmentation)
- Provide an integrated approach solving multiple issues (storm water, chloride removal, GW recharge, flood control, open space, etc.), which could bring together a number of stakeholders in Santa Clarita Valley.

Potential Challenges of Potable Reuse in Santa Clarita Valley:

- Higher costs associated with advanced treatment and brine disposal
- Higher costs associated with pumping and conveyance (for GRR and SWA projects)
- Additional regulatory requirements (i.e. permitting, monitoring, and reporting)
- Public acceptance
- Development of partnerships and agreements (with Los Angeles County Flood Control District (LACFCD) for a GRR project, Metropolitan Water District of Southern California for SWA and others)
- Regulatory uncertainty related to SWA and DPR requirements

Section 5.3 introduced potable reuse concepts and their treatment requirements. The following sections describe how potable reuse concepts could be implemented in Santa Clarita Valley. The infrastructure and flows for specific potable reuse alternatives are presented in Section 7.

Indirect Potable Reuse

The Recon Study (Carollo 2015) provided an initial assessment of groundwater recharge with recycled water through surface spreading into the alluvial aquifer and aquifer storage and recovery via groundwater injection into and extraction from the deeper Saugus formation. These options were explored in greater detail to assess the potential for recharge of excess available recycled water in the winter and off-peak irrigation months.

Surface Spreading

The Recon Study identified three recharge locations (shown in Figure 6-3) as potential spreading basins based on the six-month retention time requirement used in the GRR Regulations to achieve 10-log removal of *Cryptosporidium* and *Giardia*.

Figure 6-3 Potential Recharge Locations

Source: Recon Study (Carollo 2015)

Based on further evaluation for this RWMP:

- **Recharge Location #1** is shifted to an off-stream location just upstream of the location shown in Figure 6-3 to minimize in-river activities, challenges associated with maintaining the spreading facility during storm events and potential for discharge to the river.
- **Recharge Location #2** was eliminated as an option in the Recon Study due to its proximity to existing drinking water wells, which would result in retention times below 6-months. No further analysis on this location was considered as part of the RWMP effort.
- **Recharge Location #3** is included at the same location as an in-river option. An in-stream and off-stream spreading option are presented. The off-stream option would require the purchase of private land just upstream of the location shown in Figure 6-3.

The following initial design assumptions were made to evaluate the size, timing and quantity of recycled water that could be recharged at Recharge Locations #1 and #3:

- Use of city owned parcels where available.
- Assumed infiltration rate of 3 feet per day.
- Recycled water allocated for irrigation would take priority over recharge (i.e. a GRR project would be limited by the seasonal availability of recycled water).
- Stormwater capture would be prioritized over recycled water (i.e. during heavier months of rainfall, spreading RW would be limited).

To determine the retention times associated with Recharge Location #1 and Recharge Location #3, groundwater modeling was performed by GSI Water Solutions, Inc. (GSI). The modeling results show that there is sufficient subsurface travel time to meet the required pathogenic microorganism control log removals, however for Recharge Location #1, the modeled travel time is below 12 months. The GRR Regulations stipulate that a 6-month travel time is required, however groundwater model simulations, such as was used here, only receive 50% credit for the determined travel time. As a result implementation of an alternative using Recharge Location #1 would require one of two options: 1) Spread potable water spiked with a tracer to verify the travel time or 2) shut down well VWC-U4 for a time period on the order of 6-12 months upon project commencement while the tracer test is performed. For these tracer tests, if an intrinsic tracer is used, the travel time would need to be confirmed as 9 months or greater to receive full microorganism control credit. If an added tracer is used, the travel time would need to be confirmed as 6 months to receive full microorganism control credit. See Appendix C – Section 2.3 for additional description of the model assumptions and findings.

An important parameter in any surface spreading project is the municipal recycled wastewater contribution (RWC) and its closely related TOC requirement in the GRR Regulations. The RWC is defined as:

$$RWC = \frac{Recycled\ Water\ Applied}{Recycled\ Water\ Applied + Diluent\ Water}$$

Diluent water is defined as the pre-existing surface flow (e.g. rainfall, stormwater, or irrigation runoff), subsurface flow (e.g. native groundwater) available to blend with the RW. In the case where surface flow data is absent, such as in Recharge Location #1 and Recharge Location #3, native groundwater (herein referred to as "groundwater underflow") is relied upon as the dilution water. The available groundwater underflow was modeled by GSI as part of the Recon Study and is based on Darcy's Law, which consists of the hydraulic conductivity, cross sectional area, and hydraulic gradient of the desired recharge basin. A conservative calculation of groundwater underflow, based on the use of the cross-sectional area of the recharge basin, results in 16.1 MGD and 4.5 MGD of modeled diluent water at Recharge Locations #1 and #3 respectively.

Per the GRR Regulations, at the beginning of the project, the initial maximum RWC cannot exceed 20% unless specifically pre-approved. For the initial RWC of 20%, a maximum total organic carbon

(TOC) concentration of 2.5 mg/L must be achieved in the percolated water from a surface spreading project, as calculated in the following equation:

$$TOC_{max} = \frac{0.5\ mg/L}{RWC} = \frac{0.5\ {}^{mg}/_{L}}{20\%} = 2.5\ {}^{mg}/_{L}$$

The TOC concentration may therefore limit the quantity of water that can be recharged. For planning purposes, SCVSD provided an average TOC value of 4.7 mg/L for the Valencia and Saugus WRPs. This is above the 2.5 mg/L for an initial 20% RWC and as such two mitigation efforts would need to be utilized to meet the TOC requirement: 1) blending of tertiary wastewater with AWTF water to lower the TOC above ground and 2) receiving credit for the TOC removal that naturally occurs via SAT by monitoring TOC levels in water after percolation but before blending with native groundwater.

Assuming the TOC requirement is able to be met through the mitigation efforts presented, a 20% initial RWC would result in a recycled water application of 4.0 MGD and 1.1 MGD for Recharge Locations #1 and #3, respectively based on the modeled groundwater underflow. The diluent volume limitation of Recharge Location #3 is noticeable in the low amount of recycled water that can be spread in the initial startup of the groundwater replenishment project. Once an IPR spreading project is underway and has shown itself to be protective of public health and the environment, the sponsor (CLWA or purveyor) can petition DDW to increase the RWC, up to a value of 50% for non-advanced treated source water.

There are a number of other considerations that would influence the amount of recycled water that could ultimately be recharged at each site, including the:

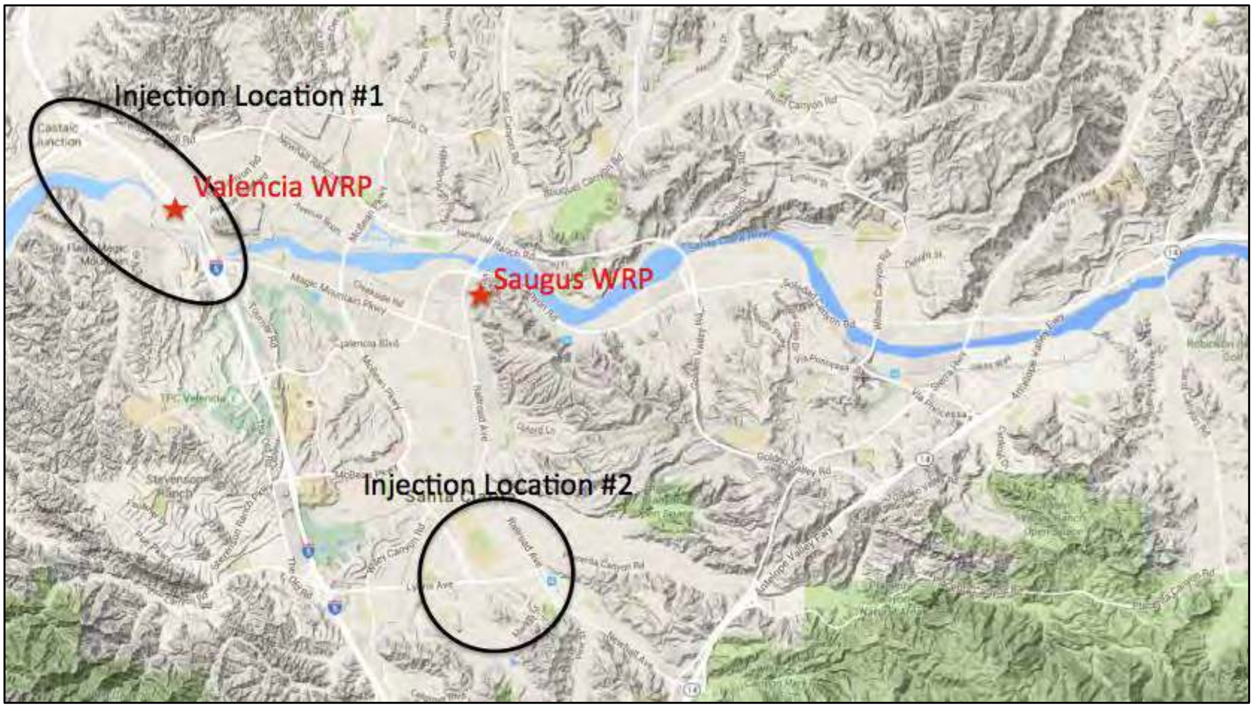
- source of recycled water (Valencia WRP or Saugus WRP)
- quantity of Valencia Blend water available
- available recycled water supply after meeting non-potable demands
- operational criteria for stormwater recharge imposed by LACFCD

These concepts are described in greater detail in Appendix C and in Section 7 – Project Alternatives Analysis. A more detailed feasibility study would be required to confirm the assumptions about the volume of recycled water that could be recharged and recovered based on current regulations, source water quality, operational and cost considerations.

Direct Injection

The Recon Study identified two potential locations for injection wells to introduce RW into either the Saugus Formation or the Alluvial Aquifer in the Valley's groundwater basin, as shown in Figure 6-4.

Figure 6-4 Potential Direct Injection Locations



Source: Recon Study (Carollo 2015)

To minimize additional costs, this RWMP assumes that the injection wells could be located at Injection Location #1 in the vicinity of the Valencia WRP, along with the AWTF. SCVSD indicated that they were uncertain if there would be available footprint, so additional conveyance costs are possible if the AWTF and injection well would need to be located further away from the Valencia WRP.

For direct injection, the GRR Regulations mandates a minimum retention time in the groundwater basin of 2 months, though no existing facilities currently operate with a retention time under 6 months. For this study, it was assumed that a travel time of 6-months could be identified within the aquifer nearby the Valencia WRP. Similar to surface spreading, additional consideration of this concept should include a detailed analysis of groundwater travel times in a follow-on feasibility study.

The direct injection of recycled water is not restricted by the RWC, as the GRR Regulations allow for 100% RWC upon commencement of the project (rather than the 20% initial RWC for surface spreading). Therefore, a direct injection project is not limited by the availability of diluent water. A direct injection project is also not hindered by inclement weather as water can be injected into the ground regardless of the weather conditions. As such, all of the available recycled water could be utilized by a direct injection project. Furthermore, given the capital investment required for the AWTF, maximizing the usage of all available recycled water would be critical for creating the most economical alternative possible. Therefore, direct injection is presented in Section 7 that includes an AWTF designed to treat all available recycled water for potable reuse.

Surface Water Augmentation

The SWA concept would require an AWTF to treat 100% of the available recycled water from the SCVSD, delivery to Castaic Lake and brine disposal via truck hauling. As discussed in Section 5.3.2, the size of the Castaic Lake and the anticipated project flow is such that at least 10:1 dilution can likely be achieved in the reservoir. The draft regulations also stipulate that a reservoir used for SWA must have a minimum theoretical retention time of 6 months, to be measured on a monthly basis. The California Department of Water Resources (DWR) tracks the flow out of the Castaic Lake Reservoir and over the past 10-years an average of 475 MGD leaves the reservoir per year (DWR, 2015). Using the low water level previously discussed, the calculated theoretical retention time is 2 months (for further information see Appendix C). Because of the large outflows from the reservoir for other purposes, reduction of project flow would not enable this project to qualify as a SWA project based on the criteria in the draft SWA regulations.

Unlike the groundwater regulations, there is no stipulation in the draft SWA regulations that allows for a project sponsor to petition the DDW for an alternative permitting process for the reservoir criteria. Currently, discussions regarding this alternative permitting process are ongoing as other potential project sponsors are finding themselves in a similar situation with a lower retention time than stipulated in the draft regulations. A decision will be made later in 2016 whether to allow some flexibility in this requirement.

Despite the regulatory uncertainty, a SWA is included in the RWMP alternatives. Similar to direct injection, the SWA alternative is not restricted by the RWC and therefore, the AWTF would be designed to treat all available recycled water. The total volume available for SWA and the associated conveyance facilities is presented in Section 7.

Direct Potable Reuse

A DPR concept could potentially utilize all recycled water not already allocated for non-potable reuse, and would require full advanced treatment of the recycled water from SCVSD, brine disposal via truck hauling and only minimal conveyance requirements. The DPR alternative would treat 100% of the available recycled water from the SCVSD at an AWTF and the purified water would be blended with the raw water entering the Rio Vista Filtration Plant (an existing drinking water treatment plant) for further treatment prior to distribution. For the purpose of this study, the treatment train would be similar to the treatment provided for direct injection or SWA but with the addition of ozone and BAC pretreatment, as previously discussed in Section 5.3.

It is important to note that this alternative is speculative as there is neither a developed framework for regulations nor any established timeframe for promulgating DPR regulations. CLWA and the purveyors should track direct potable reuse developments in California and revisit the feasibility DPR if a goal to achieve 100% re-use of available wastewater is desirable. The total volume available for DPR and the associated conveyance facilities is presented in Section 7.

Section 7: Project Alternatives Analysis

This section describes the alternatives considered and lists the planning and design criteria applied to analyze each project in a given alternative. A summary of uses, demands and facilities are provided for each project, including a project map. The potential for repurposing existing infrastructure, consideration of seasonal storage and customer retrofits are also discussed. Capital, operating and life-cycle costs are provided for each alternative in the last section.

7.1 Alternatives Evaluated

Four alternatives are explored as part of the alternatives evaluation:

- **Alternative 1 - Non-Potable Reuse Expansion (Phase 2):** looks at near-term opportunities to expand recycled water use for non-potable uses (i.e. irrigation, commercial, etc.). This alternative focuses on the Phase 2 expansion, which extends alignments beyond the existing Phase 1 system and supports upcoming design work and the pursuit of currently available grants and loans for recycled water projects.

- **Alternative 2 - Non-Potable Reuse Expansion (Future Phases):** assesses mid-term opportunities to expand recycled water use for non-potable uses. This alternative considers future alignment extensions beyond Phase 2 for landscape irrigation and other non-potable uses, as well as service to the planned new development for the Westside Communities.

- **Alternative 3 - Groundwater Recharge (Surface Spreading):** assesses mid-term opportunities to expand recycled water use for non-potable uses while implementing a groundwater recharge project via surface spreading.

- **Alternative 4 - Advanced Treatment for Potable Reuse:** considers long-term opportunities to implement a potable reuse project. This alternative considers both indirect and direct potable reuse projects that require advanced treatment to meet regulatory requirements, including: (1) groundwater recharge via direct injection in the vicinity of the Valencia WRP and other viable locations with the Valley, (2) surface water augmentation at Castaic Lake and (3) direct potable reuse by blending with the raw water supply at the Rio Vista Water Treatment Plant (WTP).

Each alternative consists of a group of projects; some can be constructed independent of other projects, while others would build on previous phases and require upsizing of facilities to meet increased future flows. A discussion of general planning and design criteria applicable to all projects is provided in Section 7.2.

7.2 Planning and Design Criteria

Conveyance Facility Evaluation

Conveyance facilities are sized to meet hydraulic requirements and customer demands for each alternative based on the demand information developed in the Market Assessment (Section 6) and input from CLWA and purveyors regarding the potential future development demands. General assumptions include:

- Conveyance facilities (pipeline and pump stations) sized to meet the peak hour demand
- Operational storage sized for approximately 75% of the peak day demand
- New Pipelines: 8"-dia to 20"-dia buried "purple" high-pressure PVC

  > 20"-dia buried steel or ductile iron
- Maximum design velocity: 6 feet per second (fps)
- Maximum system pressure: 215 pounds per square inch (psi)
- Minimum delivery pressure: 55 psi
- Optimum delivery pressure: 55 to 150 psi
- Elevation contour data provided by CLWA

Pipeline Evaluation

All new recycled water customers (beyond Phase 1 customers currently being served with recycled water) would be served by new distribution pipelines. Customers served are based on Hydraulic modeling performed to evaluate the minimum pipeline sizes required to meet a max day peaking factor of 2.25, as described in Appendix D.

Pipeline design considerations should include the following:

- Minimum cover of 3.5 to 4 feet to protect the pipeline from live loads while minimizing dewatering costs. When the minimum cover requirements cannot be met, the pipe trench loading should be further analyzed. In such cases, the use of concrete or slurry encasement may be necessary.
- As established by the DDW, the minimum separation for existing water mains and new pipelines carrying tertiary-treated recycled water shall be in conformance with Section 64572 of Title 22 California Code of Regulation. There shall be at least a 4-foot horizontal separation where lines are running parallel and a 1-foot vertical separation (water line above recycled water line) where the lines cross each other. When these criteria cannot be met, special permission must be obtained from DDW.
- A minimum clearance of at least 12 inches (when paralleling) and 6 inches (when crossing) electric lines is required by the Southern California Edison Underground Structures (UGS-100) and the California Public Utilities Commission General Order (GO-128).
- Appurtenances shall be installed appropriately to protect the pipeline from water hammer, collapse, and vacuum and to isolate and/or drain the pipe. Appurtenances shall include air and

vacuum release valves, blowoff/pumpouts, and valves. All appurtenances shall comply with applicable AWWA standards.

Pump Station Evaluation

Distribution pump stations are sized to meet customer instantaneous peak demands and pressure service requirements. New pump stations would include vertical turbine pumps with two to three operating pumps capable of delivering the required combined capacity, and one pump would operate as a standby unit. The pump configuration type is similar to those used at several pump stations throughout CLWA's potable and recycled water systems, including the Valencia WRP pump station.

Pump station total dynamic head (TDH) is estimated in order to provide conceptual level estimates of pump station capital and operating costs (Section 7.8) for the purpose of alternative comparison. Ground and water surface elevations were estimated using GIS mapping data when available and system operating pressure is assumed to be 80 psi. The hydraulic grade line (HGL) for the selected scenario should be confirmed with hydraulic modeling during preliminary pump station design and pump selection.

Pump Station design considerations should include the following:

- A pump control valve for each pump.
- A pressure relief / surge control valve on the discharge header.
- Butterfly valves on the discharge piping for isolating the pumps.
- A magnetic type flowmeter installed above grade on the discharge header.
- Air release valves for the pump discharge.
- An emergency power standby generator.
- Appropriate instrumentation and controls.

Operational Storage Evaluation

Storage is used to meet peak customer demands and diurnal demand fluctuations while allowing for constant recycled water treatment production rate. Storage requirements were modeled on an hourly time-step over a 24-hour period, as described in Appendix D. It is assumed that additional storage would not be provided for backup service in the event of a partial or complete treatment plant shutdown. Instead, standby service would be provided from potable water via air-gap connections at the storage reservoir.

Additional assumptions used for the storage sizing evaluation include:

- Treatment facilities will operate 24 hours per day to produce recycled water at a constant rate.
- Service to recycled water customers would be provided at the peak hour demand rate.
- A 25% contingency for storage volume is desirable to allow for actual peak demand times and flow rates that might be different from the estimates and assumptions used herein.

- A backup potable supply would be provided at each of the recycled water storage tanks to maintain flow through the distribution system during interruption of recycled water production to meet customer demands.
- A connection to the potable water system would require an air gap separation to protect the potable water system from cross connection with the recycled water system.

Treatment Facility Evaluation

An evaluation of treatment requirements was presented in Section 5. The assumptions related to sizing of treatment facilities are based on the type of use and source of recycled water.

Non-potable reuse alternatives: would rely on tertiary treated recycled water provided by existing or planned facilities. No additional tertiary treatment facilities would be constructed.

Indirect and direct potable reuse alternatives: would require additional treatment provided by the SCVSD's planned AWTF or a new AWTF. Due to the limited supply of recycled water available in Santa Clarita Valley (as discussed in Section 6), the indirect and direct potable reuse alternatives can take advantage of excess recycled water flows available during the winter and shoulder months when irrigation demands are low. Additional information about advanced treatment processes is provided in Appendix C.

- **Groundwater spreading alternatives** would rely on tertiary treated recycled water blended with advanced treated water provided by SCVSD (previously described in Section 5). No additional tertiary or advanced treatment facilities would be constructed.
- **Direct injection alternatives** would require construction of a new AWTF sized to meet the peak day demand during the winter months.
- **Surface water augmentation alternatives** would require construction of a new AWTF sized to meet the peak day demand during the winter months.
- **Direct potable reuse alternatives** would require additional treatment provided by a new AWTF.

Overview of Hydraulic Model Approach

A hydraulic model of the recycled water system alternatives is utilized to provide facility sizes and verify hydraulic feasibility. An extended period simulation is utilized to evaluate system pressures and pipeline velocities under maximum day demand (MDD) conditions for Alternatives 1 and 2, and winter day demand (WDD) conditions for Alternative 3. Facility sizes are determined based on meeting the design criteria described in Section 7.2. Alternative 4 is not analyzed with the hydraulic model; facility sizes are determined using Excel calculations.

For Alternatives 1 and 2, which consist of expansion of the non-potable reuse system, the hydraulic model simulation utilizes MDD conditions, which include application of an MDD peaking factor of 2.25 for annual average demand and application of an 8-hour irrigation window from 10 p.m. to 6 a.m. on a daily basis. Effectively, the peak hour demand is three times the MDD demand.

Note that hydraulic model simulations are provided for only Phase 2A and Phase 2B. As shown in Zone, three different alignments are analyzed for Phase 2A: Bouquet Canyon Road, Central Park South without Tank, and Central Park South with Tank. Facility sizes are provided for each alignment, as shown in Appendix D. One alignment is analyzed for Phase 2B.

Facility sizes for Phase 2C and Phase 2D were analyzed independent of the RWMP and hydraulic models were developed under separate projects. The modeling components have been assimilated into the overall hydraulic model and the recommended facility sizes are incorporated in this report.

Alternative 3 consists of groundwater recharge via surface spreading options. As described in Section 7.5, groundwater recharge would occur in winter months or when non-potable water demands are low. The hydraulic model simulation utilizes WDD conditions for non-potable reuse demand, which includes a WDD peaking factor of 0.2 for annual average demand and application of an 8-hour irrigation from 10 p.m. to 6 a.m. on a daily basis. The WDD peaking factor is based on historical monthly demand data for CLWA's existing recycled water system. If in the future irrigation demands decrease due to future regulatory conservation restrictions or if the minimum discharge requirements to the Santa Clara River decreases, additional supplies could be available for groundwater recharge.

The groundwater recharge demand is based on the anticipated maximum month delivery and is assumed to be constant over a 24-hour period. Note that hydraulic model simulations are provided for only 'Phase 2A + Spreading Site #1' and 'Phase 2A + Spreading Site #3a'. Facility sizes for the other options of Alternative 3 are based on the results of these two simulations.

Results of the hydraulic model simulations are provided in Appendix D. A figure is provided for each simulation showing recommended pipe sizes and pump station capacities.

7.3 Alternative 1 – Non-Potable Reuse Expansion (Phase 2)

Four projects planned to expand recycled water use within Santa Clarita Valley, which are collectively known as Phase 2, are depicted in Figure 7-1, and are currently in various stages of design. Phase 2A, 2C and 2D would use recycled water from the Valencia WRP and Phase 2B would use recycled water produced at the Vista Canyon Water Factory, which is being constructed to treat flows from the planned Vista Canyon Development.

A summary of Alternative 1 key customers, anticipated annual demands, and construction completion dates and purveyors for each phase are listed in Table 7-1. A map of each Alt 1 – Phase 2 project is provided in Figure 7-2 through Figure 7-5. Appendix A lists the anticipated recycled water demands by meter and Appendix D summarizes the hydraulic modeling results. Costs are summarized in Section 7.9 and detailed cost sheets are provided in Appendix E.

Phase 2A – consists of a new transmission main from the Valencia WRP to Central Park. The alignment runs north on Rye Canyon Road from the Valencia WRP to Newhall Ranch Road, then east on Newhall Ranch Road to Bouquet Canyon Road. At this juncture, three alignment alternatives are analyzed: Bouquet Canyon Road, Central Park South without Tank, and Central Park South with

Tank. The Bouquet Canyon Road alignment runs north on Bouquet Canyon Road from Newhall Ranch Road to Central Park. The Central Park South without Tank alignment runs east on Newhall Ranch Road from the intersection of Newhall Ranch Road and Bouquet Canyon Road, then north on a service road to Central Park. The Central Park South with Tank alignment is an identical alignment, but includes a storage tank south of Central Park. The Central Park South alignments are able to serve non-potable reuse demand from the River Village area and is conducive to expansion of the recycled water system, as described in Alternatives 2 and 3.

Phase 2B – consists of a new transmission main from the proposed Vista Canyon Water Factory south to a new storage tank close to existing Cherry Willow potable water storage tanks. The backbone main runs along Cherry Willow Drive and a distribution main runs along Lost Canyon Road from Medley Ridge Drive to Wren Drive. The system will also serve non-potable reuse demands within the proposed Vista Canyon development.

Phase 2C – consists of a new transmission main from a connection to the existing recycled water system at the intersection of Valencia Boulevard and The Old Road and terminates at Newhall Elementary School. The alignment runs east on Valencia Boulevard, then south on Rockwell Canyon Road to McBean Parkway. At this juncture, two alignments are analyzed: McBean and Drainage. The McBean alignment continues east on McBean Parkway, south on Orchard Village Road, east on 16th Street, then south on Newhall Avenue. The Drainage alignment runs south on Tournament Road, east on a stormwater drainage channel, south on Orchard Village Road, east on 16th Street, and then south on Newhall Avenue. Two of the largest customers for Phase 2C include the Valencia and Vista Valencia Golf Courses.

Phase 2D – consists of a new pump station located adjacent to the existing Recycled Water Storage Tank No. 1 and a new transmission main that extends east on Westridge Parkway, south on Old Rock Road, and west on Valencia Boulevard. This phase can potentially tie in to the proposed non-potable reuse system for the Westside Communities.

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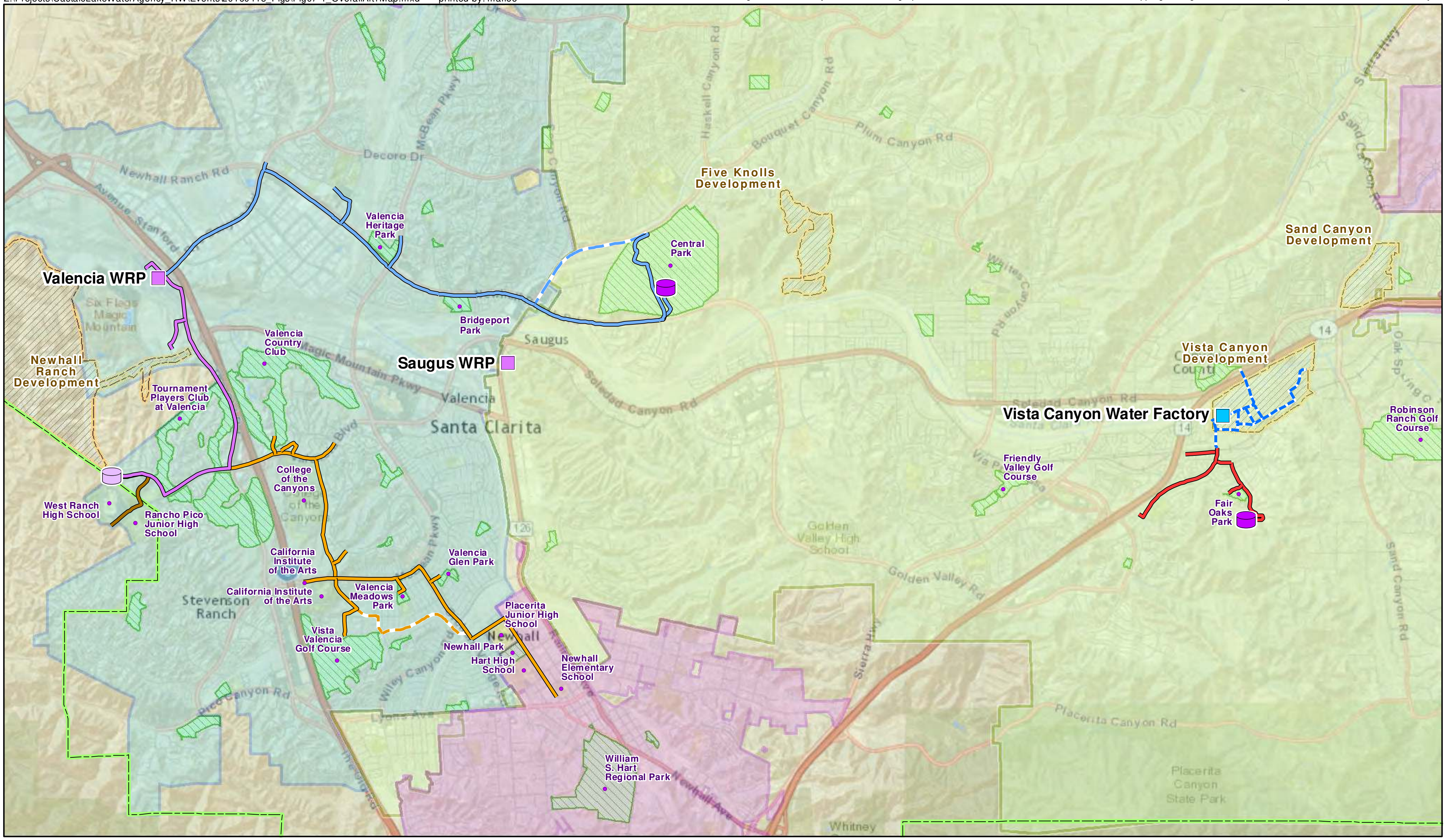

Legend

- Existing Water Reclamation Plant
- Planned Water Reclamation Plant
- Existing Recycled Water Tank
- Proposed Recycled Water Tank
- Existing Phase 1 Pipeline
- Proposed Vista Canyon RW Pipeline
- Planned Phase 2A Pipeline
- Planned Phase 2B Pipeline
- Planned Phase 2C Pipeline
- Planned Phase 2D Pipeline
- Castaic Lake Water Agency Service Area
- Newhall County Water District
- Santa Clarita Water Division
- Valencia Water Company
- Planned Developments
- Existing Parks and Golf Courses

N

0 3,000 6,000
Feet

Kennedy/Jenks Consultants

Castaic Lake Water Agency
Recycled Water Master Plan
Santa Clarita, California

Alternative 1
Non-Potable Reuse Expansion
(Phase 2)

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September2016

Figure 7-1

Table 7-1: Summary of Alternative 1 - Demands and Customers

Alt 1 Projects	RW Demand (AFY)	Purveyor Demand (AFY)			Anticipated Construction Completion Date	Key Customers
		SCWD	VWC	NCWD		
Phase 2a	560	224	336	-	2024	Central Park and irrigation customers along the pipeline alignment
Phase 2b	300	300	-	-	2017	Proposed Vista Canyon Development and nearby irrigation customers
Phase 2c	1,374	-	1,125	249	2020	West Ranch High School, Valencia Country Club, Vista Valencia Golf Course, College of the Canyons, California Institute of the Arts, Hart High School, and Newhall Elementary School
Phase 2d	186	-	186	-	2019	Ranch Pico Junior High School and customers along the way

Table 7-2: Summary of Alternative 1 Facilities

Alternative 1 - Facility Components	Alt 1 - Non-Potable Reuse Expansion (Phase 2)					
	Phase 2A			Phase 2B	Phase 2C	Phase 2D
	Bouquet Canyon Road	Central Park South w/o Tank	Central Park South w/ Tank	Combined SCWD + Vista Canyon	VWC + NCWD Extensions	VWC Extension
Total Pipeline Length (feet)	31,400	38,400	38,400	23,200	30,900	5,200
Storage (MG)	hydro tank	-	1.0	1.0	-	-
Pump Station Total Flow (gpm)	2,200	2,500	2,500	410	2,000	1,000
	-	-	-	-	5,200	-
Site Retrofit (# of Sites)	42	51	51	17	66	14

MG = million gallons, gpm = gallons per minute

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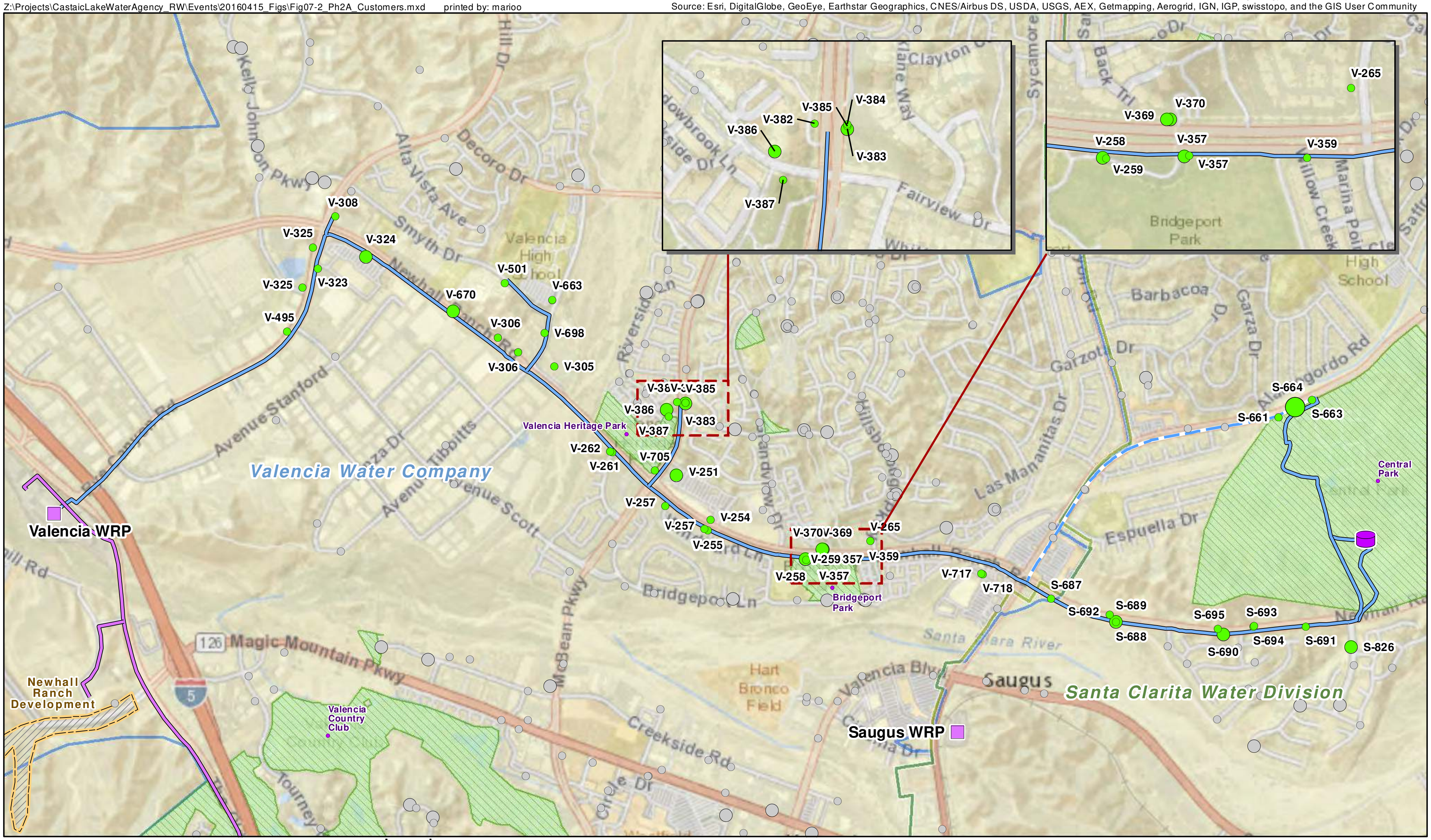

Legend

Irrigation Meters within Phase 2A (AFY)

- 0 - 10
- 11 - 50
- 51 - 145

Irrigation Meters not within Phase 2A (AFY)

- 0 - 10
- 11 - 50
- 51 - 145

- Existing Water Reclamation Plant
- Proposed Recycled Water Tank
- Existing Phase 1 Pipeline
- Planned Phase 2A Pipeline
- Castaic Lake Water Agency Service Area
- Santa Clarita Water Division
- Valencia Water Company
- Planned Developments
- Existing Parks and Golf Courses

N

0 1,150 2,300
Feet

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Recycled Water Master Plan
Santa Clarita, California

Alternative 1 - Phase 2A

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September 2016

Figure 7-2

Service Layer Credits: Sources: Esri, DeLorme, NAVTEQ, USGS, Intermap, iPC, NRCAN, Esri Japan, METI, Esri China (Hong Kong), Esri (Thailand), TomTom, 2013
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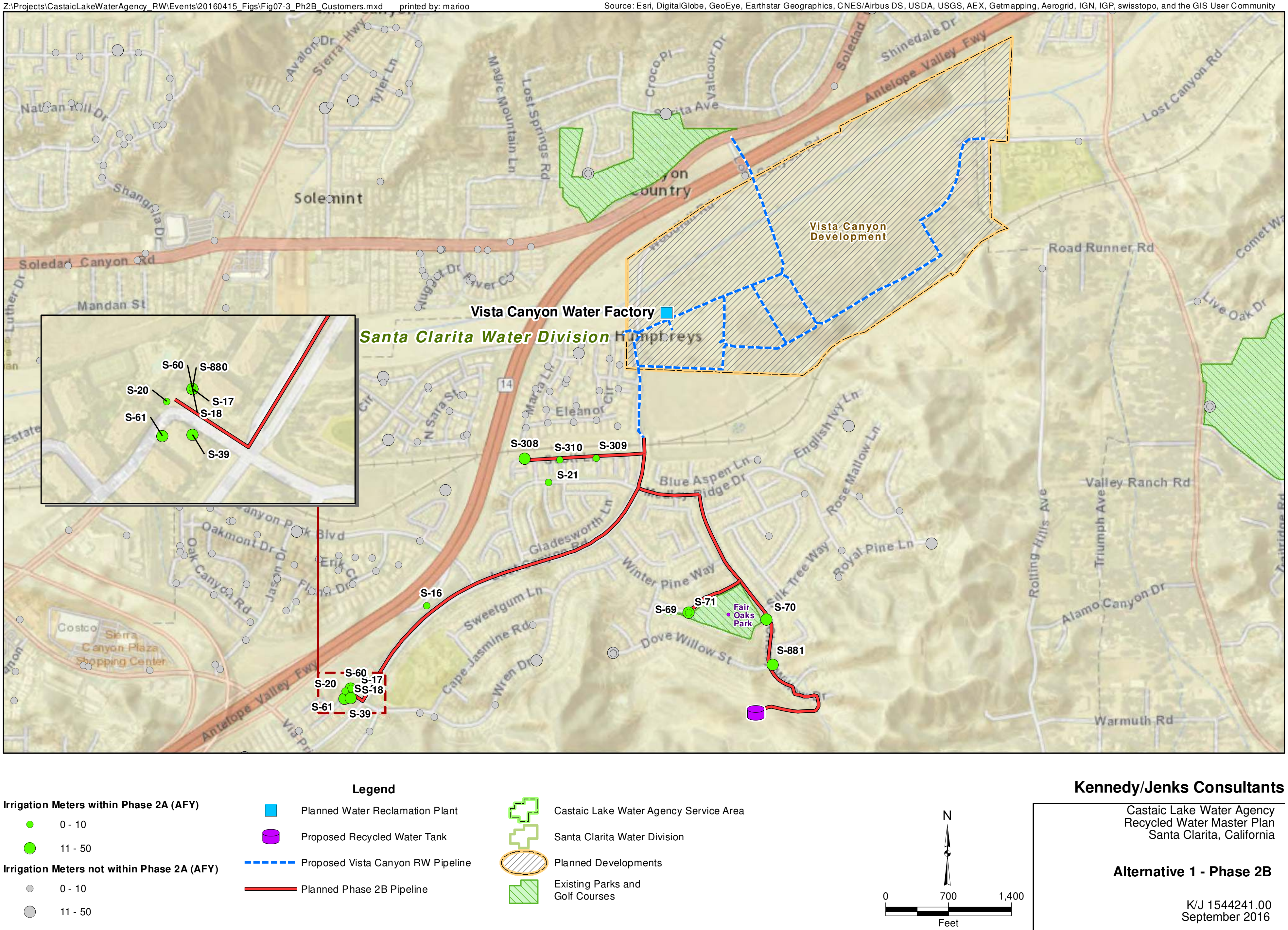

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Santa Clarita, California

Alternative 1 - Phase 2B

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Figure 7-3

Z:\Projects\CastaicLakeWaterAgency_RW\Events\20160415_Figs\Fig07-4_Ph2C_Customers.mxd printed by: marioo

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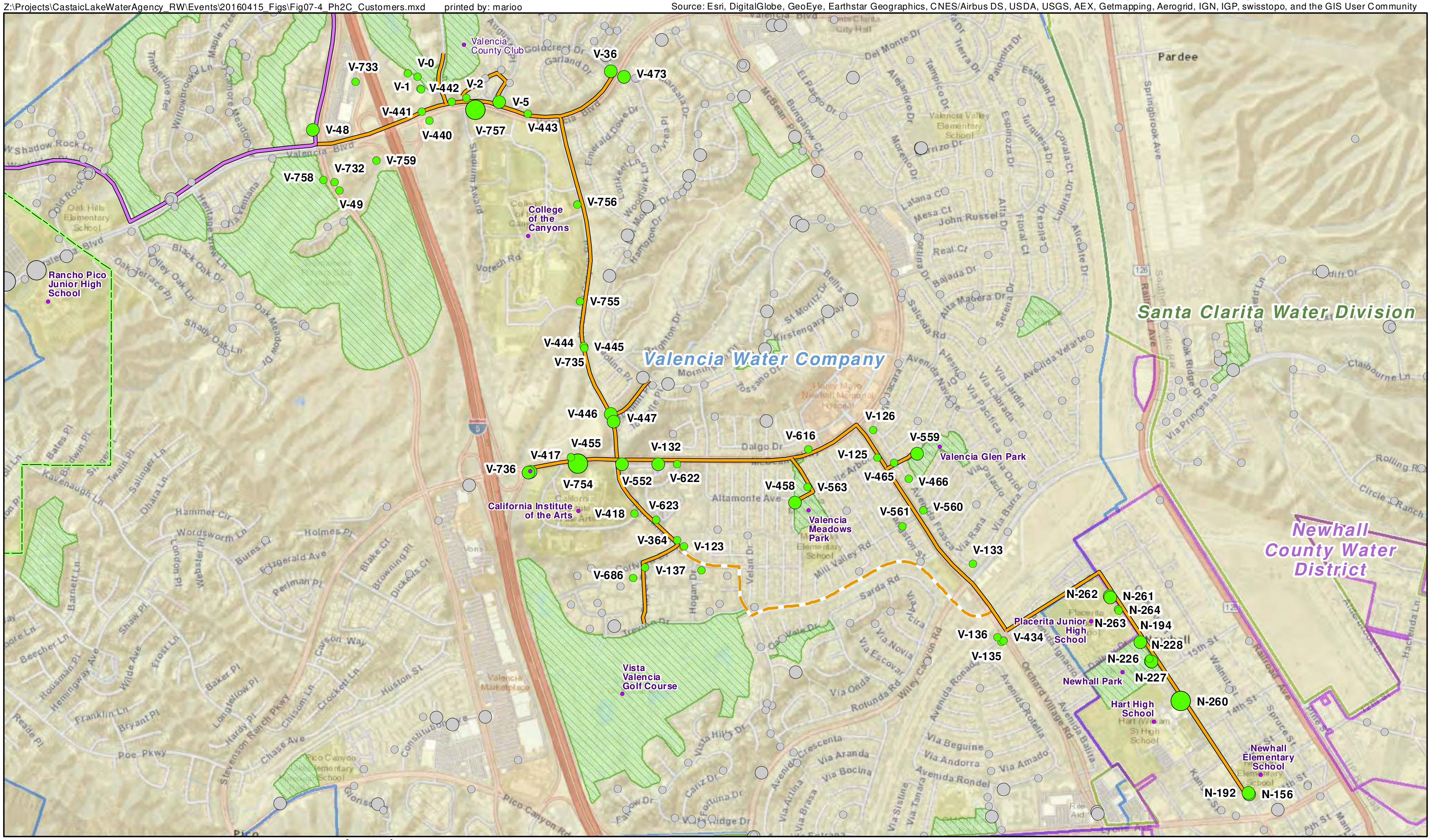

Legend

Irrigation Meters within Phase 2A (AFY)

- 0 - 10
- 11 - 50
- 51 - 145

Irrigation Meters not within Phase 2A (AFY)

- 0 - 10
- 11 - 50
- 51 - 145

- Existing Phase 1 Pipeline
- Planned Phase 2C Pipeline
- Castaic Lake Water Agency Service Area
- Newhall County Water District
- Santa Clarita Water Division
- Valencia Water Company
- Existing Parks and Golf Courses

N

0 1,000 2,000
Feet

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Recycled Water Master Plan
Santa Clarita, California

Alternative 1 - Phase 2C

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September 2016

Figure 7-4

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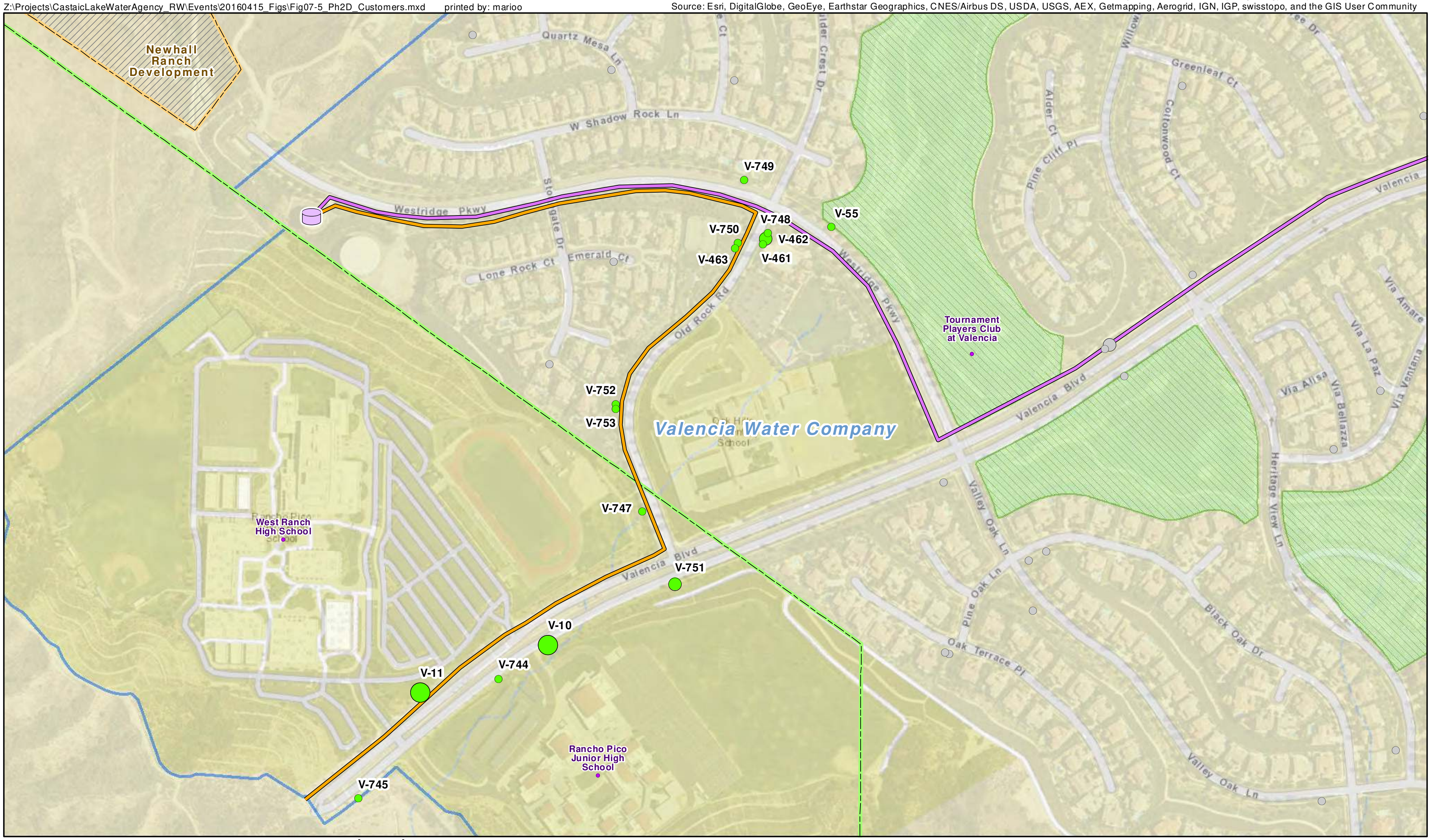

Legend

Irrigation Meters within Phase 2A (AFY)

- 0 - 10
- 11 - 50
- 51 - 145

Irrigation Meters not within Phase 2A (AFY)

- 0 - 10
- 11 - 50
- 51 - 145

- Existing Recycled Water Tank
- Existing Phase 1 Pipeline
- Planned Phase 2D Pipeline
- Castaic Lake Water Agency Service Area
- Valencia Water Company
- Planned Developments
- Existing Parks and Golf Courses

N

0 250 500
Feet

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Castaic Lake Water Agency
Recycled Water Master Plan
Santa Clarita, California

Alternative 1 - Phase 2D

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Figure 7-5

7.4 Alternative 2 – Non-Potable Reuse Expansion – Future Phases

Future recycled water use expansion beyond Phase 2 (shown in Figure 7-6) could include extensions off the Phase 2 alignments to utilize available effluent from the Valencia WRP or serving the Westside Communities development, which would use recycled water from the planned Newhall Ranch WRP supplemented by Valencia WRP recycled flows.

A summary of Alternative 2 key customers, anticipated annual demands, and construction completion dates and purveyors are listed in Table 7-3. A map of each Alternative 3 project is provided in Figure 7-7 through Figure 7-9. Appendix A lists the anticipated recycled water demands by meter and Appendix D summarizes the hydraulic modeling results. Costs are summarized in Section 7.9 and detailed cost sheets are provided in Appendix E.

Phase 2A + Future Expansion North - This project would expand the purple-pipeline network by branching off the Phase 2A system to construct four new pipeline alignments (Alignments E-H) to serve existing irrigation meters, as shown in Figure 7-7. This alternative would require upsizing the pipeline capacity of most of the Phase 2A pipeline to meet the demands for the identified customers. This alternative includes a 1 MG storage tank in Central Park associated with Phase 2A and four new pump stations (at Valencia WRP and along Alignments E, G and H). Appendix D provides additional information about the pumping requirements for each pump station.

- **Alignment E** – consists of a new transmission main that serves the Tesoro Del Valle development from a connection to the Phase 2A system at Newhall Ranch Road and runs along Copper Hill Drive. A new storage tank is provided in the biomedical park at Rye Canyon Loop.
- **Alignment F** – consists of a new transmission main from the intersection of Newhall Ranch Road and McBean Parkway, runs north on McBean Parkway, east on Decoro Drive, and terminates at Arroyo Seco Junior High.
- **Alignment G** – building off of Alignment F, consists of a new transmission main from the intersection of McBean Parkway and Decoro Drive, runs north on McBean Parkway, runs east on Copper Hill Drive, then terminates at a new storage tank at Kenton Lane.
- **Alignment H** – consists of a new transmission main from the terminus of Phase 2A, runs east on Newhall Ranch Road, south on Golden Valley Road, east on Soledad Canyon Road, south on Rainbow Glen Road, east on Avenue of the Oaks, and terminates at the Friendly Valley Golf Course.

Phase 2C + Future Expansion South - This project would expand the purple-pipeline network by branching off the Phase 2C system to construct four new pipeline alignments (Alignments A-D) to serve existing irrigation meters, as shown in Figure 7-8. This alternative would require upsizing the pipeline capacity of most of the Phase 2C pipeline to meet the demands for the identified customers. This alternative includes a 5 MG storage tank and two new pump stations (at Valencia WRP and along the Phase 2C alignment). Appendix D provides additional information about the storage and pumping requirements.

- **Alignment A** – consists of a new transmission main from the termination of Phase 2C to a new storage tank near the intersection of Placerita Canyon Road and Sierra Highway. The alignment runs east on 13th Street and Placerita Canyon Road.
- **Alignment B -** consists of a new transmission main from the termination of Phase 2C to a new storage tank in William S. Hart Park. The alignment runs south on Newhall Avenue.
- **Alignment C** – consists of a new transmission main from the intersection of McBean Parkway and Rockwell Canyon Road, runs west on McBean Parkway, south on the Old Road, west on Pico Canyon Road, and terminates at Whispering Oaks Drive.
- **Alignment D** – consists of a new transmission main loop encompassing Valencia Boulevard, McBean Parkway, and Arroyo Park Drive.

Westside Communities – for the purpose of this RWMP, the recycled water system is based on the information provided in the Valencia Water Company Recycled Water Master Plan for the Westside Communities (Dexter Wilson, 2015c). As shown in Figure 7-9, the system would include five storage tanks (ranging from 0.3 MG to 3.8 MG capacity) and seven pump stations, to serve pressure zones 1 through 5. Initially, recycled water would be provided by the Valencia WRP. Over time, recycled water produced at the planned Newhall Ranch WRP would be used to meet recycled water demands; however the projected buildout capacity of the Newhall Ranch WRP (approximately 4,200 AFY) would be insufficient to meet the total anticipated demand of the development (7,164 AF). Based on projected the monthly available supply, about fifty percent of the Westside Communities demand at buildout would be met by the Valencia WRP and the remaining fifty percent would be met by Newhall Ranch WRP.

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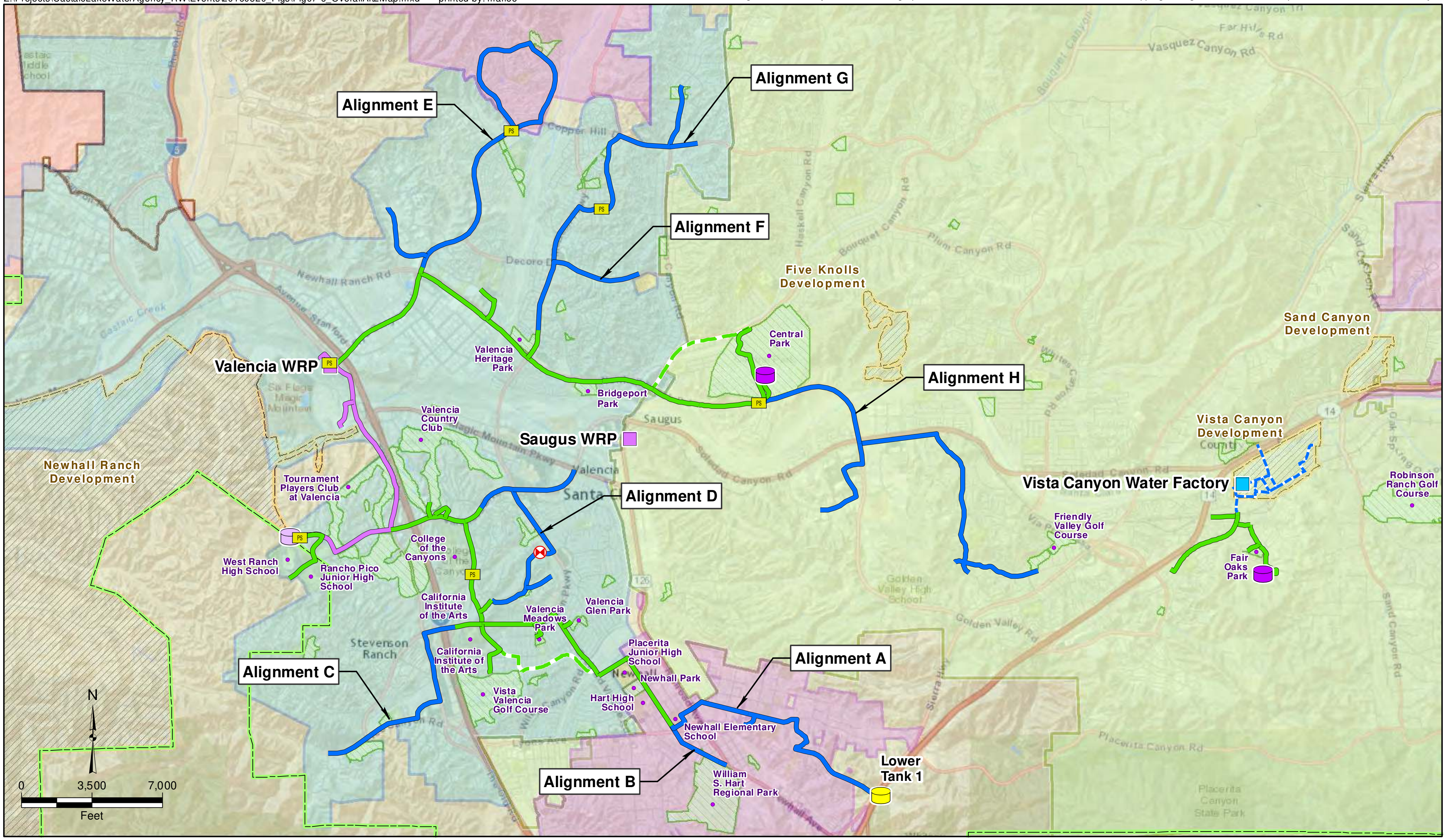

Legend

- Existing Water Reclamation Plant
- Planned Water Reclamation Plant
- Existing Recycled Water Tank
- Proposed Phase 2 Recycled Water Tank
- Proposed Phase 3 Recycled Water Tank
- Proposed Pump Station
- Proposed PRV
- Existing Phase 1 Pipeline
- Proposed Vista Canyon RW Pipeline
- Planned Phase 2 Pipeline
- Future Phase 3 Pipeline
- Planned Developments
- Existing Parks and Golf Courses
- Castaic Lake Water Agency Service Area
- Newhall County Water District
- LACWWD36
- Santa Clarita Water Division
- Valencia Water Company

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Castaic Lake Water Agency
Recycled Water Master Plan
Santa Clarita, California

Alternative 2
Non-Potable Reuse Expansion
(Future Phases)

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September 2016

Figure 7-6

Table 7-3: Summary of Alternative 2 - Demands and Customers

Alt 2 Projects	RW Demand (AFY)	Purveyor Demand (AFY)			Anticipated Construction Completion Date	Key Customers
		SCWD	VWC	NCWD		
Phase 2A + Future Expansion North	560 +1,344 1,904	643	1,041	220	2025	Phase 2A + Future Expansion North of the Santa Clara River (**Alignments E-H)**
Phase 2C + Future Expansion South	1,374 +1,017 2,391	0	1,719	672	2025	Phase 2C + Future Expansion South of the Santa Clara River (**Alignments A-D)**
Westside Communities	7,164	-	7,164	-	2024	Mission Village, Landmark Village, Entrada South, VCC (PM 18108), Homestead South, Legacy Village, Homestead North, Entrada North Potrero

Table 7-4: Summary of Alternative 2 Facilities

Alternative 2 - Facility Components	Alternative 2 - Non-Potable Reuse Expansion (Future Phases)		
	Phase 2A + Future Expansion North	**Phase 2C +** Future Expansion South	**Westside Communities**
Total Pipeline Length (feet)	121,200	83,900	161,300
Storage (MG)	1.0	5.0	8.3
Pump Station Total Flow (gpm)	8,000 1,100 1,000 1,900	2,000 5,200 - -	7 Pump Stations: 300 to 7600
Site Retrofit (# of Sites)	212	159	54

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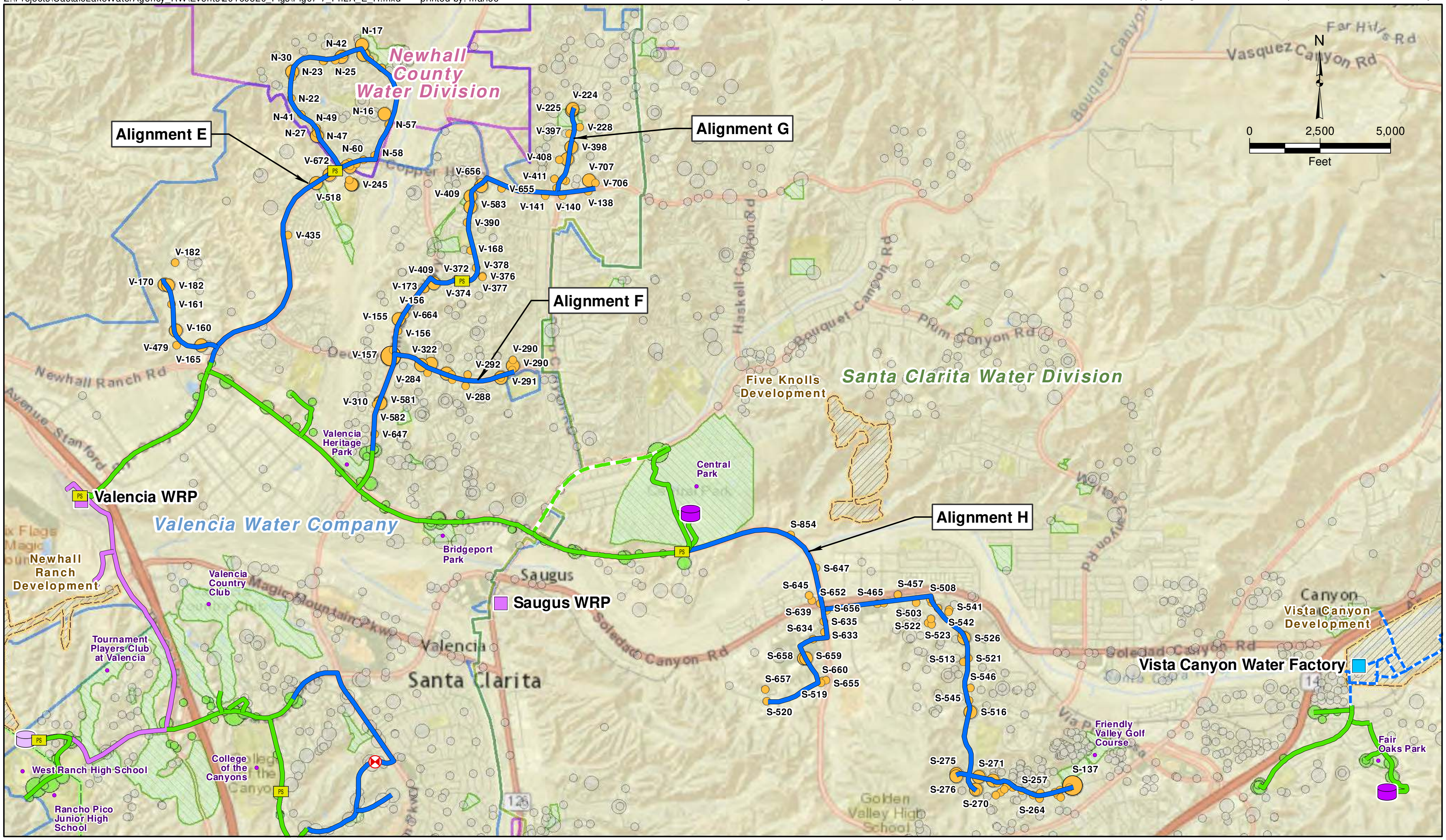

Legend

- Existing Water Reclamation Plant
- Planned Water Reclamation Plant
- Existing Recycled Water Tank
- Proposed Phase 2 Recycled Water Tank
- Proposed Pump Station
- Proposed PRV
- Existing Phase 1 Pipeline
- Planned Phase 2 Pipeline
- Future Phase 3 Pipeline
- Planned Developments
- Existing Parks and Golf Courses
- Castaic Lake Water Agency Service Area
- Valencia Water Company
- Newhall County Water District
- Santa Clarita Water Division

Irrigation Meters (AFY) within Phase 3E - H	Irrigation Meters (AFY) within Phase 2	Irrigation Meters (AFY) not within Phase 2 or Phase 3
0 - 10	0 - 10	0 - 10
11 - 50	11 - 50	11 - 50
51 - 88	51 - 145	51 - 145

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Alternative 2
Phase 2A + Alignments E-H

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Figure 7-7

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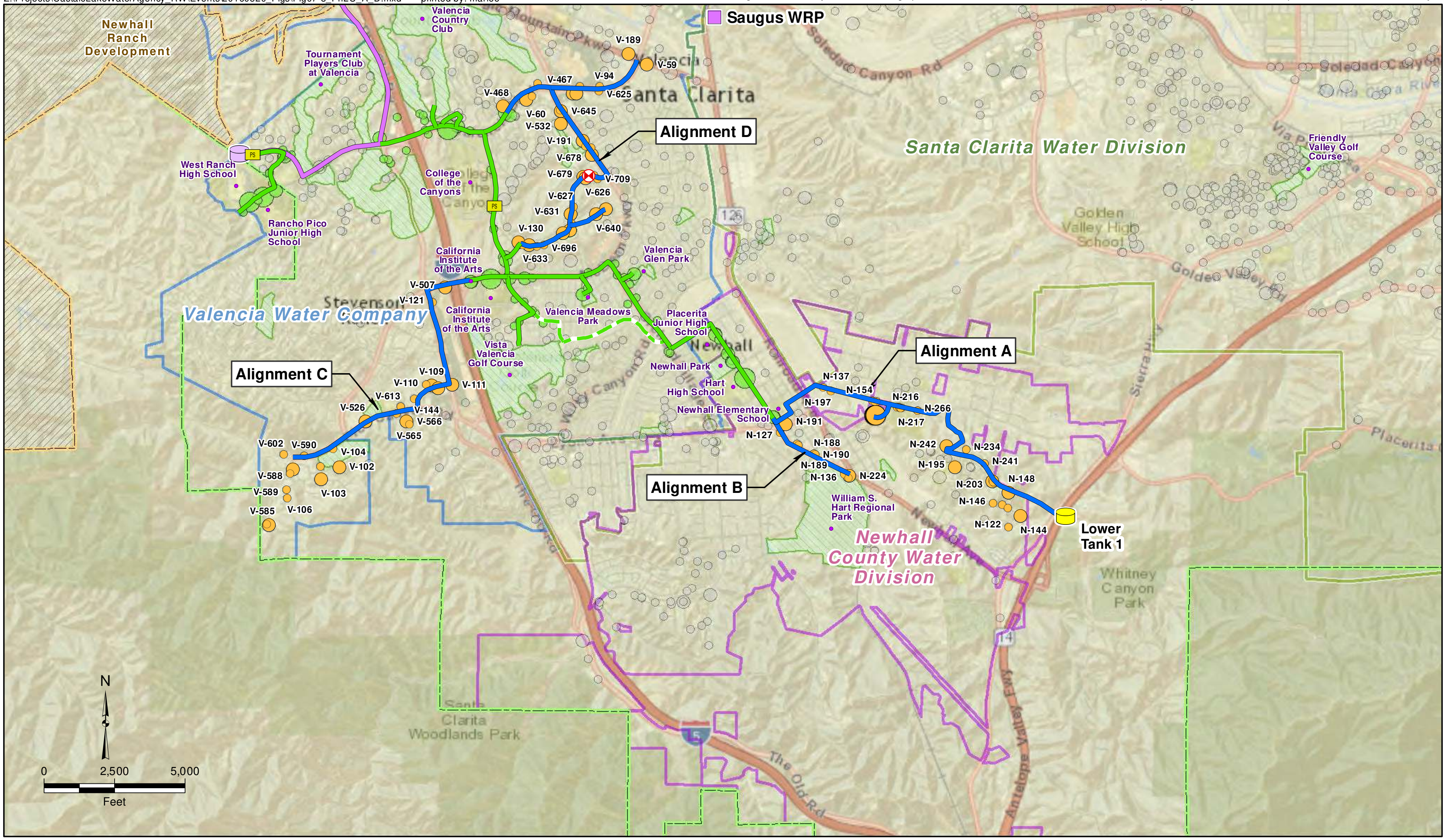

Legend

- Existing Water Reclamation Plant
- Existing Recycled Water Tank
- Proposed Phase 3 Recycled Water Tank
- Proposed Pump Station
- Proposed PRV
- Existing Phase 1 Pipeline
- Planned Phase 2 Pipeline
- Future Phase 3 Pipeline
- Planned Developments
- Existing Parks and Golf Courses
- Castaic Lake Water Agency Service Area
- Valencia Water Company
- Newhall County Water District
- Santa Clarita Water Division

Irrigation Meters (AFY) within Phase 3A -D	Irrigation Meters (AFY) USAGE_AFY	Irrigation Meters (AFY) not within Phase 2 or Phase 3
0 - 10	0 - 10	0 - 10
11 - 50	11 - 50	11 - 50
51 - 145	51 - 145	51 - 145

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Alternative 2
Phase 2A + Alignments A-D

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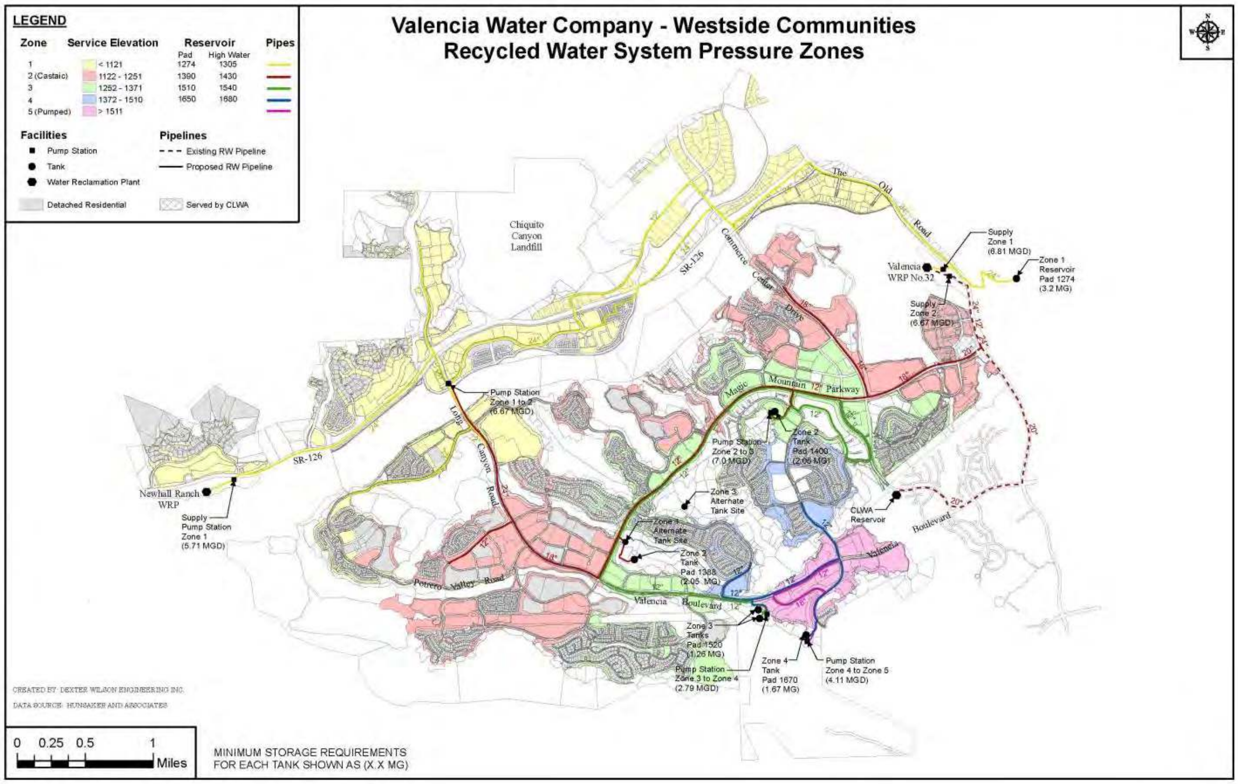
Figure 7-8

Figure 7-9: Alternative 2 – Westside Communities



Source: *Recycled Water Master Plan for Westside Communities (Dexter Wilson, 2015c)*

7.5 Alternative 3 – Groundwater Recharge via Surface Spreading

Alternative 3 includes five projects that use recycled water to recharge groundwater via surface spreading. Each project would extend off the Phase 2A system, and require upsizing the pipeline capacity of most of the Phase 2A pipeline to maximize deliveries of recycled water for non-potable use and to one or more spreading basin(s). For all of the Alternative 3 projects, the amount of recycled water that can be recharged is limited by the available supply, based on (1) wastewater generation and minimum discharge requirements as discussed in Section 4, (2) irrigation demands for Phase 1, Phase 2 and future customers using all available summer supplies that are not required for discharge and (3) operation of a recharge basin to prioritize stormwater capture, which limits the volume of recycled water that can be delivered in the winter months.

Utilizing recycled water from the Valencia WRP for surface spreading would require some additional treatment to meet the water quality objectives defined in the SNMP, particularly for sulfate (previously shown in Zone). Blending tertiary recycled water with the product water from the planned AWTF, currently being designed by the SCVSD as part of their Chloride Compliance Project, would effectively reduce sulfate concentrations to meet the water quality requirements. Based on discussions with SCVSD, a 70/30 blend of tertiary effluent to RO permeate from the AWTF is defined as the “Valencia Blend”. For the purpose of this evaluation, it is assumed that up to 5,000 AFY of Valencia Blend water may potentially be available to CLWA (at a higher cost than the tertiary recycled water) for surface spreading. The potential quantity of Valencia Blend for each Alternative 3 project is dependent on the recharge location.

- Spreading Site #1 would receive a 50/50 mix of tertiary and Valencia Blend water.
- Spreading Site #3a/b would receive 100% Valencia Blend water.
- A combined project serving Spreading Sites #1 and #3a/b would receive 100% Valencia Blend water at both sites.

During the summer months of Jun, July and August, when irrigation demand is high and there is little to no available supply for spreading, it is assumed that tertiary recycled water would be conveyed to Phase 2a irrigation customers. This would also allow for maintenance of the ponds while they are out of service. In all other months when recycled water is being sent to the spreading sites, Phase 2a irrigation customers would receive the Valencia Blend water - either at a 50/50 mix or 100% depending on the project.

The limitation for both spreading locations is the amount of available recycled water. If more recycled water were available, through future conservation efforts or other opportunities to free-up recycled water, the maximum recharge volume would then be limited by recycled wastewater contribution (RWC) and its closely related TOC requirement in the GRR. A more detailed discussion of the regulatory and water quality considerations and assumptions related to spreading restrictions is discussed in Section 2.3 of Appendix C.

It is assumed that each of the Alternative 3 projects would be implemented in partnership with the LACFCD to capture and recharge stormwater from the Santa Clara River during rain events. Anticipated stormwater recharge volumes are not included in estimated recharge volume for this study. Additional hydrologic studies would be needed to confirm the combined recycled water and stormwater recharge potential at each site.

Groundwater recharge would provide regional water supply stability and redundancy. In the past, east end purveyor groundwater wells have been shut-down during periodic times of drought. A GRR project presents an opportunity to improve water supply reliability in the eastern portion of purveyor service areas that are limited by imported water as a single source of supply. Additional hydrogeologic studies would be needed to confirm the groundwater management approach to optimize extraction of the recharged water and avoid mounding or daylighting of groundwater due to GRR project operations. Based on information on existing wells provided by NCWD and SCWD, it is assumed that the available capacity to extract groundwater would be sufficient to pump the anticipated volume of recycled water recharged for any individual Alternative 3 project; therefore no new extraction wells are included in these projects. However; the demand for the extracted water would need to be studied further to confirm the need and timing for extracting recharged water.

The availability of land for a spreading basin would need to be confirmed for both public and private parcels. The spreading basins could be designed to enhance passive recreation and habitat restoration, providing additional environmental and social benefits, which may align with other planned uses for the identified parcels.

Other qualitative benefits from a GRR project could include the ability to locally reuse the AWTF from the SCVSD Chloride Compliance Project, regional benefits to maintaining groundwater levels, the ability to use the groundwater basin to provide seasonal storage and providing redundancy by keeping aquifers full and creating an available emergency supply in case of SWP interruption. Additional analysis is needed to completely review all the qualitative benefits and adequately quantify these local reuse benefits.

A summary of key considerations for Alternative 3 projects is provided in this section and additional details about anticipated reuse volumes and facilities are listed in Table 7-5 and Table 7-6 respectively. Appendix D summarizes the hydraulic modeling results and costs are summarized in Section 7.9 with detailed cost sheets provided in Appendix E.

Table 7-5: Summary of Alternative 3 – Anticipated Irrigation and Recharge Volumes

Alt 3 Projects	Annual Irrigation Deliveries (Phase 2A) (AFY)	Initial Annual Recharge Volume [1] (AFY)	Ultimate Annual Recharge Volume [2] (AFY)	Average Annual Reuse [3] (AFY)	Anticipated Construction Completion Date
Phase 2A + Spreading Site #1	560	2,000	3,700	3,410	**2025**
Phase 2A + Spreading Site #3a	560	1,200	3,700	3,010	**2025**
Phase 2A + Spreading Site #3b	560	1,200	3,700	3,010	**2025**
Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	560	1,100	1,100	1,660	**2025**
Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)	560	2,000	3,700	3,410	**2025**

[1] The initial annual recharge volume is based on the 2025 available recycled water flows from the Valencia WRP, an initial maximum RWC of 20% (See Appendix C Section 2.3.8) and rain limitations which prioritize stormwater capture during rain events.

[2] The ultimate annual recharge volume is based on the 2050 available recycled water flows from the Valencia WRP and rain limitations which prioritize stormwater capture during rain events. The RWC does not limit recharge in 2050; rather the amount of recycle water available limits the ultimate recharge potential.

[3] Calculated as annual irrigation deliveries + average (initial 2025 recharge volume, ultimate 2050 recharge volume).

Table 7-6: Summary of Alternative 3 Facilities

Alternative 3 – Facility Components	Alternative 3 - Groundwater Recharge (Surface Spreading)				
	Phase 2A + Spreading Site #1	Phase 2A + Spreading Site #3a	Phase 2A + Spreading Site #3b	Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)
Recycled Water Quality for irrigation in summer	Tertiary in Summer (Jun-Aug)	Tertiary in Summer (Jun-Aug)	Tertiary in Summer (Jun-Aug)	Tertiary in Summer (Jun-Aug)	Tertiary in Summer (Jun-Aug)
Recycled Water Quality for spreading and irrigation in non-summer months	50% Tertiary 50% Blend	50% Tertiary 50% Blend	100% Blend	100% Blend	100% Blend
Total Pipeline Length (feet)	56,300	87,900	93,500	61,600	71,600
Spreading Basin Area (acre)	20	28	28	28	48
Storage (MG)	1.0	1.0	1.0	1.0	1.0
Pump Station Total Flow (gpm)	7,000	7,000	7,000	4,200	7,000
	-	7,000	7,000	4,200	4,200
	-	-	6,800	6,800	6,800
Site Retrofit (# of Sites)	51	51	51	51	51
Groundwater/Monitoring (# wells)	3	3	3	3	3

Phase 2A + Spreading Site #1 - this off-stream spreading site is located near the intersection of Whites Canyon Road and Via Princessa, on the south side of the Santa Clara River (Figure 7-10). The site is located on County-owned parcels, therefore the availability of this land and potential multi-use benefits would need to be confirmed and explored with Los Angeles County. Spreading site #3a was initially identified by the LACFCD in the *Santa Clara River Watershed Water Conservation Feasibility Study* (LACFCD 2007) as a potential location for in-river recharge. The Reconnaissance Study (Carollo 2015) provided an initial assessment of this site and Appendix C further analyzed the viability of recharging recycled water to meet the GRR Regulations. For this alternative project, a 24-inch-diameter pipeline and additional pumping capacity at the Valencia WRP would be required to convey a peak flow of 9.7 mgd to the spreading basin during the winter months on days when no rain is predicted. An inflatable dam diversion in the Santa Clara River would convey river water to a one acre settling basin, which would be hydraulically connected to a 20 acre recharge basin. The dam could be deflated during low flow periods and inflated when needed to capture anticipated storm flows. Recharged water would be extracted using existing SCWD and VWC wells. New monitoring wells would be installed to meet regulatory requirements.

Phase 2A + Spreading Site #3a - this in-stream spreading site is located upstream of Lang Station Road, near the intersection of Soledad Canyon Road and Antelope Valley Freeway (14) (Figure 7-11). The site is located on County-owned parcels, therefore the availability of this land and potential multi-use benefits would need to be confirmed and explored with Los Angeles County. A 24-inch-diameter pipeline, additional pumping capacity at the Valencia WRP and a booster pump station at or near Central Park would be required to convey a peak flow of 9.7 mgd to the spreading basin during the winter months on days when no rain is predicted. An inflatable dam diversion in the Santa Clara River would retain recycled water flows as well as some streamflow for recharge. The dam could be deflated periodically to allow deposited sediment to be transported downstream. It is assumed that when rain is predicted, recycled water deliveries would cease to free up capacity for stormwater capture. Recharged water would be extracted using existing SCWD and NCWD wells. New monitoring wells would be installed to meet regulatory requirements.

Phase 2A + Spreading Site #3b - this off-stream spreading site is located upstream of Site #3a, at the mouth of Bee Canyon (Figure 7-11). The site is located on privately owned parcels; therefore the availability of this land for purchase would need to be confirmed. Spreading Site #3b was added as an alternate location for the RWMP as part of the alternatives analysis due to concerns related to the viability of an in-stream basin. Spreading sites #3a and #3b s are assumed to have similar hydrogeologic characteristics and similar spreading areas (Figure 7-11), though additional modeling would need to be performed to confirm these assumptions. For this alternative project, a 24-inch-diameter pipeline, additional pumping capacity at the Valencia WRP and a booster pump station at or near Central Park would be required to convey a peak flow of 9.7 mgd to the spreading basin during the winter months on days when no rain is predicted. An inflatable dam diversion in the Santa Clara River at the mouth of Bee Canyon would be used to provide sufficient backwater to pump stormwater flow to the recharge basin during storm events. The dam could be deflated during low flow periods and inflated when needed to capture anticipated storm flows. It is assumed that when rain is predicted, recycled water deliveries would cease to free up capacity for stormwater capture. Recharged water would be extracted using existing SCWD and NCWD wells. New monitoring wells would be installed to meet regulatory requirements.

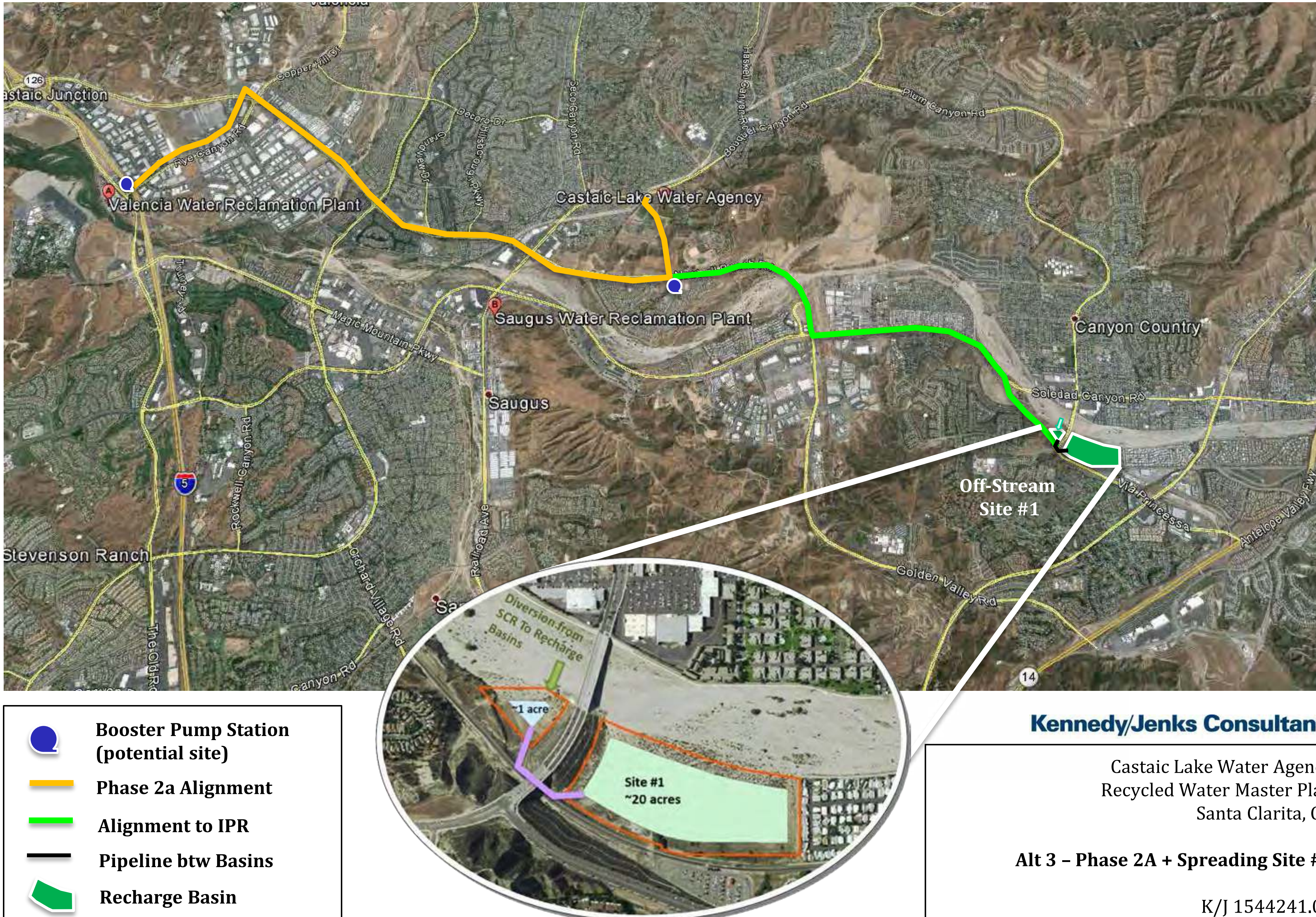
Phase 2A + Spreading Site #3b (Repurpose Infrastructure) – this project would seek to repurpose existing infrastructure to reduce costs and impacts associated with constructing new pipelines. Figure 7-12 illustrates two potential pipeline alignments that could be repurposed to convey recycled water to Spreading Site #3b:

(1) **Honby Pipeline**, which is approximately 5 miles of abandoned 14-inch diameter pipeline along Soledad Canyon Road (orange line on Figure 7-12). The condition of this pipeline is currently unknown. It is assumed that this pipeline could be repurposed to convey recycled water and if needed, the appropriate repairs were implemented (i.e. slip lining or replacing segments). The amount of recycled water delivered through the repurposed Honby Pipeline may be limited by the rehabilitated pipeline inside diameter, which could be 12-inch diameter or less, depending on the method used. For the purpose of this planning study, an internal diameter of 12 inch-diameter is assumed.

(2) **Honby Lateral**, which consists of a 30-inch-diameter segment (solid purple line on Figure 7-12), built in 2005, and a 33-inch-diameter segment (dashed purple line on Figure 7-12), built in 1978, crossing the Santa Clara River. The total length of these segments is approximately 6,000 feet. This alignment terminates near the Honby Pump Station/Sand Canyon Pump station at the corner of Santa Clara St and Furnivall Ave. This pipeline is currently being used to convey potable water, but would be available once the Honby Parallel is constructed. Design of the Honby Parallel is scheduled to begin in 2017. It is assumed that a short new section of pipeline on the east side would be required to connect to a new pump station facility for the recycled water project located at the Honby Pump Station site. Reuse of the Honby Lateral would eliminate the need for a new pipeline crossing the Santa Clara River.

Due to the smaller capacity in rehabilitated Honby Pipeline, the peak flow delivered to the Site #3b may be less than 3 mgd to the spreading basin during the winter months, which would reduce the annual recharge volume for this project to approximately 1,100 AFY. If additional recycled water supplies become available and demands for recycled water on the East side of the Santa Clarita Valley increase, an additional parallel pipeline could potentially be installed or the Honby pipeline could potentially be replaced to increase recycled water deliveries in the future. Costs are not provided for this future concept.

Phase 2A + Spreading Sites #1 and #3b (Repurpose Infrastructure) – this project would deliver recycled water to both Spreading Sites #1 and #3b for recharge and repurpose existing infrastructure to reduce costs and impacts associated with constructing new pipelines. Similar to the previously described project, the Honby Lateral and a portion of the Honby Alignment would be repurposed to convey recycled water to Spreading Site #3b. However, a new segment of 24-inch diameter pipe would be constructed from the Honby Pump Station to Spreading Site #1 to be able to maximize the annual recharge volume for this project. The spreading basins would be constructed and operated as described in the prior projects, with the exception that 100% Valencia Blend water would be the source water for both Spreading Sites #1 and #3b.



- Booster Pump Station (potential site)
- Phase 2a Alignment
- Alignment to IPR
- Pipeline btw Basins
- Recharge Basin
- SCR Diversion

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Alt 3 – Phase 2A + Spreading Site #1

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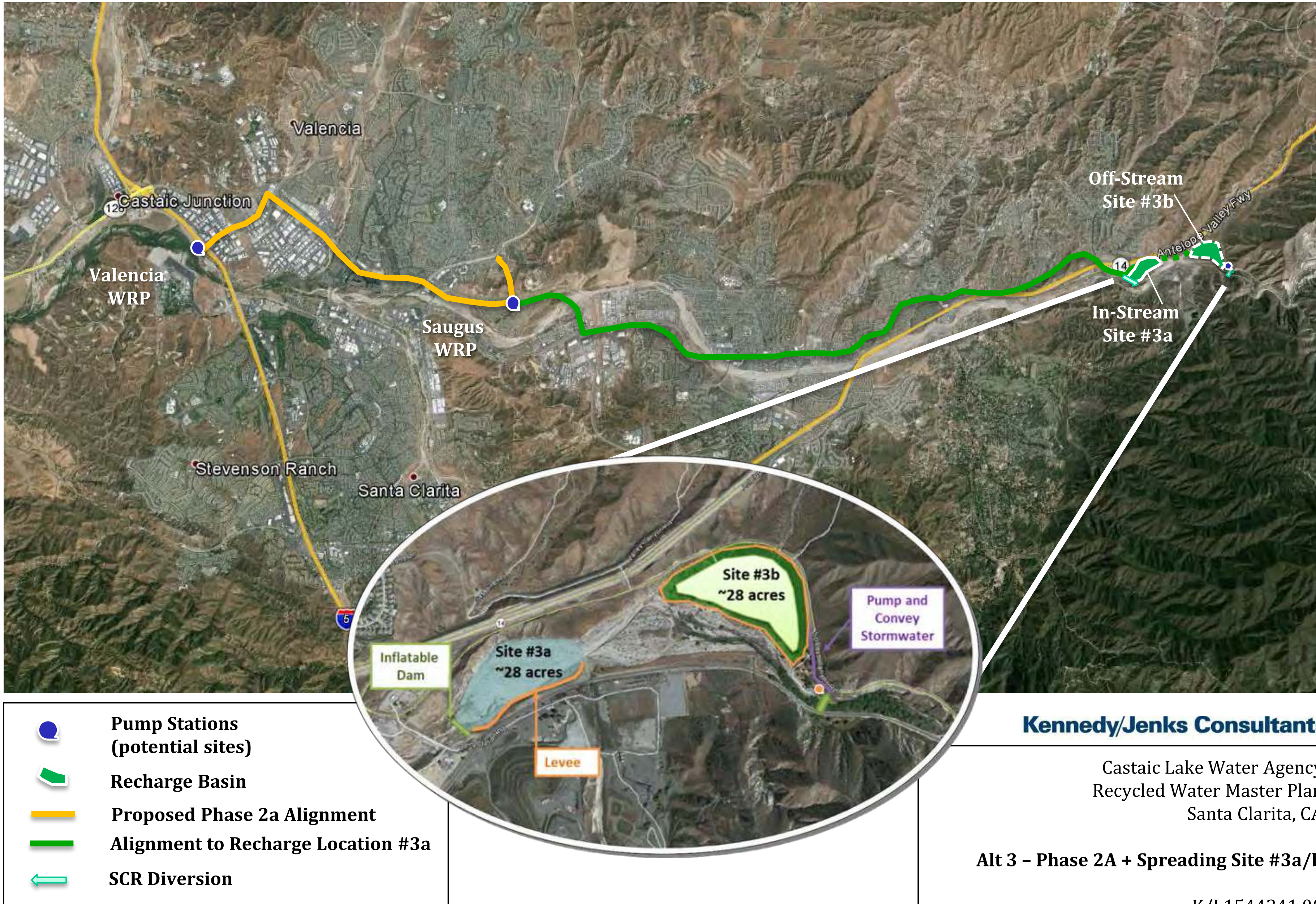
Figure 7-10



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Alt 3 – Phase 2A + Spreading Site #3a/b

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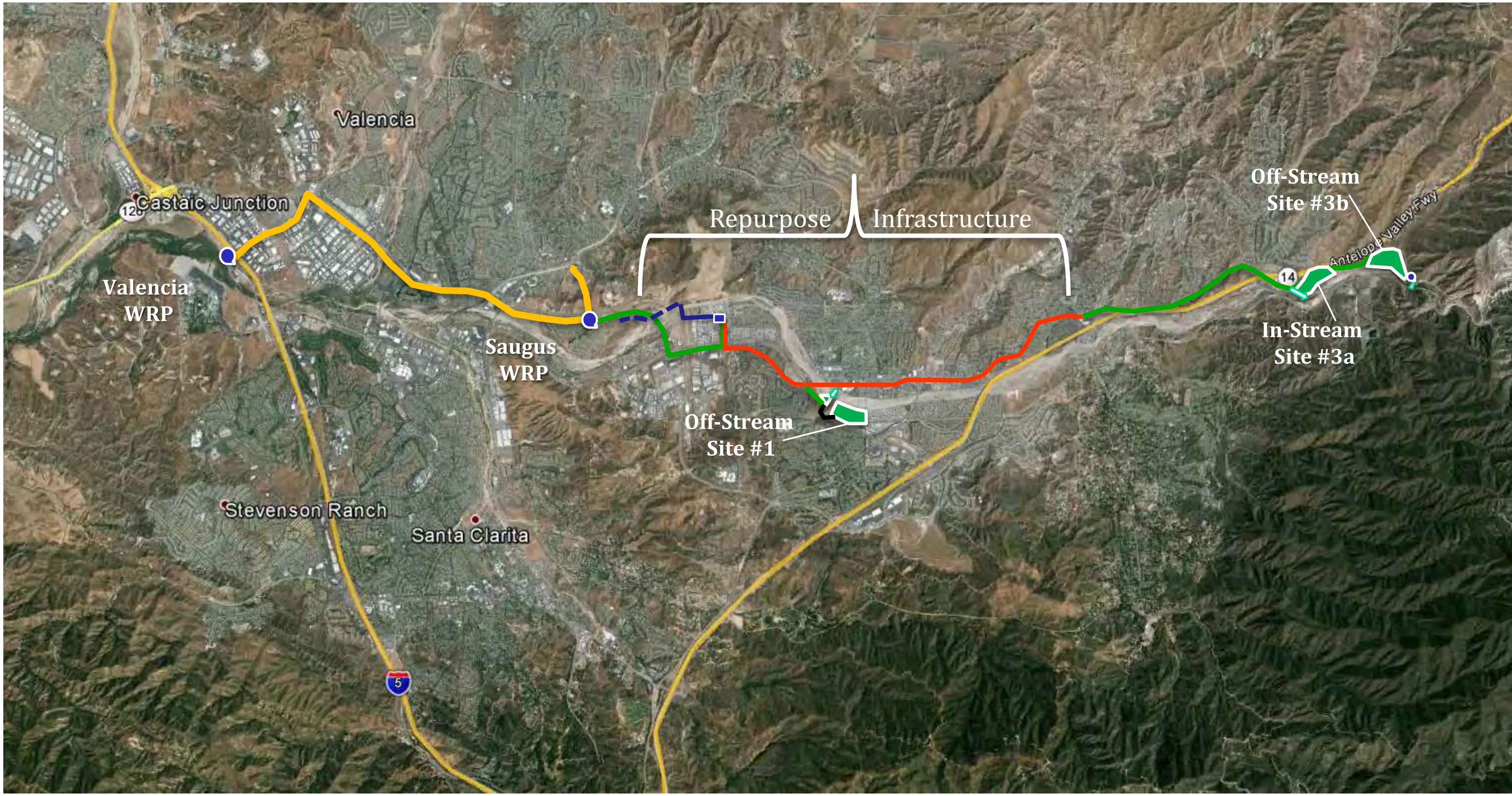
Figure 7-11



Pump Stations (potential sites)	Proposed Phase 2a Alignment
Existing Honby Pump Station	Alignment to Recharge Location #3
Recharge Basin	Existing 14"-dia Honby Pipeline
SCR Diversion	Existing 30"-dia Honby Lateral alignment
	Existing 33"-dia Honby Lateral alignment
	Pipeline between Basins

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Alt 3 – Phase 2A + Spreading Site #1 + Spreading Site #3a/b

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Figure 7-12

7.6 Alternative 4 – Advanced Treatment for Potable Reuse

Alternative 4 includes three projects that require advanced tertiary recycled water treated for potable reuse (1) direct injection into the groundwater basin, (2) surface water augmentation at Castaic Lake, and (3) direct potable reuse by blending with the raw water supply entering the Earl Schmidt Filtration Plant. Similar to Alternative 3, the amount of recycled water that can be advanced treated for potable reuse is limited by the available supply because irrigation demands for Phase 1, Phase 2 and future customers use all available summer supplies that are not required for discharge. However, since these projects would not be limited by stormwater capture prioritization, the total volume of water available in winter and shoulder months could be utilized. A more detailed discussion of the regulatory and water quality considerations is discussed in Sections 2.4, 3 and 4 of Appendix C.

Direct Injection –this project would deliver advanced-treated recycled water to the Saugus Formation in the vicinity of the Valencia WRP or other viable locations with the Valley, (as discussed in Section 6.2.1 and previously shown in Figure 6-3). It is assumed that an advanced water treatment facility (AWTF) for this project would be located at or near the Valencia WRP and would be similar to the SCVSD's Chloride Compliance Project treatment train, and would consist of MF, enhanced brine concentration (EBC), RO and UV for disinfection with the addition of high doses of advanced oxidation (AOP) to meet regulatory requirements for direct injection. (see Appendix C Section 2.4.2). New conveyance pipelines would be constructed to deliver the advanced-treated recycled water to seven new injection wells (locations and alignments were not identified for this project) and truck hauling would be the method used for brine disposal. Additional hydrogeologic analysis is necessary to identify the preferred placement of injection wells to achieve a travel time of 6-months before extraction of recharged water using existing wells.

Surface Water Augmentation –this project would deliver advanced-treated recycled water to augment surface water stored in Castaic Lake. The treatment train would be similar to the process suggested for direct injection (described above and in Appendix C Section 2.4.2) and it is assumed that the AWTF would be located either at the Valencia WRP or at the Earl Schmidt Filtration Plant. New conveyance pipelines and would constructed to deliver the advanced-treated recycled water to Castaic Lake near the boat ramp, as shown by the solid line on Figure 7-13. The dashed pipeline extension would only be constructed if required by DDW to increase retention time; however, even with this extension the theoretical retention time would be less than 6 months and thus this project would not qualify under the current draft regulations. Additional hydrodynamic modeling and operational studies would be necessary to confirm the permittability of this project.

Direct Potable Reuse –this project involves sending the advanced treated water from Valencia WRP to the Rio Vista Water Treatment Plant for further treatment prior to distribution. The treatment train would be similar to the process suggested for direct injection and SWA (described above and in Appendix C Section 2.4.2) with the addition of ozone and BAC pre-treatment to offer two new and different mechanisms to control the wide diversity of potential chemical and microbiological threats. It is assumed that the AWTF would be located either at the Valencia WRP or at the Rio Vista Water Treatment Plant. Figure 7-14 shows the conveyance concept, which would require 24-inch-diameter pipeline and additional pumping capacity at the Valencia WRP to convey a peak flow of 9.7 mgd to the Rio Vista Water Treatment Plant. It is important to note that this alternative is speculative as there is neither a developed framework for regulations nor any established timeframe for promulgating DPR regulations.

Additional details about anticipated reuse volumes and facilities are listed in Table 7-7 and Table 7-8 respectively and costs are summarized in Section 7.9 with detailed cost sheets provided in Appendix E.

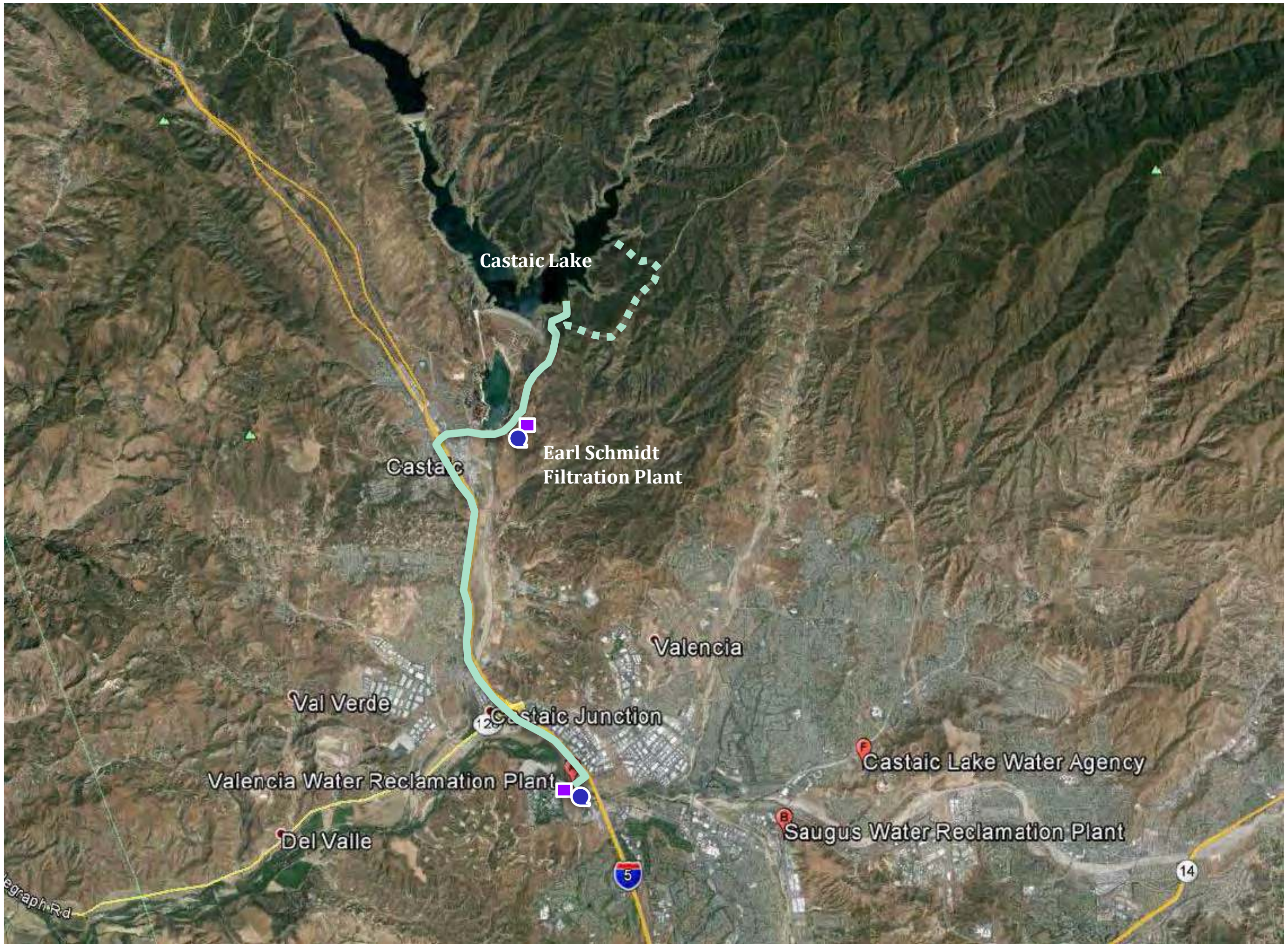
Table 7-7: Summary of Alternative 4 – Anticipated Use of Advanced Treated Water

Alt 4 Projects	Annual Irrigation Deliveries	Average Annual IPR/DPR	Total Average Annual Reuse	Peak Delivery	Anticipated Construction Completion Date
	(AFY)	(AFY)	(AFY)	(mgd)	
Direct Injection	0	4,250	4,250	9.7	**2026**
Surface Water Augmentation	0	4,250	4,250	9.7	**2032**
Direct Potable Reuse + Phase 2A	560	4,250	4,810	9.7	**2037**

Table 7-8: Summary of Alternative 4 Facilities

Alternative 4 – Facility Components	Alternative 4 - Advanced Treatment for Potable Reuse		
	Direct Injection	Surface Water Augmentation	Direct Potable Reuse +Phase 2A
Advanced Water Treatment Facility Capacity (mgd)	9.7	9.7	9.7
Pipelines	6,100	45,000	37,900
Storage (MG)	-	-	6.0
Pump Stations Total Flow (gpm)	7,000	7,000	7,000
	7,000[1]	7,000	6,000
Groundwater/Monitoring (# wells)	10	-	-
Discharge Facility (mgd)	-	4.9	-
Site Retrofit (# of Sites)	-	-	51

[1] Represents seven 1,000 gpm pump stations at each injection well



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Advanced Water Treatment Facility (potential sites)

Pump Stations (potential sites)

Pipeline alignment (Valencia WRP to Lake)

Pipeline extension (to increase retention time)

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Alt 4 – Surface Water Augmentation

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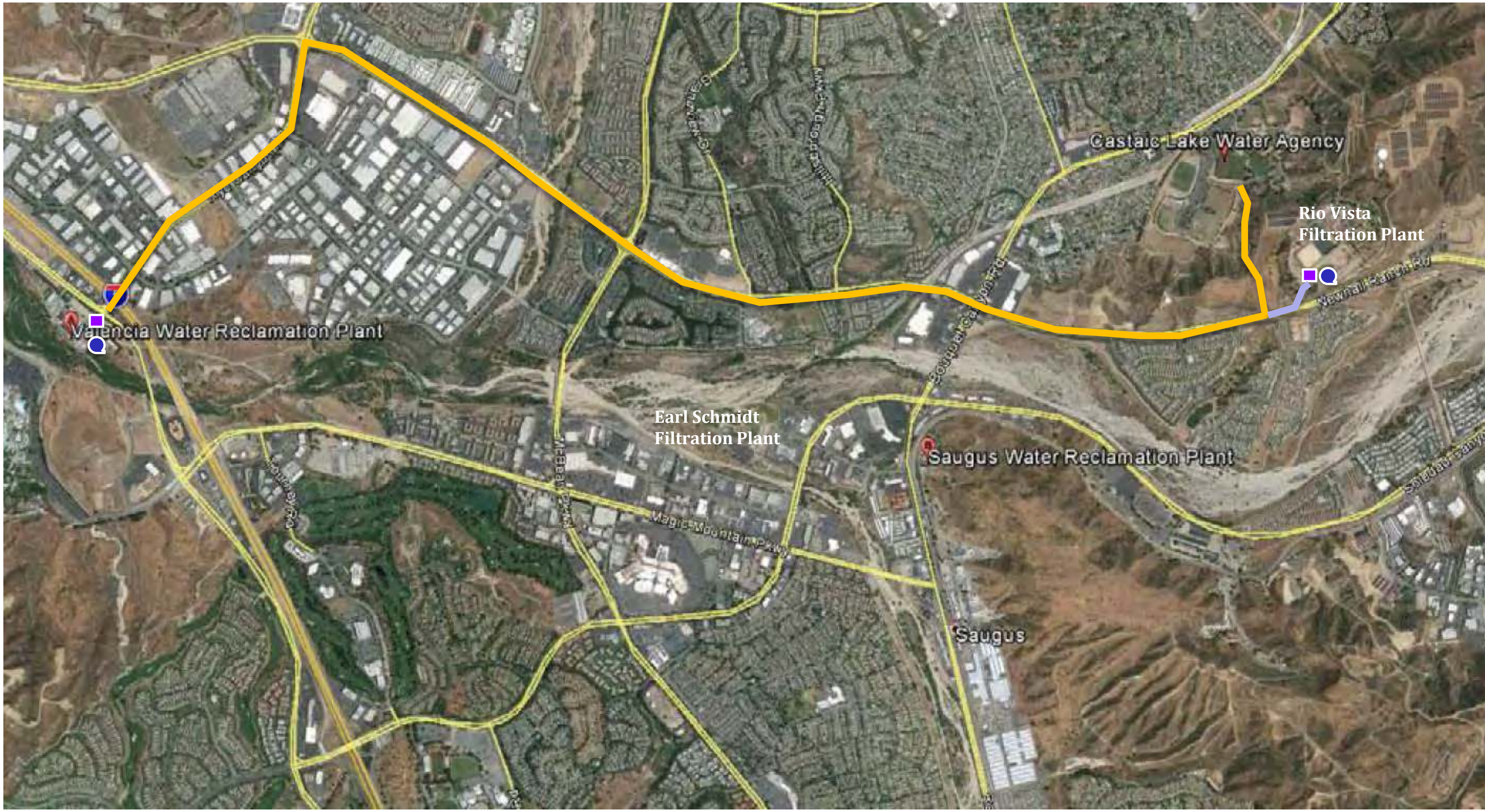
Figure 7-13



Advanced Water Treatment Facility (potential sites)

Pump Station (potential sites)

Proposed Phase 2a Alignment

Short extension to AWPC

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Alt 4 – Direct Potable Reuse

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Figure 7-14

7.7 No Project Alternative

The No Project Alternative would include the continued operation and maintenance of CLWA's existing Phase 1 recycled water system with the potential to increase non-potable reuse through the addition of infill customers located near existing recycled water pipeline alignments. No new major conveyance infrastructure would be constructed, though small service laterals could be installed to connect identified infill customers to the recycled water distribution system. CLWA is currently in communication with potential customers to increase the Phase 1 deliveries by 40 AFY. It is possible that the additional Phase 1 demand could be as high as 100 AFY if the majority of nearby customers are converted to recycled water in the future.

7.8 Other Considerations

Repurposing Existing Infrastructure

CLWA and the purveyors have identified some existing assets that could be repurposed for recycled water. For the purpose of the RWMP and associated programmatic EIR, the alternatives presented in the prior sections assume construction of new facilities (with the exception of the last two projects in Alternative 3). This section summarizes potential opportunities and challenges to repurpose the following stranded or underutilized assets, described below and shown in Figure 7-15. With all of these facilities, additional investigations and studies are required to ascertain the viability of repurposing them for use with the future recycled water system.

Groundwater transmission main: There is an existing unutilized16-inch to 20-inch treated groundwater transmission main that extends from a groundwater treatment facility on Bouquet Canyon Road near Newhall Ranch Road to the intersection of Newhall Ranch Road and Santa Clarita Parkway. This pipeline can potentially be repurposed as part of the Phase 2A system.

Honby Lateral: The Honby Lateral is a 30-inch to 33-inch pipeline that crosses the Santa Clara River at Golden Valley Road. The pipeline can potentially be repurposed as part of Alternative 2 or Alternative 3. However, the planned 60-inch Honby Parallel Pipeline must be installed prior to repurposing the Honby Lateral, so that CLWA's transmission system remains connected.

Honby Pipeline: The 14-inch Honby Pipeline extends from the Honby Pump Station, located near the intersection of Santa Clara Street and Honby Avenue, traverses Soledad Canyon Road and terminates at Sand Canyon Road. The pipeline, originally built by NCWD, has been inactive since the CLWA Sand Canyon Pipeline was built. It can potentially be repurposed as part of Alternative 3. The 'Phase 2A + Spreading Site #3b' and 'Phase 2A + Spreading Sites #1 and #3b' options specifically integrate the Honby Lateral and Honby Pipeline.

Honby Pump Station: Similar to the Honby Pipeline, the Honby Pump Station was originally for the NCWD distribution system and has been inactive since the CLWA Sand Canyon Pump Station was built, adjacent to the Honby Pump Station. A rehabilitation assessment of the pump station (Lee & Ro, 2009) determined that it was feasible to repurpose the pump station for use in a recycled water system. The pump station can potentially be used as part of Alternative 2, specifically with Alignment H, or Alternative 3.

■	Existing Honby/Sand Canyon Pump Station
▬	Existing 16"-dia Groundwater Transmission Main
▪▪▪▪	Existing 20"-dia Groundwater Transmission Main
▬	Existing 14"-dia Honby Pipeline
▬	Existing 30"-dia Honby Lateral Alignment
‒ ‒ ‒	Existing 33"-dia Honby Lateral Alignment

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Potential Reuse of Existing Infrastructure

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Figure 7-15

Seasonal Storage

To maximize unused water supply in the winter months when demand is lower, water can be stored for use in the summer months when demand is higher. This is known as seasonal storage. Based on the evaluation of monthly supply of recycled water, less recharge to the Santa Clara River and once irrigation demands utilize all available summer supply, there would be approximately 5,500 AFY of recycled water available to store seasonally in the year 2050 to allow for further expansion of recycled water for irrigation. Note that this is the same volume considered to be available for potable reuse in Alternatives 3 and 4. Nine reservoirs (Figure 7-16) within the CLWA service area were identified as potential sites for seasonal storage. Concept level estimates of storage capacity, operational capacity, dam height and crest length are summarized in Table 7-9. This table also shows a very high-level estimate of construction costs for the dam based on concept level dam dimensions and cost curves for cubic-yards of roller-compacted concrete. It is also important to note that groundwater recharge can also assist with offsetting the seasonal storage volumes.

Figure 7-16: Potential Seasonal Storage Sites

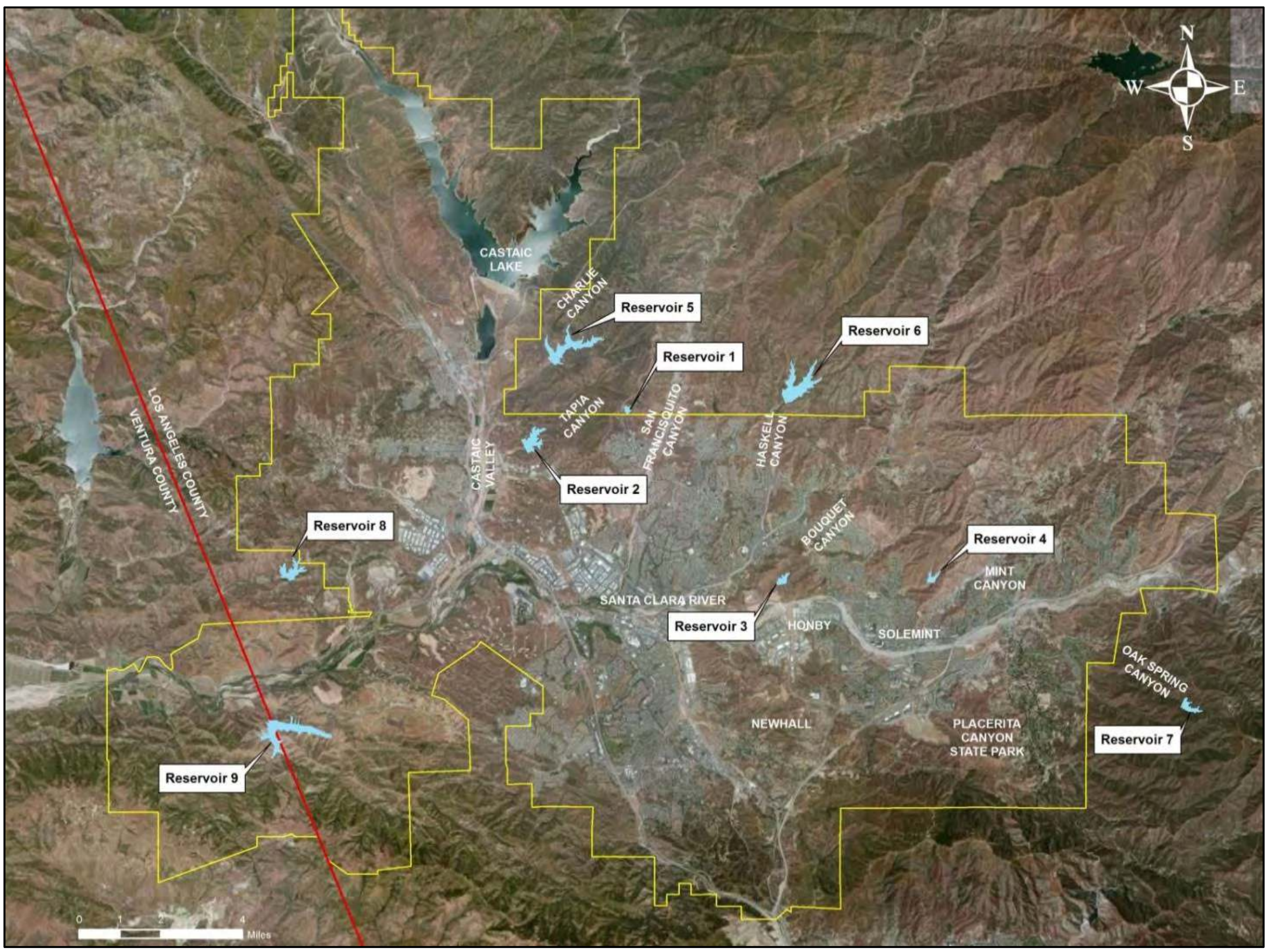


Table 7-9: Summary of Potential Seasonal Storage Sites

Reservoir #	Storage Capacity (AFY)	Operational Capacity (AFY)[a]	Estimated Height of Dam (feet)	Estimated Crest Length of Dam (feet)	Concept-Level Costs for Dam Construction ($mil)
1	1,000	850	150	800	$79
2	4,760	4,160	160	1,730	$252
3	1,160	920	110	1,090	$94
4	1,110	920	140	780	$80
5	7,890	6,430	150	1,050	$158
6	9,240	7,580	150	1,130	$155
7	1,870	1,600	150	900	$115
8	3,710	3,320	150	640	$68
9	9,930	8,430	150	930	$133

[a] Operational Storage assumes that maximum draw down for each reservoir is 50% of depth.

A high level evaluation of the reservoir sites is presented in Table 7-10. Only three reservoirs (1, 2, and 8) are close to the Valencia WRP and Reservoir 7 is the furthest from the plant. Four reservoirs (5, 6, 8, and 9) have low relative cost, two with medium relative costs, and four have high relative costs. Four reservoirs (2, 3, 4, and 9) are located inside the CLWA boundary and five reservoirs are located outside of the CLWA service area. Only Reservoirs 5 and 9 are not within close proximity to existing potential users; however, Reservoir 9 is within a planned development (Westside Communities) which could share the cost of a pipeline from the reservoir to the WRP.

Table 7-10: Storage Reservoirs Evaluation Matrix

Reservoir #	Distance to Valencia WRP[a]	Relative Unit Capital Cost[b]	Service Area	Proximity to Users	Within Planned Development
1	Low	High	Outside	Yes	No
2	Low	Medium	Inside	Yes	No
3	Medium	High	Inside	Yes	No
4	Medium	High	Inside	Yes	No
5	Medium	Low	Outside	No	No
6	Medium	Low	Outside	Yes	No
7	High	Medium	Outside	Yes	No
8	Low	Low	Outside	Yes	No
9	Medium	Low	Inside	No	Yes

[a] Distances less than 5 miles are designated as "short," distances longer than 5 miles and shorter than 10 miles are designated as "medium," and distances longer than 10 miles are designated as "long"
[b] Unit Capital Costs (Capital $/Operational Storage AFY); less than $25,000 are designated as "low," costs higher than $25,000 and lower than $75,000 are designated as "medium," and costs higher than $75,000 are designated as "high"

Reservoirs 2, 6, 8, and 9 have the best combination of a short distance to the Valencia WRP, low relative cost, within the CLWA service area, close proximity to existing potential users, and within a planned development. None of these four reservoirs has the highest rating in all the categories.

Should seasonal storage be identified as a desirable option to pursue in the future, CLWA and the purveyors would need to explore the feasibility of these sites in greater detail. The feasibility of surface seasonal storage would depend on availability of land, construction costs for reservoir, pipelines and pump stations to fill the reservoir, conveyance costs to serve new customers, permitting and environmental mitigation costs, water quality requirements, public acceptance, and ability to finance.

Customer Retrofits

Most of the landscape irrigation systems in the Santa Clarita Valley are metered separately from the potable system and could be retrofitted to receive recycled water by following the guidelines in Title 17 of the California Code of Regulations (CCR). Mixed meters that serve both the irrigation and potable system are more complex to retrofit; however for larger users such as schools or commercial/industrial areas with significant landscaping demands, it can still be cost effective. Existing buildings that have not been constructed with dual-plumbing systems can be complex and expensive to retrofit , and therefore, such sites would only be considered potential customers if a high demand use, such as a cooling tower which can be easily separated from the potable water system.

For the purpose of the alternatives analysis, the following assumptions were made:

- Existing irrigation lines would be retrofitted to connect to a recycled distribution main.
- For retrofits, meter capacity would be sized to match existing or sized to accommodate historical water use.
- Design of irrigation facilities would include isolation of existing service, cross-connection prevention, and proper tag identification to properly execute a conversion from an irrigation system served by potable water to one served with recycled water.
- Unit costs for retrofits were developed using a cost equation based on the irrigated area in square feet per a retrofit study conducted for the VWC (Dexter Wilson 2012) which was deemed conservative for planning a large scaled recycled water system.

7.9 Engineers Opinion of Probable Costs

The engineer's opinion of probable cost is based on a conceptual level estimate of the capital and operating costs for each alternative considered for the RWMP. Planning-level opinions of capital, operations and maintenance (O&M), and lifecycle costs are developed to facilitate an economic comparison of the projects within each alternative. Capital and operating costs are estimated for each alternative at a Class 5 level representing Planning to Feasibility level information with an estimated accuracy range between -30 percent and +50 percent, using assumptions stated herein. Costs then are converted to annualized lifecycle costs using basic assumptions about discount rates and life expectancy of project components. Total costs are divided by the recycled water delivery over the life of the project to obtain a uniformly derived unit cost of water in dollars per acre-foot ($/AF). Appendix E includes detailed opinions of probable cost for each alternative. Non-quantitative costs and other qualitative benefits are not included herein, but may be beneficial to assess in greater detail, particularly for Alternative 3 and Alternative 4 projects that may provide environmental and community benefits beyond offsetting potable supplies.

Capital Costs

The following assumptions are applied to estimate facility costs:

- **Distribution Pipelines:** Pipeline costs are based on a unit cost for each pipe size (i.e. dollar per inch-diameter linear foot) using conventional dry trenching techniques based on recently bid projects and professional experience. Costs include material and labor for total pipe segment. Special crossings, such as major intersections and jack-and-bore for river crossings are included at a higher unit cost.
- **Pump Stations:** Pumping costs were estimated based on brake horsepower requirements, assuming a redundancy factor, and outside pumps with an enclosed control building. Land acquisition costs for pump stations were not included in the cost estimate.
- **Operational Storage:** The unit cost for storage tanks (concrete and steel) is based on cost curves from RS Means, recently constructed projects in California and from professional experience.
- **Spreading Basins:** Constructing earthen off-stream storage ponds are estimated at approximately $30,000 per AF of storage created. Construction of levees for in-stream storage ponds is based on a unit cost per linear foot a typical level with 3:1 horizontal to vertical slope that is 5 to 8 feet tall. Land purchase costs are assumed for the privately owned parcels at Site #3b, but not for County owned parcels at Sites #1 and #3a.
- **Advanced Water Treatment Facility:** Cost estimates for AWTF treatment trains were based on information provided in the SCVSD Chloride Compliance Project EIR. These costs represent processes selected to minimize brine generation, including an RO train with an anticipated recovery of 99%.
- **Site Retrofit Costs:** As described in Section 7.8, unit costs for retrofits were developed using a cost equation based on the irrigated area in square feet.
- **Wells:** Estimated cost for monitoring wells include cost for drilling and construction of a 400-600 feet monitoring well based on recent project experience. Costs for new extraction wells are not included since existing wells are assumed to be sufficient to extract recharged water.
- **Inflatable Rubber Dam:** To facilitate stormwater capture of river flows with operational flexibility to periodically flush sediment, an inflatable rubber dam is proposed and a unit cost per linear foot developed based on project data in California. The cost assumes materials and installation of a rubber bladder, foundation and necessary control features.
- **Discharge Facility:** Based on a unit cost for a standard bank outfall with erosion protection and energy dissipation.
- **Repurposing Existing Infrastructure:** There is considerable uncertainty related to the capital costs required to repurpose existing infrastructure. For the purposes of this high level cost estimate, it is assumed that abandoned pipelines would require slip lining with HDPE pipeline, receiving/insertion pits every 1,000 linear feet and a reduction in the inside diameter to withstand pumping pressure. Costs for repurposing infrastructure that is currently in use is assumed include appurtenances and new pipeline extensions as needed.

The following allowances, contingencies and non-contract cost percentages are applied to the **Subtotal Facility Costs:**

- **Additional Facility Capital Costs:** The following percentages are applied to subtotal of treatment, pump station, storage and discharge costs: site development costs at 5%, yard piping at 5% and Electrical, Instrumentation and Controls (I&C), and Supervisory Control And Data Acquisition (SCADA) Control at 25%.
- **Taxes:** 9% is applied to materials (estimated at 40% of the total facility cost).

The following allowances, contingencies and non-contract cost percentages are applied to the **Subtotal Additional Facility Costs**:

- **Allowance for Unlisted Items:** A markup of 5% for mobilization, bonds and permits and 15% for Contractor Overhead and Profit are applied to the subtotal additional facility capital costs.
- **Estimate Contingency:** A markup of 30 percent of the total Subtotal Cost was added to pay contractors for overruns on quantities, changed site conditions, change orders, etc. Contingencies are considered as funds to be used after construction starts and not for design changes or changes in project planning.

The resulting **Subtotal with Contractor Markups and Contingency** is increased by 2% per year to reflect escalation to midpoint of construction based on project implementation timeline assumptions. **The Project Capital Cost** includes all facility costs, allowances, markups, contingencies and the escalation to the midpoint of construction.

O&M Costs

Operations and maintenance (O&M) costs are estimated to include the following items:

- Energy costs for pumping based on a unit cost for electricity based on commercial electricity rates in Santa Clarita at $0.12/kWh
- Advanced Water Treatment Facility costs, including energy, labor, chemicals, materials and replacement costs by process type based on average operating flow over the year (as dictated by each Alternative) based on the SCVSD Chloride EIR for near zero discharge system, including brine disposal facilities
- Maintenance Costs based on 5% of direct facility costs for pipelines, injection and monitoring wells, including a 10% contingency
- Labor Costs based on full time salary of $100,000 per year

O&M costs also include costs for purchasing recycled water from SCVSD based on:

- Tertiary RW Rate = $200/AF
- AWTF product water (MF/RO) Rate = $1,430 (based on preliminary estimate from SCVSD)
- Valencia Blend Rate = $569 (based on a 70:30 blend of Tertiary: AWTF flow)

For Alternative 1 and 2 projects only tertiary recycled water would be purchased to serve non-potable demand. For Alternative 3, a blend of tertiary and Valencia Blend water would be purchased depending on the recharge location. Spreading Site #1 would receive a 50/50 mix of Valencia tertiary and Valencia Blend water. Spreading Site #3a/b would receive 100% Valencia Blend water. Flows during the peak summer months, for Alternative 3, would be tertiary recycled water since all available supplies would be used to serve non-potable demands. Alternative 4 would only purchase tertiary recycled water because all additional treatment would occur at the AWTF.

Annualized Unit Costs

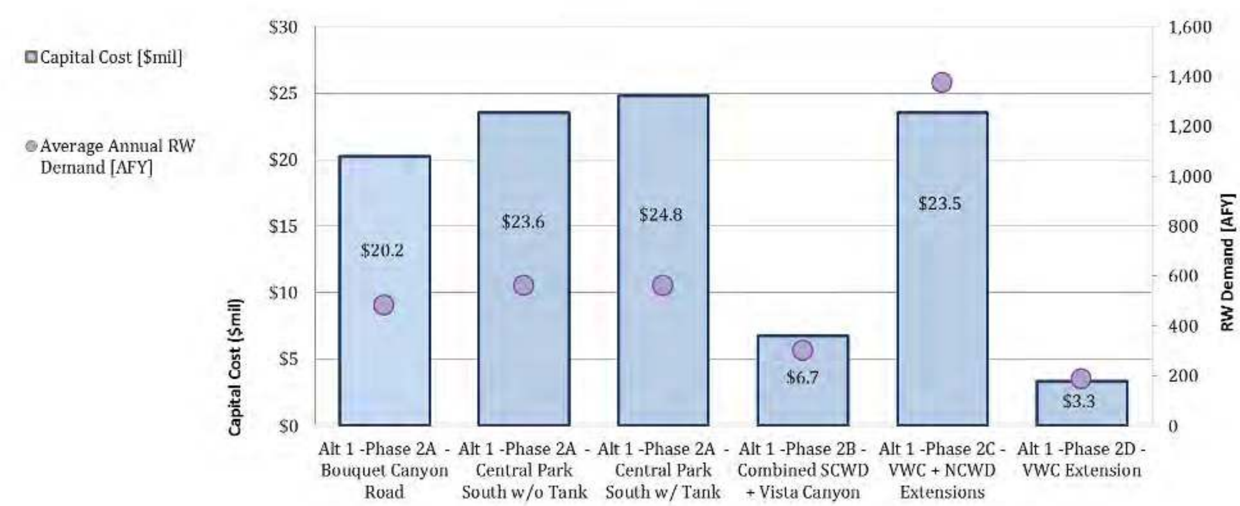
An annualized unit cost is developed for each alternative to compare the cost per acre foot to build and operate a given project. An annualized capital cost is calculated based on a project life of 30 years and an interest rate of four percent. The annualized capital cost is added to the annual O&M costs to estimate the total cost per year to construct and operate the project over the life of the project. The annual cost per year is then divided by the average annual volume of recycled used over the life of the project to calculate an annualized unit cost per acre foot.

Summary of Capital, O&M and Annualized Unit Costs

The engineer's opinion of capital, O&M and annualized unit costs for each alternative are summarized in Figure 7-17 through Figure 7-20. An overall summary of demands and costs for Alternatives 1-4 is provided in Table 7-11.

Figure 7-17: Summary of Costs for Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

Facility Component	Alternative 1 - Non-Potable Reuse Expansion (Phase 2)					
	Phase 2A			Phase 2B	Phase 2C	Phase 2D
	Bouquet Canyon Road	Central Park South w/o Tank	Central Park South w/ Tank	Combined SCWD + Vista Canyon	VWC + NCWD Extensions	VWC Extension
Pipelines	$14,670,000	$17,030,000	$17,020,000	$2,650,000	$15,380,000	$1,500,000
Storage or Hydro-pneumatic Tank	$480,000	$480,000	$1,730,000	$2,510,000	$0	$0
Pump Station	$3,110,000	$3,680,000	$3,680,000	$810,000	$5,120,000	$1,260,000
Site Retrofit Costs	$1,950,000	$2,360,000	$2,360,000	$750,000	$3,010,000	$560,000
Total Construction Cost ($)	**$20,210,000**	**$23,550,000**	**$24,790,000**	**$6,720,000**	**$23,510,000**	**$3,320,000**
Estimated Construction Cost ($mil)	**$20.2**	**$23.6**	**$24.8**	**$6.7**	**$23.5**	**$3.3**
Annualized Cosntruction Cost ($mil/yr)	$1.2	$1.4	$1.4	$0.4	$1.4	$0.2
Ave Annual Demand (AFY)	482	560	560	300	1,374	186
Annualized Unit Cosntruction Cost ($/AF)	**$2,400**	**$2,400**	**$2,600**	**$1,300**	**$1,000**	**$1,000**
Annual O&M Cost ($mil/yr)	$0.2	$0.3	$0.3	$0.1	$0.5	$0.1
Annual O&M Cost ($/AF)	$490	$480	$560	$260	$270	$660
Total Annual Cost ($/AF)	**$2,890**	**$2,880**	**$3,160**	**$1,560**	**$1,270**	**$1,660**



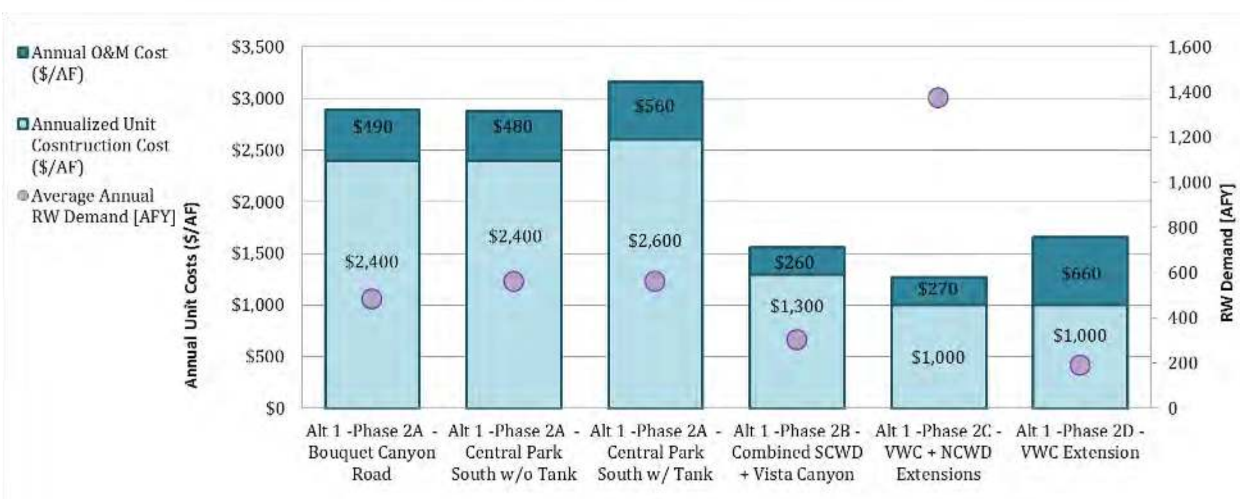
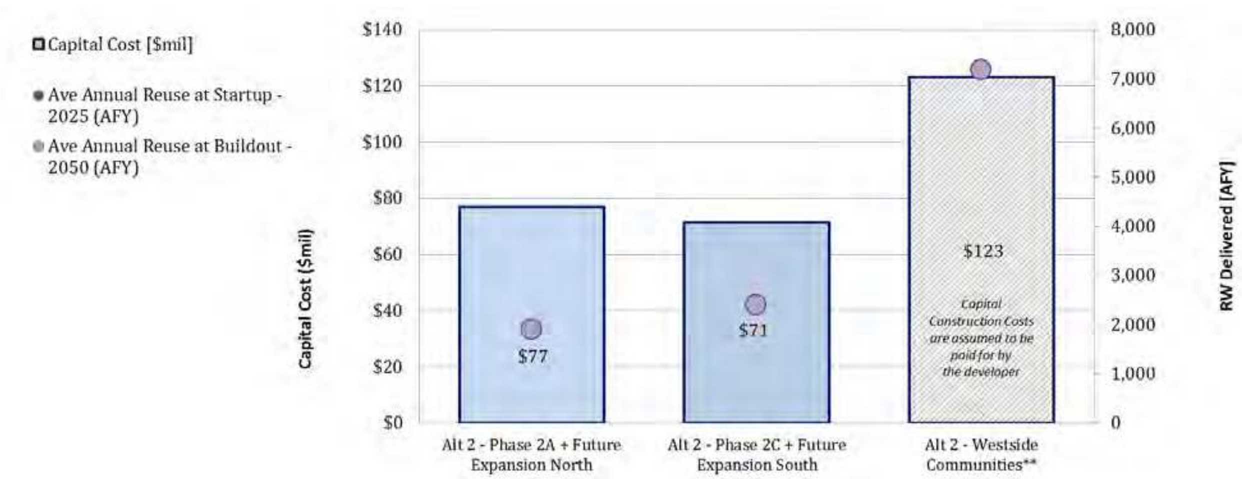


Figure 7-18: Summary of Costs for Alternative 2 - Non-Potable Reuse Expansion (Future Phases)

Facility Component	Alternative 2 - Non-Potable Reuse Expansion (Future Phases)		
	Phase 2A + Future Expansion North	Phase 2C + Future Expansion South	Westside Communities**
Treatment	$0	$0	$0
Pipelines	$52,570,000	$39,240,000	$80,690,000
Spreading Basin or Storage Tank	$1,730,000	$11,950,000	$20,220,000
Pump Station	$12,840,000	$12,460,000	$19,580,000
Site Retrofit Costs	$9,650,000	$7,730,000	$2,490,000
Total Construction Cost ($)	**$76,790,000**	**$71,380,000**	**$122,980,000**
Estimated Construction Cost ($mil)	**$77**	**$71**	**$123**
Annualized Cosntruction Cost ($mil/yr)	$4.4	$4.1	$7.1
Ave Annual Reuse at Startup - 2025 (AFY)	1,904	2,391	7,184
Ave Annual Reuse at Buildout - 2050 (AFY)	1,904	2,391	7,184
Annualized* Buildout Unit Construction Cost ($/AF)	**$2,300**	**$1,700**	**$1,000**
*based on average flow over 25 years			
Annual O&M Cost ($mil/yr)	$1.1	$1.2	$2.2
Annual O&M Cost ($/AF)	$600	$490	$300
Total Annual Cost at Buildout - 2050 ($/AF)	**$2,900**	**$2,190**	**$300**

** Total Construction Costs for the Westside Communities are assumed to be paid for developer and are not included in Total Annual Cost at Buildout



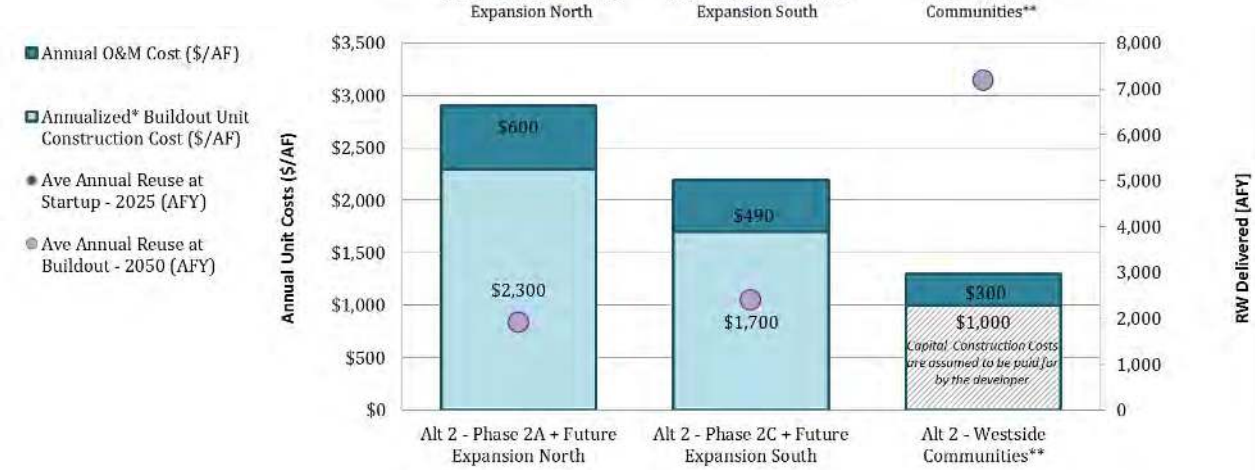


Figure 7-19: Summary of Costs for Alternative 3 - Groundwater Recharge (Surface Spreading)

Facility Component	Alternative 3 - Groundwater Recharge (Surface Spreading)				
	Phase 2A + Spreading Site #1	Phase 2A + Spreading Site #3a	Phase 2A + Spreading Site #3b	Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)
Treatment	$0	$0	$0	$0	$0
Pipelines	$44,340,000	$67,200,000	$70,790,000	$30,400,000	$46,140,000
Spreading Basin or Storage Tank	$17,610,000	$7,720,000	$15,140,000	$15,140,000	$32,870,000
Pump Station	$10,000,000	$16,510,000	$18,230,000	$12,390,000	$15,580,000
Site Retrofit Costs	$2,400,000	$2,400,000	$2,400,000	$2,400,000	$2,400,000
Groundwater/Monitoring Well	$1,170,000	$1,170,000	$1,170,000	$1,170,000	$1,170,000
Total Construction Cost ($)	**$75,520,000**	**$95,000,000**	**$107,730,000**	**$61,500,000**	**$98,160,000**
Estimated Construction Cost ($mil)	**$76**	**$95**	**$108**	**$62**	**$98**
Annualized Cosntruction Cost ($mil/yr)	$4.4	$5.5	$6.2	$3.6	$5.7
Ave Annual Reuse at Startup - 2025 (AFY)	2,560	1,760	1,760	1,660	2,560
Ave Annual Reuse at Buildout - 2050 (AFY)	4,260	4,260	4,260	1,660	4,260
Annualized* Buildout Unit Construction Cost ($/AF)	**$1,300**	**$1,800**	**$2,100**	**$2,100**	**$1,700**
*based on average flow over 25 years					
Annual O&M Cost ($mil/yr)	$2.3	$3.1	$3.1	$1.4	$3.1
Annual O&M Cost ($/AF)	$500	$700	$700	$900	$700
Total Annual Cost at Buildout - 2050 ($/AF)	**$1,800**	**$2,500**	**$2,800**	**$3,000**	**$2,400**

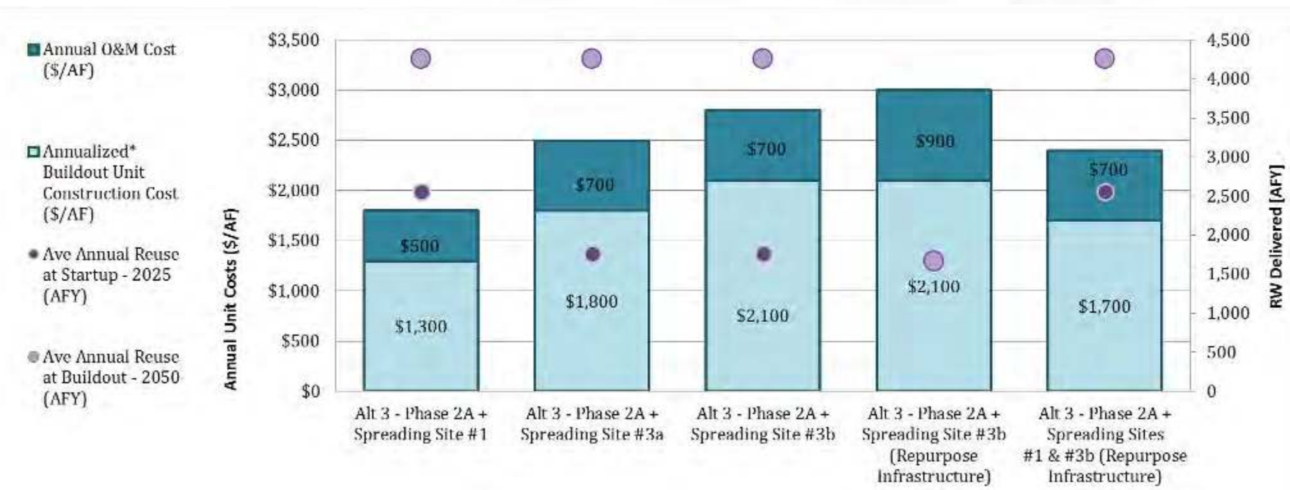
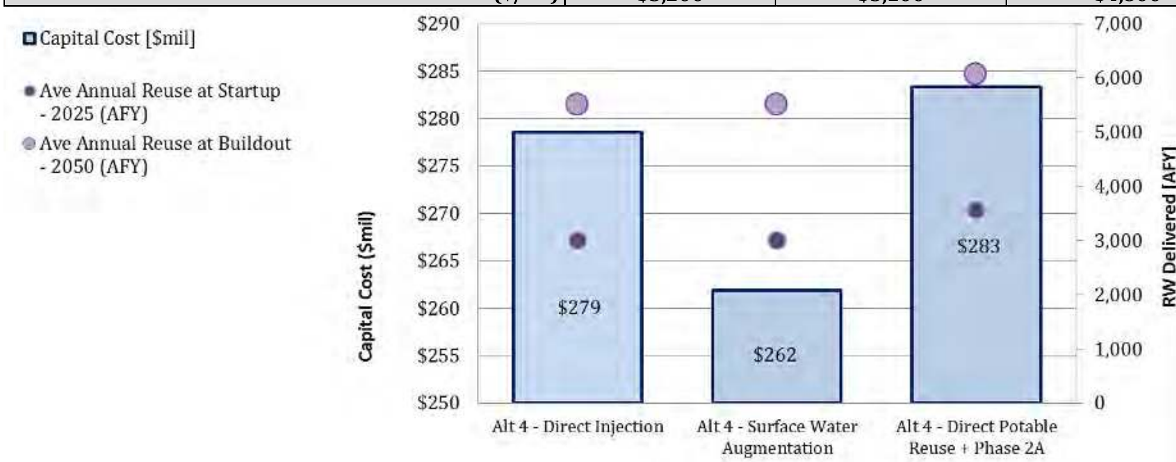


Figure 7-20: Summary of Costs for Alternative 4 - Advanced Treatment for Potable Reuse

Facility Component	Alternative 4 - Advanced Treatment for Potable Reuse		
	Direct Injection	Surface Water Augmentation	Direct Potable Reuse + Phase 2A
Treatment	$260,220,000	$220,500,000	$243,300,000
Pipelines	$4,350,000	$27,100,000	$26,800,000
Spreading Basin or Storage Tank	$0	$0	$5,100,000
Pump Station	$2,410,000	$10,800,000	$8,200,000
Groundwater/Monitoring Well	$11,580,000	$0	$0
Discharge Facility	$0	$3,500,000	$0
Total Construction Cost ($)	**$278,560,000**	**$261,900,000**	**$283,400,000**
Estimated Construction Cost ($mil)	**$279**	**$262**	**$283**
Annualized Cosntruction Cost ($mil/yr)	$16.1	$15.1	$16.4
Ave Annual Reuse at Startup - 2025 (AFY)	3,000	3,000	3,560
Ave Annual Reuse at Buildout - 2050 (AFY)	5,500	5,500	6,060
Annualized* Buildout Unit Construction Cost ($/AF)	**$3,800**	**$3,600**	**$3,400**
*based on average flow over 25 years			
Annual O&M Cost ($mil/yr)	$7.6	$8.5	$7.9
Annual O&M Cost ($/AF)	$1,400	$1,500	$1,400
Total Annual Cost at Buildout - 2050 ($/AF)	**$5,200**	**$5,100**	**$4,800**



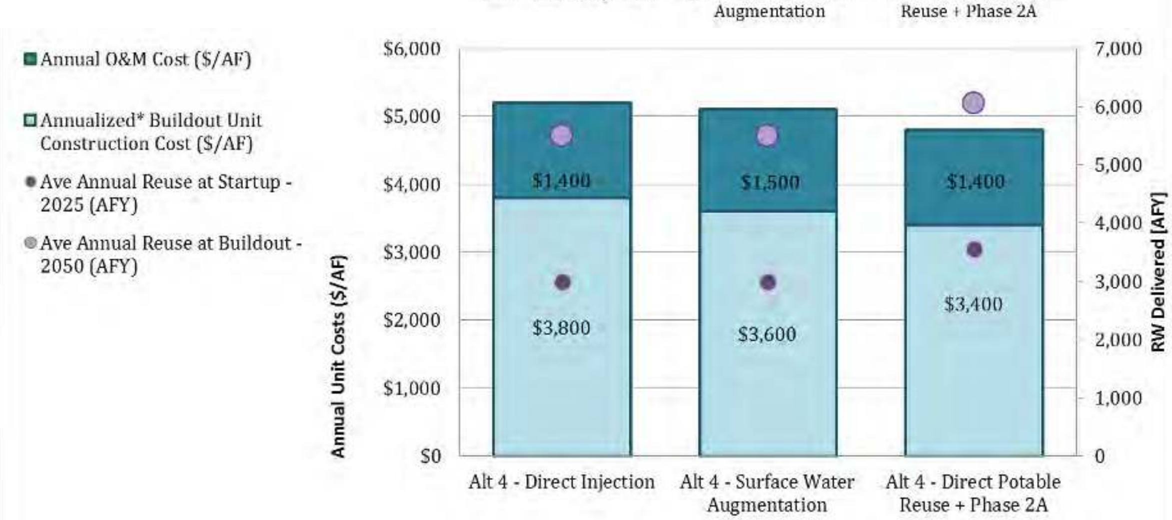


Table 7-11: Summary of Demands and Costs for Alternatives 1 through 4

Alternative	Project	Description	Ave Annual Demand (AFY)	Estimated Construction Cost ($mil)	Annualized Construction Cost ($mil/yr)	Annualized Unit Construction Cost ($/AF)	Annual O&M Cost ($/AF)	Total Annual Cost ($/AF)
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)	Phase 2A[1]	Bouquet Canyon Road	482	$20	$1.2	$2,400	$490	$2,890
		Central Park South w/o Tank	560	$24	$1.4	$2,400	$480	$2,880
		Central Park South w/ Tank	560	$25	$1.4	$2,600	$560	$3,160
	Phase 2B	Combined SCWD + Vista Canyon	300	$7	$0.4	$1,300	$260	$1,560
	Phase 2C	VWC + NCWD Extensions	1,374	$24	$1.4	$1,000	$270	$1,270
	Phase 2D	VWC Extension	186	$3	$0.2	$1,000	$660	$1,660
Alternative 2 - Non-Potable Reuse Expansion (Future Phases) [3]	Phase 2A + Future Expansion North	Includes Phase 2A and Future Expansion (Alignments E-H) North of the Santa Clara River	1,904	$77	$4.4	$2,300	$600	$2,900
	Phase 2C + Future Expansion South	Includes Phase 2C and Future Expansion (Alignments A-D) South of the Santa Clara River	2,391	$71	$4.1	$1,700	$490	$2,190
	Westside Communities[2]	Non-potable demands for proposed developments, independent of Phase 1 & 2	7,184	$123	not included	not included	$300	$300
Alternative 3 - Groundwater Recharge (Surface Spreading)[4]	Phase 2A + Spreading Site #1	Includes Phase 2A costs and maximizes deliveries to Off-Stream Spreading Site #1	3,410	$76	$4.4	$1,300	$500	$1,800
	Phase 2A + Spreading Site #3a	Includes Phase 2A costs and maximizes deliveries to In-Stream Spreading Site #3a	3,010	$95	$5.5	$1,800	$700	$2,500
	Phase 2A + Spreading Site #3b	Includes Phase 2A costs and maximizes deliveries to Off-Stream Spreading Site #3b	3,010	$108	$6.2	$2,100	$700	$2,800
	Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	Includes Phase 2A costs and reuses Honby lateral and Honby pipeline to deliver to In-Stream Spreading Site #3b	1,660	$62	$3.6	$2,100	$900	$3,000
	Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)	Includes Phase 2A costs, splits deliveries between Spreading Sites #1 & #3b, and reuses Honby lateral and Honby pipeline	3,410	$98	$5.7	$1,700	$700	$2,400
Alternative 4 - Advanced Treatment for Potable Reuse[5]	Direct Injection	Direct injection of advance-treated water near Valencia WRP	4,250	$279	$16	$3,800	$1,400	$5,200
	Surface Water Augmentation	Augment Castaic Lake with advance-treated water	4,250	$262	$15	$3,600	$1,500	$5,100
	Direct Potable Reuse + Phase 2A	Augment raw water to Rio Vista WTP with of advance-treated water (Includes Phase 2A)	4,810	$283	$16	$3,900	$1,400	$5,300

Notes:

[1] Three variations are shown for Phase 2A; only one {Phase 2A project would be selected

[2] Capital Construction Costs for the Westside Communities (estimated at $138 million) are assumed to be paid for by the developer and are therefore not included in the annualized total cost.

[3] Due to limited supply of recycled water in the summer months, not all of the Alternative 2 projects could be implemented.

[4] Since spreading would occur primarily in winter and shoulder months, an Alternative 2 project and an Alternative 3 project could both be implemented; however only one Alternative 3 project would be selected.

[5] An Alternative 4 project would utilize all water not used for irrigation and could be implemented instead of an Alterantive 3 project; only one Alternative 4 project would be selected.

Section 8: Recommended Project

This section begins with considerations that guided selection of the recommended project and then describes the decision flow process that identified the near-term recommended project and highlights the next steps to evaluate mid-term and long-term projects. A potential phasing plan, operations plan and other considerations for implementation are discussed.

8.1 Selection Considerations

The following considerations guided the selection of the recommended alternatives:

1. Cost Comparison
2. Water Supply Availability
3. Readiness to Proceed
4. Permittability
5. Required Agency Coordination/Collaboration
6. Ease of Implementation
7. Environmental Considerations

Cost Comparison

Table 8-1 ranks all projects from the lowest to highest total annual costs, which is calculated as the annualized construction cost plus the annual O&M cost divided by the average annual demand over the life of the project. See Table 7-11 or Appendix E for additional details about the costs for each project.

Table 8-1: Summary of Annualized Costs from Least to Most Expensive

Alternative - Project	Ave Annual Demand (AFY)	Estimated Construction Cost ($mil)	Annual O&M Cost ($/AF)	Total Annual Cost ($/AF)
Alt 2 - Westside Communities2	7,184	not included	$300	$300
Alt 1 - Phase 2C	1,374	$24	$270	$1,270
Alt 1 - Phase 2B	300	$7	$260	$1,560
Alt 1 - Phase 2D	186	$3	$660	$1,660
Alt 3 - Phase 2A + Spreading Site #1	3,410	$76	$500	$1,800
Alt 2 - Phase 2C + Future Expansion South	2,391	$71	$490	$2,190
Alt 3 - Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)	3,410	$98	$700	$2,400
Alt 3 - Phase 2A + Spreading Site #3a	3,010	$95	$700	$2,500
Alt 3 - Phase 2A + Spreading Site #3b	3,010	$108	$700	$2,800
Alt 2 - Phase 2A + Future Expansion North	1,904	$77	$600	$2,900
Alt 3 - Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	1,660	$62	$900	$3,000
Alt 1 - Phase 2A	560	$25	$560	$3,160
Alt 4 - Surface Water Augmentation	4,250	$262	$1,500	$5,100
Alt 4 - Direct Injection	4,250	$279	$1,400	$5,200
Alt 4 - Direct Potable Reuse + Phase 2A	4,810	$283	$1,400	$5,300

A comparison of costs is summarized below on an annual unit cost basis ($/AFY):

- The least expensive project is **Alternative 2 – Westside Communities** (~$300/AFY), based on the assumption that capital construction costs (estimated at $123 million) will be paid for by the developer.
- The next least costly projects are **Alternative 1 – Phases 2B, 2C and 2D** (~$1,300/AFY - $1,700/AFY). These projects require no additional treatment and relatively less conveyance infrastructure per volume of recycled water delivered.
- **Alternative 3 – Phase 2A Spreading Site #1** is the next least costly project ($1,800/AFY) due to the high flow rates that can be delivered to the spreading basin located closest to the Valencia WRP. The cost for **Alternative 3 – Phase 2A Spreading Site #3a/b** projects are higher ($2,500-$2,800/AFY) due to the added conveyance costs and the need to purchase more expensive Valencia Blend recycled water (a mixture of tertiary recycled water with the advanced treated water) to meet water quality requirements for recharge. The unit costs for all Alternative 3 projects include the conveyance costs for Phase 2A and the Phase 2A demand.
- **Alternative 2 - Phase 2C + Future Expansion South** ($2,200/AFY) is less expensive than **Alternative 2 - Phase 2A + Future Expansion North** ($2,900/AFY) due to the higher volume of recycled water delivered. The unit costs for all Alternative 2 projects include the conveyance costs for Phase 2C or Phase 2A (as noted) and the associated Phase 2 demands.
- The **Alternative 4** project costs are significantly higher ($5,100/AFY - $5,300/AFY) due to the need for advanced treatment, which is assumed to include enhanced brine concentrating facilities and truck hauling for brine disposal. **Alternative 4 – Direct Potable Reuse + Phase 2A** includes higher treatment costs and inclusion of the conveyance costs for Phase 2A. **Alternative 4 – Direct Injection** requires less conveyance but includes seven new injection wells. **Alternative 4 – Surface Water Augmentation** requires more conveyance pipelines and pumping to discharge into Castaic Lake.

Water Supply Availability

As discussed in Section 6, the Santa Clarita Valley is supply limited, both annually and during the peak irrigation months now and in the future. Figure 6-2 illustrates that only half of the total summertime demand for all existing and potential future irrigation needs in the Valley can be met.

As shown in Table 8-2, there is sufficient supply to meet the **Alternative 1 projects** demand from the Valencia WRP. However, once Phase 2A 2C and 2D are online, there is not enough supply of recycled water in the summer months to meet all of the projected **Alternative 2 projects** demand from the Westside Communities plus future expansion north (off Phase 2A) and south (off Phase 2C). The total supply of recycled water to meet irrigation needs for existing customers (450 AF), Alt 1 projects (2,420 AF), and Alt 2 extensions (9,525 AFY) is 12,395 AFY; which less than the annual supply of recycled water in Santa Clarita Valley (17,140 AFY per Table 6-3). However, not all of these projects could be implemented due to limited supply of recycled water in the summer months when irrigation demands are greatest.

Although recycled water supply would be fully utilized in the summer with implementation of one or more Alternative 2 projects; there would still be surplus supply available in the winter, when irrigation demand is low, for implementation of an Alternative 3 or Alternative 4 project. It is anticipated that 3,000 AFY of winter supply would be available by 2025 and 5,500 AFY would be available by 2050. Table 8-3 summarizes the available winter supplies for Alternatives 3 and 4.

For **Alternative 3 projects**, the initial recharge volume is based on available flows in 2025 and accounts for the allowable initial recycled water concentration (RWC) to meet the GRR Regulation requirements. Other factors taken into account include available diluent water from underflow, available area for spreading and required prioritization of stormwater recharge over recycled water recharge (see Appendix C for more details). The ultimate recharge volume is based on available flows in 2050 and similarly accounts for available diluent water, available spreading area and prioritizes stormwater recharge. The recharge volume for most Alternative 3 projects would steadily increase from 2,000 AFY to 3,700 AFY based on the available flow from the Valencia WRP. All of the Valencia WRP supply cannot be utilized for recharge due to reduced recycled water deliveries before and after rain events to allow for stormwater recharge.

The **Alternative 4 projects** are not limited by diluent water requirements nor competition with stormwater flows; therefore, these projects would be able to utilize all available flows from the Valencia WRP not destined for irrigation or discharge.

Table 8-2: Summary of Alternative 1 and 2 – NPR Demands that can be met by the Valencia WRP Supply

Supply and Demand	Annual Volume (AFY)	Peak Summer Month (AFY)[1]	Notes
Valencia WRP - Available RW Supply			
Near-Term Supply (2020)	5,800	495	Projected recycled water at the Valencia WRP less 8.5 mgd discharge to the Santa Clara River.
Long-Term Supply (2050)	11,400	972	
RW Demand - Existing Phase 1 + Alt 1 Projects			
Existing Phase 1	450	65	Recycled water delivered in 2015.
Phase 2A	560	81	
Phase 2B	300	43	To be met by Vista Canyon Water Factory supply.
Phase 2C	1,374	198	
Phase 2D	186	27	
Total Existing and Alt 1 Demands	**2,870**	**413**	
Alt 1 Demand met by Valencia WRP Supply	**2,570**	**370**	Does not include Phase 2b demand.
Remaining Valencia WRP Supply in 2020	**3,230**	**124**	Sufficient Valencia WRP supply in 2020 and 2050. Able to meet all Alt 1 RW Demands.
Remaining Valencia WRP Supply in 2050	**8,830**	**601**	
RW Demand - Alt 2 Projects			
Future Expansion North	1,344	194	Does not include Phase 2A demands.
Future Expansion South	1,017	147	Does not include Phase 2C demands.
Westside Communities	**7,164**	**1,035**	Total planned development demand.
To be met by Newhall Ranch WRP supply	3,549	354	Projected WRP effluent at development is insufficient to meet demands from March to October.
To be met by Valencia WRP Supply	3,615	681	Supplemental Supply from Valencia WRP (Mar-Oct).
Total Alt 2 Demand	**9,525**	**1,375**	
Alt 2 Demand met by Valencia WRP Supply	**5,976**	**1,021**	Does not include Westside Communities demand met by the Newhall Ranch WRP.
Remaining Valencia WRP Supply in 2020	**-2,746**	**-897**	Insufficient Valencia WRP supply in 2020.
Remaining Valencia WRP Supply in 2050	**2,854**	**-420**	Summer shortfall in 2050. Unable to meet all Alt 2 RW Demands.

[1] Peak summer irrigation demand based on historical monthly demand distribution for Phase 1 system

Table 8-3: Summary of Alternative 3 and 4 – RW Recharge and Advanced Treated Reuse that can be met by the Valencia WRP Supply

Supply and Demand	Annual Volume (AFY)		Notes
Valencia WRP - Available RW Supply for Recharge			
Mid-Term Supply (2025)	3,000		After NPR demands are maximized (i.e. no summer flow is available).
Long-Term Supply (2050)	5,500		
RW Recharge - Alt 3 Projects	Initial Recharge (AFY)	Ultimate Recharge (AFY)	
Phase 2A + Spreading Site #1	2,000	3,700	
Phase 2A + Spreading Site #3a	1,200	3,700	
Phase 2A + Spreading Site #3b	1,200	3,700	
Phase 2A + Spreading Site #3b (Repurpose Infrastructure)	1,100	1,100	
Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)	2,000	3,700	
Maximum Alt 3 Recharge Volume	**2,000**	**3,700**	
Remaining Valencia WRP Supply in 2025 *(Mid-Term Supply - Max Alt 3 Initial Recharge Volume)*	**1,000**	n/a	The ultimate recharge volume for most Alt 3 projects could not be reached until more flow is available from the Valencia WRP. The remaining supply not utilized is due to reduced RW deliveries before and after rain events to allow for stormwater recharge.
Remaining Valencia WRP Supply in 2050 *(Long-Term Supply - Max Alt 3 Ultimate Recharge Volume)*	n/a	**1,800**	
RW for Advanced Treated Reuse - Alt 4 Projects	Initial Potable Reuse (AFY)	Ultimate Potable Reuse (AFY)	
Direct Injection	3,000	5,500	
Surface Water Augmentation	3,000	5,500	
Direct Potable Reuse + Phase 2A	3,000	5,500	Does not include Phase 2A demands.
Maximum Alt 4 Advanced Treated Reuse	**3,000**	**5,500**	
Remaining Valencia WRP Supply in 2025 *(Mid-Term Supply - Max Alt 4 Reuse)*	**0**	n/a	The Alt 4 projects would use all available flows from the Valencia WRP not destined for irrigation or discharge.
Remaining Valencia WRP Supply in 2050 *(Long-Term Supply - Max Alt 4 Reuse)*	n/a	**0**	

Readiness to Proceed

The **Alternative 1 Projects** are in varying stages of preliminary design/environmental evaluation and are anticipated to be constructed between 2017 and 2024. These projects are also in line (or will soon be in line) for funding from the SWRCB Proposition 1 grant and loan program. Phase 2B is planned for construction first, to align with the Vista Canyon Development Project. Phase 2C and 2D would follow closely behind. Phase 2A could serve as the transmission infrastructure required for an IPR or DPR project or for future pipeline expansions north; therefore, this project would be the last of the Alternative 1 projects to be constructed to allow time to confirm the most appropriate sizing for conveyance facilities.

The readiness to proceed of the **Alternative 2 Projects** is contingent on the timeline of the Westside Communities Development and the availability of flows from the Valencia WRP. If the Westside Communities Development proceeds on-schedule, the conveyance facilities and Newhall Ranch WRP could be operational by 2024 but would not reach its full production capacity until 2035. Valencia WRP would initially meet all recycled water demands for the Westside Communities. Once the Newhall WRP reaches its ultimate capacity, the Valencia WRP would provide about fifty percent of the demands. If additional Valencia WRP recycled water supply becomes available, either from reduced demands or reduced discharge requirements, then the Future Expansion Projects North and/or South could be implemented.

The **Alternative 3 Groundwater Recharge Projects** require additional study to confirm feasibility prior to initiating design or environmental work. It is recommended that a potable reuse groundwater recharge feasibility study be performed to further study hydrogeologic conditions, underflow assumptions, extraction scenarios and other regulatory and permitting requirements.

Similarly, **Alternative 4 Advanced Treated Reuse Projects** would also require additional study to confirm feasibility and permittability. These projects would have the longest timeline and would not be implemented if an Alternative 3 GRR project was pursued.

Permittability

Non-potable reuse projects are currently permittable under CCR - Title 22 and Title 17, thus **Alternative 1 and 2** projects could be permitted under existing regulations. GRR regulations were finalized in June 2014; however, the permittability of the **Alternative 3** projects would require discussions with regulatory agencies to evaluate site specific conditions such as the underflow diluent water assumptions and the permitting requirements for an in-stream or off stream basin.

The permittability of **Alternative 4** projects is less certain. Surface Water Augmentation criteria are anticipated to be released by December 31, 2016, which should provide more clarity as to whether augmenting Castaic Lake with recycled water could qualify as a Surface Water Augmentation Project. It is likely that DPR would take longer to be permittable in California; thus the progress of DPR regulations should continue to be tracked.

Required Agency Coordination/Collaboration

All recycled water projects require coordination with the agencies responsible for the regulation of recycled water: the SWRCB-DDW and individual RWQCBs. Most recycled water projects in the Santa Clarita Valley will require some level of coordination between CLWA and the purveyors. Projects that serve new developments, such as Vista Canyon (**Alternative 1 - Phase 2B**) and the Westside Communities (**Alternative 2**), also require coordination with the project developer. The **Alternative 3 - Groundwater Recharge** projects would require partnership with the LACFCD to operate the basins for stormwater recharge and would require additional coordination with Fish and Wildlife Service and other agencies involved with management of the Santa Clara River. Any groundwater recharge project (spreading or injection) would require close coordination with agencies that operate extraction wells in the vicinity of the projects to optimize recovery of recharged water. An **Alternative 4 – Surface Water Augmentation** project would require coordination with the Metropolitan Water District given the shared use of Castaic Lake for water supply.

Ease of Implementation

The **Alternative 1 and 2** non-potable reuse projects would be the easiest to implement in terms of regulatory requirements, public acceptance and infrastructure requirements. **Alternative 3** projects require additional study; entail more complex permitting requirements, additional agency coordination and local partnerships, and a concerted public outreach effort. **Alternative 4** projects would be the most complex to implement given regulatory uncertainties, the need for costly additional treatment facilities and public acceptance.

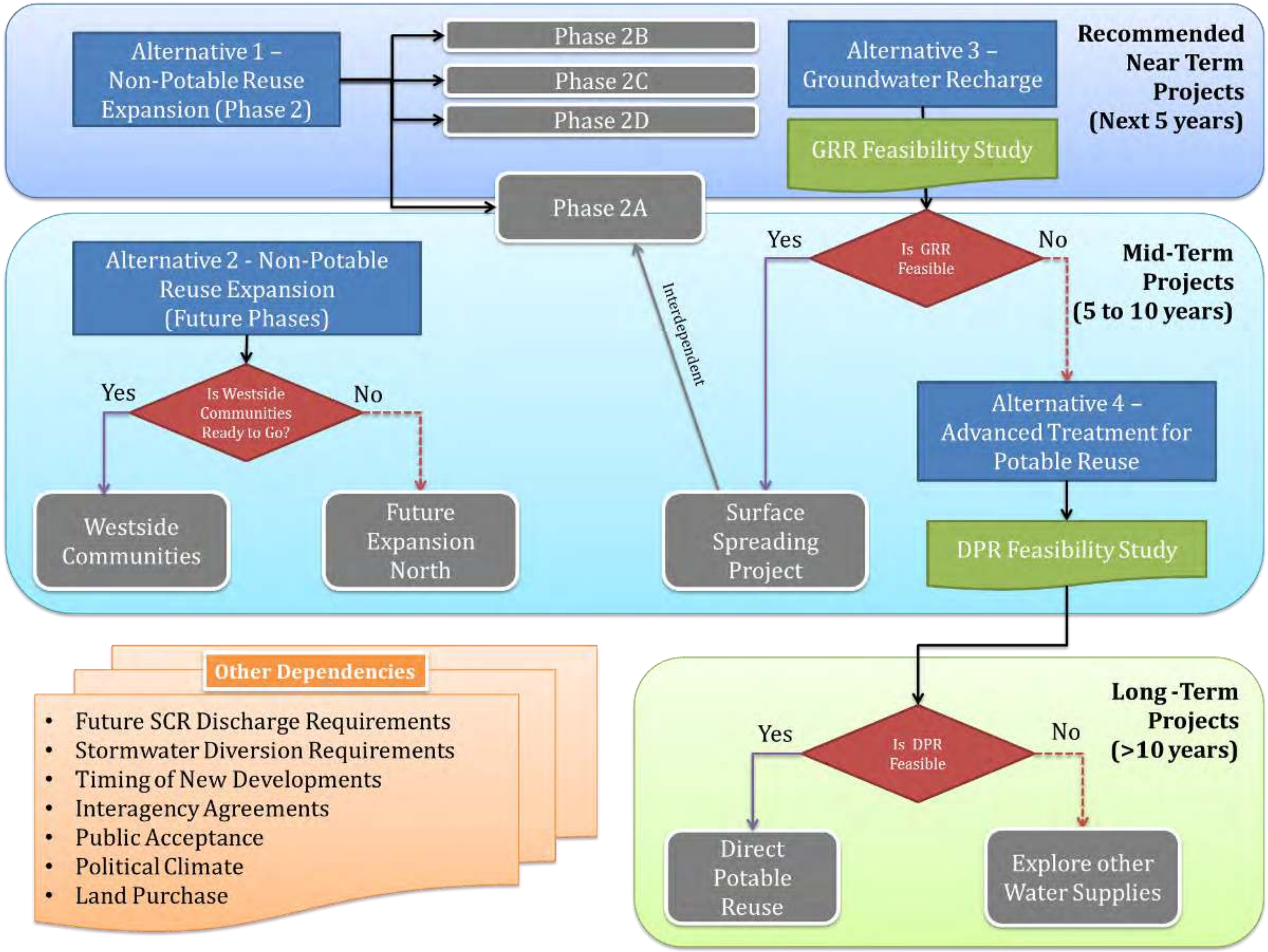
Environmental Considerations

Environmental issues for each of the alternative projects will be evaluated in a Programmatic Environmental Impact Report (PEIR) being developed for CLWA under a separate contract. The PEIR will identify potentially significant environmental impacts in accordance with California Environmental Quality Act (CEQA). The PEIR will include: the required contents and a detailed inventory of existing conditions; CEQA provided thresholds of significance used for evaluation of impacts; an analysis of the environmental impacts and levels of significance; and appropriate mitigation measures for each environmental disciples. The PEIR will include both the anticipated direct effects and anticipated secondary effects of implementing the Recycled Water Master Plan Update at a programmatic level.

8.2 Decision Flow Process

Based on above considerations, the decision flow process presented in Figure 8-1 illustrates the decision points to guide the future expansion of the CLWA recycled water program, interdependence of projects and other dependences.

Figure 8-1: Recycled Water Program – Decision Flow Chart



In summary:

- **Alternative 1 Phase 2B, 2C and 2D projects** are recommended to proceed first because these projects (1) are the lowest cost projects that serve existing irrigation customers, (2) have sufficient recycled water supply available, (3) have initiated design and environmental work and are in-line for funding, (4) are currently permittable and would be similar in operation to the existing Phase 1 system, (5) are the easiest to implement in terms of regulatory requirements, public acceptance and infrastructure requirements. The **Phase 2A** project has similar circumstances; however, since it is dependent on the outcome of the GRR Feasibility Study and future expansion decisions, this project would proceed once the status of those projects is clearer.

- The **Alternative 3 GRR projects** offer a unique opportunity to create a multi-beneficial project and utilize excess recycled water available in the winter and shoulder months. These projects provided the added benefit of comingling recycled water and stormwater to recharge areas of the groundwater aquifer with a local and drought proof supply. Due to the unique nature of these projects, additional evaluation is needed to confirm the feasibility, permittability and public acceptability of groundwater replenishment in the Santa Clarita Valley. Thus it is recommended that a **GRR Feasibility Study be** conducted to confirm the viability of this alternative.

- **Alternative 2 Westside Communities** is the most cost effective project and would be the recommended expansion beyond Phase 2; however, the benefit of the developer funding the capital infrastructure is balanced by the challenge of having less control over the schedule for development. Thus, uncertainty of the readiness of this development to proceed may defer this project and result a decision to pursue one of the other Alternative 2 Projects.

- The **Alternative 4 Advanced Treated Reuse Projects** would be the most expensive due to the need for advanced treatment and brine disposal. These projects are subject to more regulatory uncertainty and the public acceptance of potable reuse is also uncertain at this time. A comparison of direct injection to surface spreading could be performed as part of the GRR Feasibility Study. If groundwater replenishment is not found to be feasible, then the viability of Direct Potable Reuse and Surface Water Augmentation should be tracked in the long-term.

8.3 Description of Recommended Project

As discussed in Section 8.2, there are many projects, which provide viable opportunities to expand the use of recycled water in Santa Clarita Valley in the near-, mid- and long-term. The decision to pursue one project over another may in some cases depend on external factors, such as the progress of private developments, future discharge requirements, the availability of land, political climate, agreements with other agencies and the permittability and public acceptance of potable reuse.

For the purpose of this RWMP the Recommended Project is defined as a course of action in the near-term to expand recycled water in Santa Clarita Valley. The Recommended Project includes the following activities:

(1) **Implement Alternative 1 - Non-Potable Reuse Expansion Projects - Phase 2B, 2C and 2D.** These projects, previously shown in Figure 7-1, are currently in various stages of design and environmental work and are progressing through the efforts of CLWA and/or the lead purveyor assigned to each project. These projects are already in-line for Proposition 1 funding and may be competitive for other funding opportunities. Together, these three projects will increase the recycled water delivery in Santa Clarita Valley from 450 AFY to 2,310 AFY.

(2) **Complete preliminary design and environmental work for Alternative 1 Non-Potable Reuse Expansion Project - Phase 2A.** Given the interdependency of the Phase 2A transmission pipeline with other potential future expansion opportunities, it is recommended that the backbone pipeline be sized with a 24-inch diameter pipeline to meet potential future demands for Alternative 2 – Future Expansion North, Alternative 3 - GRR or Alternative 4 – DPR. Final design for Phase 2A should be deferred until the feasibility of GRR is determined.

(3) **Initiate a GRR Feasibility Study to evaluate the viability of Alternative 3 GRR projects.** The feasibility study would include additional hydrogeologic, hydrologic and operations modeling to confirm assumptions, coordination with LACFCD regarding implementing a cooperative recycled water-stormwater recharge project, discussions with DDW and the RWQCB regarding permitting, communication with land owners to confirm the availability of the spreading sites and outreach to the public about indirect potable reuse. The study would include evaluation of the alternatives for surface spreading of tertiary recycled water, previously shown in Figures 7-10 to 7-12, and could explore opportunities for direct injection of advanced treated recycled water (Figure 6-4).

Beyond the Recommended Project, activities conducted in the mid-term should be focused on optimizing expansion of the recycled water system beyond Phase 2, and would include:

- ✓ Tracking recycled water deliveries from the Phase 1 and 2 projects to understand peak irrigation demands and to improve operational efficiency of the recycled water system.
- ✓ Following SCVSD's efforts related to the Chloride Compliance Project and instream flow requirements. Potential changes to future discharge requirements or other opportunities may make more (or less) recycled water available in the summer months. The availability of advance treated recycled water will also influence the viability of a GRR project.
- ✓ Closely monitoring the status of the Westside Communities development. A key decision point may arise if this development is only partially built or put on hold indefinitely, at which point CLWA and the purveyors would have the opportunity to pursue other projects within Alternatives 2, 3, or 4. The selection of the next best project(s) would likely be influenced by a combination of the outcome of the GRR Feasibility Study, climatic conditions, water supply availability, imported water rates, and political influences.

The Alternative 4 projects represent long-term opportunities to maximize reuse in the Santa Clarita Valley. These projects would only be pursued if GRR is not selected for implementation and would require an AWTF and brine disposal at a high capital and operating cost. A DPR project represents the most cost effective Alternative 4 project but is contingent on regulatory and legislative progress and public acceptance. CLWA and the purveyors should continue to track DPR developments to understand the possibilities, benefits and limitations for implementing a project in Santa Clarita Valley in the future.

8.4 Implementation Plan for the Recommended Project

Memorandums of Understanding (MOUs) between CLWA and each purveyor were developed to establish a framework to guide the implementation of the recycled water program[5]. The MOUs also provide additional specifications on purveyor roles and responsibilities. Specifically, the MOUs:

- Include definitions of terms,
- Define responsibilities,
- Discuss environmental review,
- Describe procedures for future project approval and implementation (budgeting, design, backbone costs, improvements, implementation considerations, grants, insurance, payment, etc.), and
- Include indemnification, terms, termination, severability, entire agreement, third party beneficiaries, governing law, etc.).

[5] The executed MOU for each purveyor is included in Tab 14 of the SCV Rules and Regs Handbook (Kennedy/Jenks 2016b).

CLWA, the project proponent (i.e. identified purveyor), the public entity governing the development of the project (if applicable) and the private developer (if applicable) would be responsible for executing the appropriate agreements to clearly define the roles and responsibilities for each entity for implementing recommended project. Agreements would likely be developed to define cost sharing, funding mechanisms, ownership and operations and maintenance responsibilities over the life of the project.

The implementation plan for the recommended project is discussed in the following sections in terms of development activities that need to occur and issues that need to be addressed for Phase 2 and the next steps to initiate a GRR Feasibility Study.

Due to uncertainties related to the feasibility of a GRR Project and the availability of recycled water for beyond Phase 2, an implementation plan for future expansion is not included herein, though a high-level phasing plan for the overall CLWA Recycled Water Program is discussed in Section 8.5.

Implementation Plan for Alternative 1 – Phase 2 Projects

The following is a listing of the major implementation elements and issues to be addressed which are common to the Phase 2 projects. Many of the activities apply to all the Phase 2 projects; however, some issues are unique to individual projects or facilities. For example, some of the Phase 2 projects have already completed some of these steps or work is currently being performed under separate contracts. The activities are generally listed in order of occurrence; however, most would require concurrent effort through the duration of implementation.

- **Customer Development –** Verify customer commitment, connection locations, retrofit requirements, and DDW approvals.
- **Preliminary Design/Engineering Feasibility –** Evaluate alternative pipeline routes, collect detailed utility and traffic information, prepare updated cost estimates, and update with new information from customer development activities. Preliminary design can be initiated following initial verification of customer information, provided updated customer information does not identify other significant issues. Agreements to define roles, responsibilities, and cost sharing should be established at this time.
- **Regulatory Approvals –** Identify required permits and regulatory approvals, including DDW, RWQCB, CEQA, and construction permits. Develop management plan and schedule to obtain regulatory approvals, considering appropriate review periods for regulatory agencies. Regulatory activities should be initiated concurrently with preliminary design and continue through implementation and operation.
- **Design/Construction –** Incorporate any updated customer information, regulatory requirements, and community concerns. Re-evaluate economics with updated information and design level cost estimate. Design and construction efforts can begin immediately following preliminary design.

- **Training –** Training and guidance to the site supervisors assigned by each recycled water user is provided by LACSD, along with education of site supervisors on the proper use of recycled water, recycled water regulations, and basic principles of backflow prevention and cross-connection control. Refer to the SCV Rules and Regulations Handbook (Kennedy/Jenks 2016b) for additional information.

Implementation Plan for GRR Feasibility Study

The recommended project includes conducting a GRR Feasibility Study to further evaluate the viability of groundwater recharge with recycled water in Santa Clarita Valley. This would include exploration of surface spreading (Alternative 3) and could also explore direct injection (Alternative 4) to identify a GRR Project that is implementable, acceptable by the CLWA Board and staff, supported by the regulators and stakeholders, and affordable. The GRR Feasibility Study could be led by CLWA or a project proponent and ideally would engage the LACFCD to be a project partner.

The GRR Feasibly Study may include, but not be limited to, the following work:

1. Perform additional groundwater modeling to evaluate and confirm assumptions related to: (1) hydrogeologic conditions (including travel time, percolation rates and potential for mounding), (2) underflow availability of diluent water for spreading, (3) preferred placement of injection wells for direct injection to achieve a travel time of 6-months before extraction (if direct injection is further studied), (4) groundwater management approach to optimize extraction of the recharged water, using existing or new extraction wells.
2. Perform a hydrologic evaluation to identify the potential stormwater recharge for different hydrologic year types and operational practices. This will require coordination with LACFCD to understand the preferred design parameters and operational approach to utilizing the spreading basins for stormwater recharge.
3. Coordination with SCVSD regarding the use of tertiary and Valencia Blend[6] water for groundwater replenishment, including the available volume and cost documented in a contract or agreement. Further explore potential treatment options for a direct injection project (if direct injection if further studied).
4. Engage DDW and the RWQCB to provide information and gain preliminary input and build support for the project. Initial discussions should cover permitting requirements, use of underflow as diluent water and operation of the spreading basins as a combined recycled water-stormwater recharge project. It is likely that an Independent Advisory Panel (IAP) may be engaged to review technical assumptions and provide scientific support for the project.
5. Initiate communication with land owners to confirm the availability of the spreading sites.

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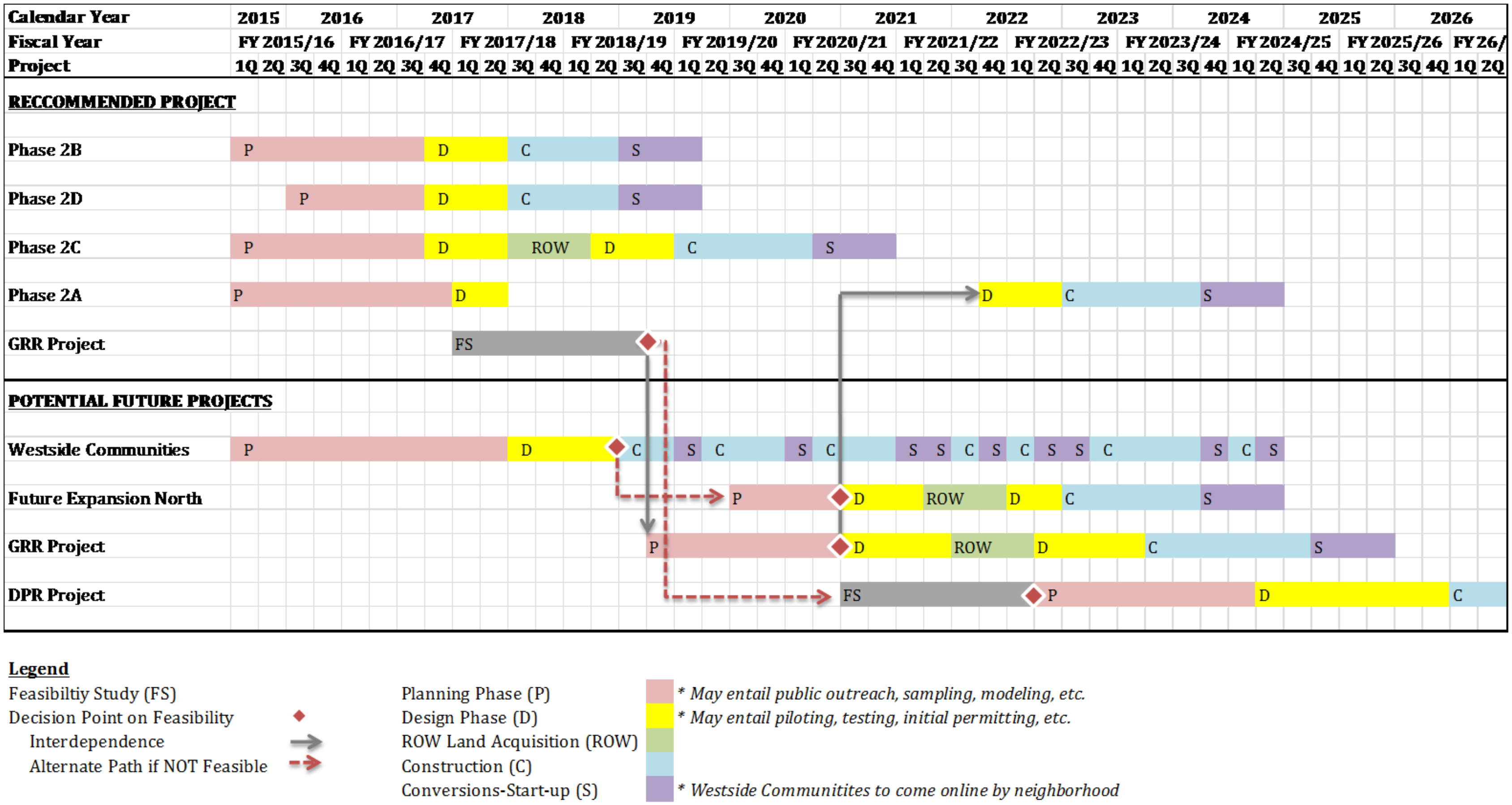
[6] Based on discussions with SCVSD, "Valencia Blend" is defined as a 70/30 blend of tertiary effluent to RO permeate from the AWTF.

6. Further explore the feasibility of re-purposing existing infrastructure (i.e. Honby lateral and Honby alignment).
7. Develop and implement an outreach strategy to communicate with the public about the potential opportunity for groundwater replenishment with recycled water in Santa Clarita Valley.
8. Perform additional environmental analysis to understand environmental issues for each GRR project site.
9. Update project costs and further explore non-quantifiable benefits for the different GRR projects. If GRR is determined to be feasible, identify a preferred project and the next steps for implementation.

8.5 Potential Phasing Plan for CLWA Recycled Water Program

A potential phasing plan for the CLWA recycled water program is presented in Figure 8-2 based on the considerations discussed earlier in this section and the decision flow process presented in Figure 8-1.

Figure 8-2: Recycled Water Program – Potential Phasing Plan



Note: Phase 2 project schedules extracted from Major Capital Projects calendar and updated on Sept 25, 2016

The implementation plan for **Phase 2 projects** is based on the CLWA schedule for Major Capital Improvements updated March 16, 2016. Design activities and environmental work for Phase 2B, 2C and 2D have been initiated. Preliminary CEQA documentation for Phase 2A began in 2015 to support the Prop 1 funding application package for the Phase 2 projects. Design efforts for Phase 2A will resume in 2017; however, final design will be deferred until 2021 based on the interdependence of the transmission pipeline sizing with potential future projects.

The **GRR Feasibility Study** is assumed to be initiated once the RWMP and PEIR are finalized at the end of 2016. If a preferred and feasible GRR Project is identified in the feasibility study, the next steps for implementation may require additional planning activities (i.e. pilot testing, field samplings, or other additional studies) prior to the design phase.

If a GRR Project is deemed infeasible then a **DPR Feasibility Study** could be initiated to assess the viability to advance treat excess recycled water not used for irrigation or river discharge to supplement potable water supplies. By the end of 2016, the DPR feasibility report will be released by DDW; however, a formal DPR regulatory framework will not be developed at that time. CLWA and the purveyors should track direct potable reuse developments in California and revisit the feasibility of DPR if a goal to achieve 100 percent re-use of available wastewater is desirable.

Planning efforts have been in progress for the **Westside Communities** for many years and a Recycled Water Master Plan for the proposed development was revised in November 2015 (Dexter Wilson, 2015c) to describe potential facilities to deliver recycled water for non-potable use. The implementation schedule shown in Figure 8-2 is based on estimates provided by VWC and do not reflect the uncertainty of the timeline due to recent rulings by the California Supreme Court.

If the Westside Communities development is indefinitely delayed or reduced in size, additional summer flows may be available to support additional recycled water pipelines beyond Phase 2A for **Further Expansion North**. Reduced minimum discharge requirements, increased recycled water production or decreased anticipated demands could also free-up additional supplies to support further expansion of the system over time. It is recommended that additional planning commence in 2020 once the Phase 2b, 2C and 2D systems are online and the status of the Westside Communities is more defined.

Other factors that should be considered during the implementation of each project to optimize the design include:

- Ease or willingness of customers to connect to recycled water
- Community impacts and development requirements
- Water utility involvement/cooperation
- Funding and financing availability
- Reliability and operational costs considerations
- System flexibility
- Project specific regulatory requirements

8.6 Operations Plan

A recycled water system **Operations Plan** is typically required by the RWQCB as part of the permitting process for a recycled water system. The purpose of the Operation Plan is to:

- Provide information to regulators that is not addressed in the Title 22 Engineering Report (described in Appendix B – Section B.3)
- Describe how the reuse site will be operated and maintained to comply with the SCVSD Requirements for Recycled Water Users.
- Provide staff with a comprehensive project document that serves as an outline for operation of the recycled water project facilities.

The Operations Plan complements the Title 22 Engineering Report in that it provides further detail about program administration, regulatory requirements, treatment processes, Supervisory Control and Data Acquisition System (SCADA) operation, alarms, storage and distribution system operation, pump control strategies, monitoring and reporting requirements, backup potable supply, cross-connection prevention, emergency response, site retrofit process, and site management.

The Operations Plan is based on concepts outlined in the Engineering Report. Therefore, the Operations Plan should be prepared after the Engineering Report has been approved by DDW to ensure the fundamental concepts are acceptable to DDW and the RWQCB. The requirement to develop an Operations Plan may be a condition of the RWQCB recycled water permit or a condition of DDW approval of the Engineering Report.

An independent Operations Plan may need to be developed for each project, depending on the entities involved, associated infrastructure, type of use and training requirements.

- **Entities Involved:** CLWA, the project proponent (i.e. purveyor), the public entity governing the development of the project (if applicable) and the private developer (if applicable) would be responsible for executing the appropriate agreements to clearly define the roles and responsibilities for each entity for implementing recommended project, including operations and maintenance responsibilities over the life of the project. Recognizing the significant level of customer service and support required for sustainable delivery of recycled water in the Santa Clarita Valley, the Purveyors will play a major role in the success of on-going operations and recycled water delivery.
- **Associated Infrastructure:** The infrastructure needed to deliver recycled water service to Santa Clarita Valley customers will be differentiated into: (1) a "backbone" system to be owned and funded by CLWA, and (2) the retail distribution system to be owned and funded by the Purveyors. While this delineation of ownership and funding responsibilities is easily understood in broad terms, identifying precise locations for demarcation of facilities is highly dependent on the specific local circumstances and conditions associated with each project. Functional responsibility for operations and maintenance of infrastructure should be assigned

to the party best suited and equipped to undertake the defined role, irrespective of whether that party owns the facilities or not.

- **Type of Use:** For non-potable uses that use recycled water from the Valencia WRP (i.e. Phase 2A, 2C and 2D), the Operations Plan may be similar to the existing Phase 1 Recycled Water System Plan, with the appropriate entities and responsibilities defined for each piece of new infrastructure. For Phase 2B and the Westside Communities, the Operations Plan would need to be developed based on the permit requirements for the planed Vista Canyon Water Factory and Newhall Ranch WRP. For a GRR Project, a unique Operations Plan would need to be developed as part of the design and permitting efforts.
- **Training requirements:** Implementation of a recycled water project will create additional permanent workload due to the operation and maintenance of new facilities, monitoring and reporting requirements, site supervisor training, and ongoing public outreach and education. A training program should be developed for all personnel who are involved with the recycled water project. The training will provide staff with a foundation of knowledge about the project and to ensure any communication with the public is handled appropriately.

8.7 Other Considerations for Implementation

The SCV Rules and Regulations Handbook (Kennedy/Jenks 2016b) provides a step-by-step process for obtaining permission to use recycled water from the SCVSD WRPs (see Section 2.1) and describes requirements for the following activities:

1. Obtaining a User Agreement
2. Completing a User Application
3. Preparing an Emergency Cross-Connection Response Plan
4. Completing an Operations Manual
5. Submitting Plans and Specifications
6. Developing a Title 22 Engineering Report.
7. Meeting CEQA Requirements
8. Arranging Pre-and Post-Construction Requirements
9. Arranging for Project Startup Activities
10. Designating and Defining Responsibilities for a Site Supervisor

The requirements set forth in the SCV Rules and Regs Handbook must be followed by all recycled water projects.

Section 9: Construction Financing Plan and Revenue Program

This section describes potential funding opportunities and financing mechanisms for the Recommended Project, including a summary of current applicable grant and loan opportunities. A discussion on recycled water pricing policies and potential avoided costs and lost revenues is also included.

9.1 Funding and Financing Alternatives

The availability of adequate funds to cover construction costs is a primary constraint in implementing any capital improvement project. There are a variety of funding sources available to help pay for planning studies, design documents, construction activities and research for recycled water projects. To finance the construction cost of the proposed facilities, CLWA can obtain capital through the following funding sources:

1) Capital Reserves
2) Grants and loans
3) Long-Term Debt Issuance
4) Revenue Sources for Loan/Dept. Service Repayment

These potential funding sources are discussed in detail in the following sections

Capital Reserves

CLWA receives revenues from facility capacity fees, one percent property taxes, water rates and interest on investments. CLWA policy is that the facility capacity fees and one percent taxes pay for capital expenses; while water rates cover O&M expenses. It also has the authority to levy standby charges but has, thus far, not exercised this authority. To the extent that the one percent property tax revenues exceed existing debt, and capital expenditures, capital reserves shall accumulate. A portion of these reserves is utilized as security for the repayment of debt. The remainder is available for CLWA's Capital Improvement Program (CIP) in which the recycled water program is included.

Grants and Low Interest Loans

Federal, state and local governments have policies to encourage recycled water projects. These policies have led to several grant and low interest loan programs. Available funds for grants and low interest loans are dependent upon legislative approval and available monies. The grant application process can be highly competitive, has set application deadlines and may require varying levels of effort for the application process. Projects that integrate multiple benefits and meet various objectives tend to perform better on competitive grants. Elements such as regional partnerships, integrated project benefits, incorporation of water conservation, stormwater capture, renewable energy and job creation or preservation would make for a more competitive grant application.

For each proposed project, potential funding sources should be identified and vetted during preliminary design work and as part of the financial planning phase. A summary of some of the current funding opportunities is provided in the Table 9-1 with additional details to follow.

Table 9-1: Summary of Potential Funding Sources

Granting Agency	Funding Opportunity	Funding Available	Max Grant : Min Match (%)
State Water Resources Control Board (SWRCB)	Facilities Planning Grant Program	unknown	$75,000: 50%
	Water Recycling Construction Funding Program	~$625 million	$15 million: 50%
	Clean Water State Revolving Fund Loan (CWSRF)	$500 million	$50 million: n/a
	Stormwater Grant Program (SWGP)	unknown	$50k to $500k Planning Grant: 50% $250k to $10 mil Implementation Grant: 50%
Department of Water Resources (DWR)	Prop 84 IRWM Implementation Grant Program	~$510M Statewide	Depends on funding area: 50%
U.S. Bureau of Reclamation (USBR)	Title XVI Feasibility Studies (WaterSMART)	$21 million	$4 million: 75%
	Title XVI Water Reuse Research Funding Opportunity	$2 mil	$400,000 for pilot projects: 75%
U.S. Army Corps of Engineers (USACE)	Water Resources Reform and Development Act (WRRDA)	TBD	TBD

SWRCB Grants and Loans

Facilities Planning Grant Program

Grants are available to assist public agencies in determining the feasibility of using recycled water and selecting a recommended alternative to offset and augment the use of fresh/potable water from state and/or local supplies. This excludes pollution control studies for which water recycling is an alternative. Recycled water sources may be treated municipal wastewater and/or treated groundwater from sources contaminated due to human activities. Grants will cover 50% of eligible costs up to $75,000. Planning costs incurred before the Study Scope is approved are ineligible. Grant funds will be provided in two disbursements. Grant funds can be leveraged with or a combination of CWSRF and/or Prop 1 loans. CWSRF financing can be used as all or part of the local match requirement.

A Facilities Planning Grant may be a potential funding source for the **GRR Feasibility Study**.

Water Recycling Construction Funding Program

Grants are available for the construction of water recycling facilities to offset and augment state fresh water supplies. Funding will be provided to projects that: 1) provide direct benefit to and/or submitted by a severely disadvantaged community (DAC), and/or support the human right to water; 2) DPR projects; 3) IPR projects; 4)recycled water distribution systems; 5) groundwater recharge facilities; 6) recycled water treatment facilities. Recycled water sources may be treated municipal wastewater and/or treated groundwater from sources contaminated due to human activities. Funding is also available for pilot projects for new potable reuse that will develop new information that does not currently exist and increase the body of knowledge regarding technologies that help the understanding of how potable reuse can effectively be achieved through the innovative application of current and new technologies.

Construction grant funding will be made available on an individual project basis and will depend upon the availability of grant funds and the applicant's readiness to proceed. Construction grants will be limited to 35% of eligible construction costs up to $15 million. DACs may receive grants of up to 40% eligible costs, up to a maximum $20 million. Pilot projects may receive funds up to 35% of eligible costs, up to $1 million. Overall, there is a 50% minimum match requirement. CWSRF financing can be used as all or part of the local match requirement.

There are four packages required for submittal to the SWRCB to complete the full application:

1. General package (2 page preliminary application)
2. Project Report
3. Financial Security package (proof of funding)
4. Environmental package (MND/CEQA)

Preliminary applications for **Phase 2 projects** have been submitted to the SWRCB. Once all four packages are submitted, the SWRCB attorneys work with agencies to execute an agreement.

Clean Water State Revolving Fund (CWSRF) Loan

The SWRCB offers low cost financing for a wide variety of water quality projects. The program has significant financial assets, and is capable of financing projects from <$1 million to >$100 million. Loans are available for water reclamation projects and water recycling at a rate of half the state's most recent general obligation bond rate with a maximum term of 30 years.

CWSRF financing can be used as all or part of the local match requirement for water recycling construction grants. Proposition 1 and CWSRF loans can provide up to 100 percent of eligible construction costs.

Phase 2 Projects may benefit from combining a CWRSF loan with a Water Recycling Construction grant through the SWRCB.

Stormwater Grant Program (SWGP)

Grants will be available through this program for multi-benefit stormwater management projects. Planning grants are available for development of Storm Water Resource Plans. Implementation grants are available for green infrastructure, rainwater and stormwater capture, and stormwater treatment facilities, with the intent to reduce and prevent stormwater contamination of rivers, lakes, and streams. To be eligible, implementation projects must be included in an Integrated Regional Water Management Plan (IRWMP) and Stormwater Resource Plan.

A total of $200 million from Proposition 1 is being allocated to this program. There will be a 50% cost share requirement, which can be waived or reduced for DACs. Minimum planning grant will be $50,000; maximum will be $500,000. Minimum implementation grant will be $250,000; maximum will be $10,000,000.

A **GRR Project** in partnership with the LACFCD could be competitive for this type of SWGP planning grant. The planning grant for this particular fund was due in March 2016; however, a second round of funding is anticipated in 2018.

Department of Water Resources (DWR) Grant

2016 Prop 1 Integrated Regional Water Management Grant Program

The IRWM Grant Program is designed to encourage integrated regional strategies for management of water resources by providing funding for projects and programs that support integrated water management. Specific programs to be funded include: the Disadvantaged Community Involvement Program ($51 million), Planning Grant Program ($5 million), and the Implementation Grant Program ($418 million). Proposition 1 (Water Code §79744) authorized $510 million for projects that are included in and implemented in an adopted IRWM plan that is consistent with Water Code §10530, et seq., and respond to climate change and contribute to regional water security. There is a 50% cost share requirement.

If a **GRR Project** is found to be feasible, this type of project may be competitive for future IRWM grants due to the regional and multi-benefit nature of the project.

U.S. Bureau of Reclamation

The U.S. Bureau of Reclamation's water reclamation and reuse grant program was developed via the Reclamation Wastewater and Groundwater Study and Feasibility Act of 1992 (Title XVI of Public Law [P.L.] 102-575, as amended). This program investigates and identifies opportunities for reclamation and reuse of municipal, industrial, domestic, and agricultural wastewater, and naturally impaired ground and surface waters, for the design and construction of demonstration and permanent facilities to reclaim and reuse wastewater, and to conduct research, including desalting, for the reclamation of wastewater and naturally impaired ground and surface water. The Act also provides a program for federal participation (through cost sharing) of specific water reuse projects up to certain amounts specified in the Act. Construction funds can be provided only for projects specifically authorized by Congress pursuant to Title XVI, although at times USBR comes out with grants for project studies and research such as the following:

Title XVI Feasibility Studies (WaterSMART) - Provides funding for a water reuse project that reclaims and reuses municipal, industrial, domestic, or agricultural wastewater and naturally impaired ground water and/or surface waters. There may be an opportunity to match a SWRCB planning grant with a WaterSMART grant to help fund a GRR Feasibility Study depending on the timing and availability of funds through this program.

Title XVI Water Reuse Research Funding Opportunity - Funding to investigate opportunities to reclaim and reuse wastewaters and naturally impaired ground and surface water in the 17 Western states and Hawaii. If a potable reuse project is pursued, a research grant such as this may be beneficial to support a pilot project or related research.

Grants such as these could offer an opportunity to fund the **GRR Feasibility Study** or follow up research projects on **potable reuse.** The availability and applicability of these grants would need to be assessed at a later date.

U.S. Army Corps of Engineers

Water Resources Reform and Development Act (WRRDA)

This program is still being developed but is intended to provide money to assist in the design and construction of water related infrastructure. The WRDA of 2016 was approved by the full House Committee on Transportation and Infrastructure on May 25, 2016. It is now awaiting consideration by the full House of Representatives. Once approved by the House, differences with the Senate passed version of the bill will need to be reconciled. Many are hopeful that the bill will be passed by the Congress and sent to the President before the August Congressional recess.

WRDA would require the Army Corps to develop criteria for feasibility studies that project proponents will submit to the Army Corps. Depending on the timing and outcome of the bill, WRDA may provide an opportunity to fund construction of **Phase 2 Projects** or future recycled water infrastructure.

Long-Term Debt Issuance

CLWA has used long-term debt issuance to fund CIP in the forms of Certificates of Participations (COPs) or revenue bonds. Long-term debt issuance is used to smooth out cash flow issues and to ensure future users pay for long-term facilities; sometimes referred to as "generational equity".

Revenue Sources for Loan/Debt Service Repayment

Capital costs and debt services associated with CLWA's capital improvement program are allocated to existing users and new growth. Costs attributable to existing users are funded by the one percent property tax, interest on investment, reserves, and standby charges, if levied. Costs attributable to new growth are funded by facility capacity fees.

Recommended Funding and Financing Approach

Due to the numerous grant options and low interest loan programs, CLWA should consider maintaining its accumulated reserves for other purposes and financing the recycled water project through available grant monies and loan programs.

- For the **Phase 2 Projects**: A preliminary application for a SWRCB construction grant has already been submitted and CLWA and the project proponents are committed to completing the four part application package once design and environmental work is ready. A CWSRF loan should also be considered, and could be used as a match against the SWRCB construction grant. CLWA should continue to track other funding opportunities that offer construction grants and loans as these projects become shovel ready.
- For the **GRR Feasibility Study**: Depending on the timing and participants in the study, a SWRCB Facilities Planning Grant or Stormwater Planning Grant, DWR IRMP Grant or USBR Title XVI WaterSMART grant may present opportunities to obtain partial funding.

It should be noted that the loan programs and majority of grant programs are not retroactive; therefore, the sponsoring agency must approve the project prior to the applicable phase (e.g., feasibility study, planning, and construction). It is recommended that coordination with each sponsoring agency listed in Table 9-1 occur immediately following project approval to confirm eligibility, application requirements and deadlines.

9.2 Recycled Water Pricing Policy

This section provides a high-level discussion of typical recycled water rate structures and current practices for recycled water rates in Santa Clarita Valley. Recycled water rate structures vary from agency to agency and may be tied to the cost of services, tied to potable rates, assigned by user class or linked to volume of use.

In addition to project costs, some factors to consider when setting recycled water rates are:

- Providing incentive for customers to use recycled water.
- Providing support for customers who are performing their own retrofits.
- Providing contractual supply reliability benefit to customers to use recycled water (i.e., drought-proof supply for irrigation sites).
- Establishing an ordinance to require the use of recycled water if available, rather than relying solely on pricing incentives and voluntary connections.

Rates Based on Costs of Service

One approach to setting the recycled water price is to set the wholesale recycled water rate at a level to recover costs of furnishing the recycled water. Table 9-2 summarizes the annualized costs for the recommended Phase 2 Projects in 2015 dollars.

Table 9-2: Summary of Annualized Costs for Recommended Phase 2 Projects

Facility Component	Recommended Phase 2 Projects			
	Phase 2A	**Phase 2B**	**Phase 2C**	**Phase 2D**
	Central Park South w/ Tank	Combined SCWD + Vista Canyon	VWC + NCWD Extensions	VWC Extension
Total Construction Cost ($mil)	**$24.8**	**$7.1**	**$23.5**	**$3.3**
Annualized Unit Construction Cost ($/AF)	$2,600	$1,400	$1,000	$1,000
Annual O&M Cost ($/AF)	$560	$260	$270	$660
Total Annual Cost ($/AF)	**$3,160**	**$1,660**	**$1,270**	**$1,660**

* Note: Annual O&M includes the cost to purchase tertiary recycled water from the SCVSD.

The estimated annualized capital and operating cost of the Phase 2 projects range from $1,300 to $3,200 per AF, which is significantly greater than the current $530 per AF average wholesale rate VWC pays for recycled water, which is based on an 80% of the retail rates for potable water.

Regardless of the program utilized to finance the recycled water system, the basic source of funds for CLWA's portion of facilities would be the facility capacity fees, standby charges, property taxes, and water rates currently collected by CLWA. Based on this system of financing, it is not necessary to include annualized capital in the cost of service since the capital costs do not need to be recovered. The estimated cost for the recycled water system excluding annualized capital costs for the Phase 2 projects range from $260 to $660 per AF, which is comparable to or lower than the current wholesale rate for recycled water.

The funds contributed by project proponents for their portion of facilities may rely upon a project specific finance program.

Rates Based on Percentage of Potable Water Rate

Although the wholesale recycled water rate should reflect the actual cost of providing service, it may be preferable for CLWA to base its recycled water rate on a percentage of the potable water rate. This is desirable when a straightforward method of calculation is preferred. Often, this method is necessary because the rate based upon costs of service exceeds the potable water rate. Based on the need to provide an incentive to utilize recycled water, a recycled water rate of 70 to 90 percent of the potable water rate is typical.

Rates by User Class

A method used by some water agencies for setting recycled water rate is to establish different rates for various user categories. For example, the Irvine Ranch Water District charges a rate for commercial/landscape users, including homeowner associations, that is approximately nine percent greater than the rate charged for the larger/agricultural users. Because the cost of furnishing recycled water would not differ substantially between types of customers in Santa Clarita Valley, it seems appropriate for users of CLWA's recycled water system to be initially charged at the same rate. However, a rate surcharge may be appropriate for users of high-pressure water since pumping costs are higher. This would need to be assessed on a project specific basis for future extensions of the recycled water system.

Rates by Volume of Use

Another concept for rate setting is to apportion rates based the volume of recycled water used. **Tiered rate structures** are commonly used for potable rate setting; where a rate per unit is tiered according to demand levels. Another approach is **Allocation-based rates**, for which a rate per unit is tiered according to an allocation for each customer. This typically requires a customer site assessment to set the allocation. A more common practice for recycled water is to provide **Uniform rates** (not tiered), where the rate per unit of water consumed does not vary with the amount of water use. This may serve to emphasize the benefit of recycled water as reliable and consistent source of water that is not subject to drought and conservation requirements.

Potential Rate Considerations

CLWA's historical recycled water rate structure is at 80% of the potable rate. Over the last ten years the average rate for recycled water deliveries purchased by VWC has been approximately $430 per AF. CLWA expenditures during this same period averaged approximately $140 per AF and the unit cost of recycled water purchased from the SCVSD averaged approximately $150 per AF.

CLWA and the purveyors generally agreed that a uniform wholesale rate for recycled water provided by CLWA to its Purveyors should be cost-based and uniformly applied. It was also discussed that it would be beneficial for the retail rate for recycled water provided by the Purveyors within their Service Areas to be set at a price below the cost of potable water to encourage recycled water use. To the extent feasible, the Purveyors should seek to apply a consistent discount below the potable rate to their customers and participate in coordinated Valley-wide messaging regarding recycled water benefits and costs.

The recycled water rate structure was supposed to be studied as part of the 2012 wholesale water rate study. However, due to delays in constructing the Phase 2 projects, this rate study has been deferred (CLWA 2016). It is recommended that a comprehensive recycled water rate study be performed to identify the optimal pricing policy for recycled water in Santa Clarita Valley for CLWA and the Purveyors.

9.3 Avoided Costs and Lost Revenues

Accounting for avoided costs and lost revenues can provide a more comprehensive view of the true cost and benefit of expanding the recycled water program. The potential cost savings and revenue losses should be considered when conducting a rate study to provide a complete financial picture for the future.

The primary avoided cost to any recycled water system is the cost of the potable supply that is being offset by reuse. For Santa Clarita Valley, that offset may be associated with reduced groundwater pumping, reduced potable purchases or a combination of the two. The cost of reduced groundwater pumping could be estimated based on the energy required to extract and convey groundwater plus the proportionate annual cost to maintain those facilities. The cost of reduced potable purchases of SWP water due to recycled water use could be estimated based on the current rate for SWP and a projected increase over the life of the project.

Lost revenue from potable sales (CLWA and the Purveyors selling less potable water due to recycled water offset) should also be accounted for in the overall financial assessment.

Additional study of avoided costs and lost revenues should be considered as part of the recycled water rate study to predict how wholesale and retail rates are projected to increase over time.

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Appendix A: Recycled Water Supply and Demands

This appendix includes supporting information for the recycled water market assessment. The following tables are included:

Table A-1: Projected Available Recycled Water Supply

Table A-2: Historical Recycled Water Demands (AFY)

Table A-3: Existing Phase 1 Recycled Water Meters

Table A-4: Anticipated Phase 2A Recycled Water Demands

Table A-5: Anticipated Phase 2B Recycled Water Demands

Table A-6: Anticipated Phase 2C Recycled Water Demands

Table A-7: Anticipated Phase 2D Recycled Water Demands

Table A-8: Potential Future Alignment Recycled Water Demands - Alignment A

Table A-9: Potential Future Alignment Recycled Water Demands - Alignment B

Table A-10: Potential Future Alignment Recycled Water Demands - Alignment C

Table A-11: Potential Future Alignment Recycled Water Demands - Alignment D

Table A-12: Potential Future Alignment Recycled Water Demands - Alignment E

Table A-13: Potential Future Alignment Recycled Water Demands -Alignment F

Table A-14: Potential Future Alignment Recycled Water Demands - Alignment G

Table A-15: Potential Future Alignment Recycled Water Demands -Alignment H

Demand date provided for non-potable reuse is based on 2013 meter data.

Table A-1: Projected Available Recycled Water Supply

Year	Projected Wastewater Influent based on Population (mgd)[a]	Anticipated Discharge Requirement (mgd)[b]	Projected Available RW Supply (mgd)[c]	Projected Available RW Supply (AFY)[c]
2015	18.6	13	5.6	6,268
2016	18.8	13	5.8	6,510
2017	19.0	13	6.0	6,752
2018	19.2	13	6.2	6,993
2019	19.5	13	6.5	7,235
2020	19.7	13	6.7	7,477
2021	20.1	13	7.1	7,954
2022	20.5	13	7.5	8,432
2023	21.0	13	8.0	8,909
2024	21.4	13	8.4	9,387
2025	21.8	13	8.8	9,865
2026	22.2	13	9.2	10,341
2027	22.7	13	9.7	10,817
2028	23.1	13	10.1	11,293
2029	23.5	13	10.5	11,769
2030	23.9	13	10.9	12,245
2031	24.3	13	11.3	12,666
2032	24.7	13	11.7	13,087
2033	25.1	13	12.1	13,507
2034	25.4	13	12.4	13,928
2035	25.8	13	12.8	14,349
2036	26.0	13	13.0	14,533
2037	26.1	13	13.1	14,716
2038	26.3	13	13.3	14,899
2039	26.5	13	13.5	15,083
2040	26.6	13	13.6	15,266
2041	26.8	13	13.8	15,451
2042	27.0	13	14.0	15,636
2043	27.1	13	14.1	15,821
2044	27.3	13	14.3	16,006
2045	27.5	13	14.5	16,191
2046	27.6	13	14.6	16,374
2047	27.8	13	14.8	16,558
2048	27.9	13	14.9	16,741
2049	28.1	13	15.1	16,925
2050	28.3	13	15.3	17,108

a) Based on a 65 gpcd wastewater generation rate multiplied by the projected population

b) Assumes that SCVSD will be required to maintain 8.5 mgd from the Valencia WRP and 4.5 mgd from the Saugus WRP for river discharge to the Santa Clara River

c) Includes projected recycled water produced at the Valencia WRP, Saugus WRP, planned Newhall WRP and planned Vista Canyon Water Factory.

Table A-2: Historical Recycled Water Demands (AFY)

Month	2003	2004	2005	2006	2007	2008	2009	2010	2011	2012	2013	2014	2015	Ave
Jan		1	4	14	17	4	8	7	8	22	17	21	12	11
Feb		2	2	16	14	11	3	1	12	16	20	16	11	10
Mar		24	10	6	43	39	24	2	10	25	38	26	24	23
Apr		53	36	12	38	37	39	0	38	30	51	46	38	35
May		55	46	42	58	56	30	51	41	58	58	64	55	51
Jun		58	59	66	63	34	46	56	54	64	64	58	61	57
Jul		64	67	75	78	26	71	54	64	68	27	64	65	60
Aug		61	57	63	67	63	59	60	57	67	41	60	65	60
Sep	31	90	66	67	55	44	17	39	54	60	37	47	55	51
Oct	61	26	39	33	37	38	39	22	37	32	38	40	40	37
Nov	11	0	20	20	25	4	18	11	10	17	9	23	15	14
Dec	2	14	21	12	7	1	9	5	11	2	16	0	9	8
Total	**107**	**448**	**427**	**426**	**501**	**358**	**364**	**307**	**396**	**462**	**416**	**465**	**450**	**417**

Source: Monthly data as reported by CLWA

Table A-3: Existing Phase 1 Recycled Water Meters

Purveyor	Address	Meter No.
VWC	25700 VALENCIA BLVD, TPC LAKE	36544302
VWC	26840 THE OLD RD, #6204786	37565301
VWC	27236 THE OLD RD, #6203749	37571301
VWC	27231 THE OLD RD, #4110307	37567300
VWC	27009 THE OLD RD, #6203745	37568301
VWC	26853 THE OLD RD, #4110303	37569301
VWC	26848 THE OLD RD, #6204785	37564301
VWC	27061 THE OLD RD, #4110306	37566302
VWC	27233 THE OLD RD, #6203748	37570302
VWC	27347 THE OLD RD, #6203748	40289302
VWC	27345 THE OLD RD, #6204782	40291302
VWC	27545 THE OLD RD, #6204783	40290302
VWC	27640 THE OLD RD, #6203751	40288302

Table A-4: Anticipated Phase 2A Recycled Water Demands

Purveyor	Address	Meter No.	Demand (AF)
SCWD		69496998	5.96
SCWD		1565908	2.75
SCWD		1565977	145.47
SCWD		67298978	0.25
SCWD		67298996	1.66
SCWD		67298983	15.68
SCWD		67298991	11.21
SCWD		67298986	6.80
SCWD		67298993	3.48
SCWD		67298985	4.69
SCWD		67298987	9.21
SCWD		67298984	5.62
SCWD		68837441	11.36
VWC	27931 KELLY JOHNSON PKWY	2866039028	24.10
VWC	24023 NEWHALL RANCH RD	115540039	20.00
VWC	24003 NEWHALL RANCH RD	115533486	4.39
VWC	23902 NEWHALL RANCH RD	115531358	8.85
VWC	23904 NEWHALL RANCH RD	115531357	4.24
VWC	23660 NEWHALL RANCH RD	2811071901	12.42
VWC	23650 NEWHALL RANCH RD	115531364	5.77
VWC	24156 NEWHALL RANCH RD	115538406	2.76
VWC	24158 NEWHALL RANCH RD	2811062904	2.33
VWC	27601 HILLSBOROUGH PKWY	115525602	5.83
VWC	27560 NEWHALL RANCH RD	2810067011	7.74
VWC	27260 NEWHALL RANCH RD	2810043070	2.72
VWC	28188 NEWHALL RANCH RD	2840120004	9.98
VWC	25190 RYE CYN RD	115534340	3.86
VWC	28031 NEWHALL RANCH RD	2866040016	10.57
VWC		2866006055	8.93
VWC	23518 NEWHALL RANCH RD	115531363	9.68
VWC	23528 NEWHALL RANCH RD	115531363	15.44
VWC	23410 NEWHALL RANCH RD	115531362	7.89
VWC	23657 NEWHALL RANCH RD	2811001283	11.48
VWC	23655 NEWHALL RANCH RD	115525641	26.69
VWC	27355 MCBEAN PKWY	115536329	7.97
VWC	27300 MCBEAN PKWY	115515052	13.86
VWC	27304 MCBEAN PKWY	115515050	3.81

Table A-4: Anticipated Phase 2A Recycled Water Demands (con't)

Purveyor	Address	Meter No.	Demand (AF)
VWC		2811032055	9.23
VWC	24007 FAIRVIEW DR	2810070004	48.49
VWC	23893 FAIRVIEW DR	2810070900	2.73
VWC		115531357	4.24
VWC	25273 RYE CANYON RD	2866010006	3.27
VWC	27819 SMYTH DR	2810043081	1.30
VWC	27690 NEWHALL RANCH RD	2810043070	2.72
VWC	27751 DICKASON DR- #4742849	115525465	4.14
VWC	27879 NEWHALL RANCH RD	115540026	12.44
VWC		2866006055	4.10
VWC	24602 DICKASON DR	2810043060	0.63
VWC	27213 MCBEAN PKWY	115533487	6.97
VWC	26453 BOUQUET CYN RD	2811068021	0.63
VWC	26415 BOUQUET CYN RD	2811068036	3.85
	Phase 2A Total Demand		**560**

Table A-5: Anticipated Phase 2B Recycled Water Demands

Purveyor	Address	Meter No.	Demand (AF)
SCWD		74152096	7.16
SCWD		74152095	13.43
SCWD		74152092	18.32
SCWD		62124826	1.51
SCWD		60919566	1.34
SCWD		74152097	13.03
SCWD		74152094	19.21
SCWD		61676863	13.23
SCWD		65652278	20.20
SCWD		67298992	12.65
SCWD		67250431	1.51
SCWD		63416964	14.71
SCWD		65403696	5.65
SCWD		63843027	0.98
SCWD		65447356	2.49
SCWD		71134886	10.63
SCWD	Fair Oaks Community School [a]		7.1
SCWD	Vista Canyon Development [b]		137
	Phase 2B Total Demand		**300**

a. Fair Oaks Community School (Estimated in Phase 2B Preliminary Design Report dated 10/2015)

b. Vista Canyon Development to utilize 137 AF

Table A-6: Anticipated Phase 2C Recycled Water Demands

Purveyor	Address	Meter No.	Demand (AF)
NCWD	24607 WALNUT ST	2920404091	20.46
NCWD	24607 WALNUT ST	2920404090	20.46
NCWD	NEWHALL PARK	2920304102	16.96
NCWD	24923 NEWHALL AVE	2920304126	5.77
NCWD	NEWHALL PARK	2920304128	16.96
NCWD	NEWHALL PARK	2920304129	16.26
NCWD	24825 NEWHALL AVE	2920304228	95.13
NCWD	25017 NEWHALL AVE	0	23.31
NCWD	25017 1/2 NEWHALL AVE	0	22.81
NCWD	25015 1/2 NEWHALL AVE	0	6.06
NCWD	25015 NEWHALL AVE	0	4.37
VWC	26700 SPRINGFIELD CT - #3376444	2861060105	2.27
VWC	25752 SPRINGFIELD RD	2861060104	0.33
VWC	24928 IRONWOOD DR - #2835065	115532381	7.52
VWC	26819 WOODLANDS DR - #785865	115530882	11.96
VWC	25752 SPRINGFIELD RD - #2984633	2861060104	0.33
VWC	26700 SPRINGFIELD CT - #3376444	2861060105	2.27
VWC	26809 GOLDCREST DR #785095	2861065004	13.17
VWC	25330 SILVER ASPEN - #4380067	115536851	15.53
VWC	26650 THE OLD RD - #4380069	2826142015	3.74
VWC	25816 TOURNAMENT RD - #6201462	2858018043	1.77
VWC	25659 ORCHARD VILLAGE RD	115521866	10.03
VWC	23875 VIA JACARA	115521903	0.61
VWC	24506 MCBEAN PKY	115521765	10.48
VWC	23578 VIA BARRA - #3376528	115521894	4.02
VWC	25375 AVE RONADA - #4177390	2856001024	2.67
VWC	25372 AVE RONADA - #4804323	115521914	0.11
VWC		116621781	4.82
VWC	25840 TOURNAMENT RD - #1858488	2858018047	5.13
VWC	24710 MCBEAN PKWY - #789321	115521762	17.79
VWC	25901 TOURNAMENT RD - #3032202	2851001001	0.74
VWC	25374 AVE RONADA - #3032196	115524303	0.29
VWC	25100 VALENCIA BLVD - #12789094	2861004011	3.99
VWC	24995 VALENCIA BLVD- #3376446	2861060106	4.46
VWC	27000 TOURNEY RD #4911507	2861060012	2.81
VWC	24801 VALENCIA BLVD #3003469	115515009	9.32
VWC	26100 ROCKWELL CYN RD - #1726038	2861037020	0.32

Table A-6: Anticipated Phase 2C Recycled Water Demands (con't)

Purveyor	Address	Meter No.	Demand (AF)
VWC	26102 ROCKWELL CYN RD - #L065270	2861040013	7.33
VWC	26002 ROCKWELL CYN RD - #1520424	2861023045	22.77
VWC	25998 ROCKWELL CYN RD - #2899169	2861005091	18.70
VWC	24699 MCBEAN PKWY - #4278919	2861004071	2.34
VWC	25800 LOCHMOOR/MEADOWS - #4149210	2858007900	14.71
VWC	23773 VIA GAVOLA	115521901	0.20
VWC	25601 AVE JOLITA	2859008010	1.20
VWC	26511 GOLDCREST DR - #5375827	115518348	23.70
VWC	24508 MCBEAN PKY	115521764	12.42
VWC	23752 VIA GAVOLA	2859008900	17.05
VWC	23723 MILL VALLEY RD	115521897	0.38
VWC	25526 LANGSTON ST	115521872	0.36
VWC	25671 FEDALA/MEADOWS	2858004900	2.20
VWC	24001 MCBEAN PKY	115518533	0.29
VWC	24405 MCBEAN PKY	115521860	6.71
VWC	25915 TOURNAMENT RD	115521849	8.32
VWC	25791 TOURNAMENT DR	116621781	4.82
VWC		2851007074	6.74
VWC	26704 Valencia Blvd #3272084		2.80
VWC	26930 The Old Rd #3272068		9.89
VWC	26104 Rockwell Cyn Rd #L065269		0.00
VWC	24712 McBean Pkwy #2984634		16.24
VWC	24700 McBean Pkwy #1280284 - Cal Arts		54.08
VWC	26455 Rockwell Cyn Rd #2083445 - COC		2.92
VWC	26455 Rockwell Cyn Rd #2082687 - COC		0.45
VWC	25000 Valencia Blvd #5083981 - COC		26.16
VWC	26851 The Old Rd #3272080		8.63
VWC	25234 Valencia Blvd #3272070		2.80
VWC	Little V Golf Course (Vista Valencia) [a]		183.17
VWC	Big V Golf Course (Valencia Golf Course) [a]		531.55
	Phase 2C Total Demand		**1,374**

a. 2015 usage from an existing groundwater well

Table A-7: Anticipated Phase 2D Recycled Water Demands

Purveyor	Address	Meter No.	Demand (AF)
VWC	26250 VALENCIA BLVD - #8043193 - Rancho Pico Jr High School	115537416	59.12
VWC	26255 VALENCIA BLVD - #5372477 - Westridge High School	2826009902	73.50
VWC	26750U WESTRIDGE PKWY - #6173013	115535213	0.96
VWC	26762 OLD ROCK RD- #4380070	115536763	0.64
VWC	26760 OLD ROCK RD - #6172254	2826156004	10.36
VWC	26775 OLD ROCK ROAD - #3272099	2826155037	3.06
VWC	26252 Valencia Blvd #4482741		0.17
VWC	26260 Valencia Blvd #3376418		0.00
VWC	26705 Old Rock Rd#3376419		1.17
VWC	26756 Old Rock Rd #3272146		4.07
VWC	26770 Westridge Pkwy #6903696		4.00
VWC	26773 Old Rock Rd #3272128		6.11
VWC	26800 Valencia Blvd #4742882		15.21
VWC	27050 Old Rock Rd #3272064		5.38
VWC	27052 Old Rock Rd #6169191		1.93
	Phase 2D Total Demand		**186**

Table A-8: Potential Future Alignment Recycled Water Demands - Alignment A

Purveyor	Address	Meter No.	Demand (AF)
NCWD		2920607086	10
NCWD		2920305123	3
NCWD		2920607026	12
NCWD		2920507065	8
NCWD		2920507052	12
NCWD		2920406011	0
NCWD		2920406010	2
NCWD		2920406013	2
NCWD		2920506073	11
NCWD		2920405016	0
NCWD		2920507044	9
NCWD		2920507043	5
NCWD		2920507040	11
NCWD		2920507039	10
NCWD		2920406033	0
NCWD		2920406032	0
NCWD		2920406030	0
NCWD		2920406023	2
NCWD		2920406024	4
NCWD		2920406029	2
NCWD		2920406025	7
NCWD		2920406028	1
NCWD		2920406027	1
NCWD		2920406036	0
NCWD		2920406096	10
NCWD		2920507013	9
NCWD		2920406074	13
NCWD		2920406103	53
NCWD		2920406104	53
NCWD		2920406105	53
NCWD		2920406106	53
NCWD		0	1
NCWD		2920507088	9
NCWD		2920507095	8
Alignment A Demands			**374**

Table A-9: Potential Future Alignment Recycled Water Demands - Alignment B

Purveyor	Address	Meter No.	Demand (AF)
NCWD		2920405228	3
NCWD		2920405229	2
NCWD		2920405120	0
NCWD		2920505112	0
NCWD		2920505010	4
NCWD		2920505011	2
NCWD		2920505013	2
NCWD		2920405072	16
NCWD		2920505114	20
NCWD		0	0
	Alignment B Demands		**49**

Table A-10: Potential Future Alignment Recycled Water Demands -Alignment C

Purveyor	Address	Meter No.	Demand (AF)
VWC	25060 SOUTHERN OAKS DR	2826131025	9.62
VWC	25619 MAGNOLIA LN	115532780	13.32
VWC	25648 MORNING MIST DR	2826124013	10.46
VWC	0 PICO CANYON MEDIAN	115538940	0.82
VWC	24880 SOUTHERN OAKS DR	115533406	9.75
VWC	25536 FOUNTAIN GLEN CT	2826085014	15.43
VWC	24979 CONSTITUTION AVE	115538961	6.58
VWC	25520 THE OLD RD	2826096011	14.32
VWC	24959 PICO CYN RD	2826085005	11.21
VWC	25932 THE OLD RD	2826095005	5.85
VWC	24979 CONSTITUTION AVE	2826085022	9.28
VWC	25950 THE OLD RD- MANIFOLD 33	2826095011	12.32
VWC	25313 PICO CANYON RD U	2826160900	13.78
VWC	25205 GLORISO LN U	115539310	8.42
VWC	25210 GLORISO LN U	115539311	11.89
VWC	25306 PICO CANYON RD U	2826133005	7.63
VWC	24800 GREENSBRIER DR	115533919	11.21
VWC	24801 GREENSBRIER DR	115533920	0.56
VWC	25051 WHISPERING OAKS DR	115535631	11.95
VWC	25790 WHISPERING OAKS RD U	115534034	3.87
VWC	24894 SOUTHERN OAKS DR U	115533313	9.49
VWC	25751 PICO CANYON RD	115535067	0.20
VWC	25790 PICO CANYON RD	2826097004	5.93
VWC	25577 HUXLEY DR	115538732	2.27
	Alignment C Demands		**206**

Table A-11: Potential Future Alignment Recycled Water Demands - Alignment D

Purveyor	Address	Meter No.	Demand (AF)
VWC	25900 BELLIS DR	115518519	30.75
VWC	23636 MAGIC MOUNTAIN PKWY - 992928	118818471	16.43
VWC	24452 VALENCIA BLVD - #1104385	2861057074	10.19
VWC	24375 VALENCIA BLVD	2861062900	1.83
VWC	26250 CITRUS STR	2861009038	1.29
VWC	24053 VALENCIA BLVD	115524488	5.03
VWC	24100 ARROYO PARK DR	2861027053	34.13
VWC	24443 ARROYO PARK DR	2861024041	10.96
VWC	23807 MAGIC MOUNTAIN PKWY	2811002063	11.10
VWC	26201 MCBEAN PKWY	115518366	28.79
VWC	26120 MCBEAN PKWY	115518393	5.08
VWC	24182 VALENCIA BLVD - #4221772	2861026020	17.55
VWC	26822 GOLDCREST DR - #782308	115530155	12.19
VWC	24442 VALENCIA BLVD - #1108706	2861057001	9.33
VWC	24184 VALENCIA BLVD	2861026021	13.78
VWC	24419 ARROYO PARK DR	2861005073	3.32
VWC	24182 DEL MONTE DR	2861051014	14.16
VWC	23920 VALENCIA BLVD	2861026909	6.19
VWC	23973 ARROTO PARK DR	2861052003	14.63
VWC	24031 ARROYO PARK DR	2861029042	7.08
VWC	24095 ARROYO PARK DR	2861030065	3.82
VWC	24102 ARROYO PARK DR	2861025026	19.13
VWC	24251 ARROYO PARK DR	2861024039	14.40
VWC	24402 ARROYO PARK DR	2861024040	10.76
VWC	24421 ARROYO PARK DR	2861023063	5.25
VWC	24100 KIRSTENGEARY WY	2861030067	20.23
VWC	26110 MCBEAN PKY	2861051015	0.66
VWC	26410 MCBEAN PKY	115524649	3.30
VWC	26412 MCBEAN PKY	115524650	10.84
VWC	23977 ARROYO PARK DR	116618511	7.47
VWC	26131 MCBEAN PKY	116618509	11.83
VWC	24025 ARROYO PARK DR	116618513	16.86
VWC		2861027055	2.70
VWC		2861035140	7.26
	Alignment D Demands		**388**

Table A-12: Potential Future Alignment Recycled Water Demands - Alignment E

Purveyor	Address	Meter No.	Demand (AF)
VWC	28132 KELLY JOHNSON PKWY	115532589	10.56
VWC	28205 KELLY JOHNSON PKWY	2866048031	4.10
VWC	28188 NEWHALL RANCH RD	2840120004	9.98
VWC	27931 KELLY JOHNSON PKWY	2866039028	24.11
VWC	27926 KELLY JOHNSON PKWY	2866039023	17.94
VWC	28323 KELLY JOHNSON PKWY'	2866048022	14.45
VWC	28310 KELLY JOHNSON PKY	2866047034	10.11
VWC	28851 RIO NORTE DR	115539431	13.89
VWC	28801 RIO NORTE DR	2810111006	11.39
VWC		2810110011	3.59
VWC	25112 AURORA DR	2866039030	7.44
VWC	23449 COPPER HILL DR	115528232	3.47
VWC	23975 U COPPER HILL DR	2810118028	10.14
VWC	23500 COPPER HILL DR	115528229	1.08
VWC	23501 COPPER HILL DR	3244159068	9.14
VWC	23502 COPPER HILL DR	2810081061	12.51
VWC	23451 COPPER HILL DR	3244177034	6.55
VWC	24015 COPPER HILL DR	2810119014	15.16
VWC		2810111218	5.32
NCWD		2940205004	15
NCWD		2940305118	8
NCWD		2940404090	3
NCWD		2940404091	7
NCWD		2940305459	11
NCWD		2940205003	14
NCWD		2940305117	10
NCWD		2940304294	9
NCWD		2940304043	2
NCWD		2940304044	3
NCWD		2940304046	2
NCWD		2940404019	7
NCWD		2940404021	11
NCWD		2940304113	19
NCWD		2940405165	4
NCWD		2940304170	6
NCWD		2940304365	9
NCWD		2940304011	11
NCWD		2940305010	20
NCWD		2940305052	5
NCWD		2940305389	1

Table A-12: Potential Future Alignment Recycled Water Demands - Alignment E (con't)

Purveyor	Address	Meter No.	Demand (AF)
NCWD		2940304012	7
NCWD		2940404022	7
NCWD		2940404020	1
NCWD		2940404018	5
NCWD		2940405008	2
NCWD		2940405162	5
NCWD		2940405160	2
NCWD		2940405159	9
Alignment E Demands			**406**

Table A-13: Potential Future Alignment Recycled Water Demands - Alignment F

Purveyor	Address	Meter No.	Demand (AF)
VWC	27745 MCBEAN PKWY	2810071271	0.97
VWC	27370 SHELBURNE DR	2811049066	7.13
VWC	23700 DECORO DR	2811045062	10.06
VWC	27404 HILLSBOROUGH PKWY	115515086	7.04
VWC	23699 DECORO DR	115536155	8.83
VWC	27350 HILLSBOROUGH PKWY	2811050064	5.15
VWC	27216 BLUERIDGE DR	2810032031	13.16
VWC	27205 BLUERIDGE DR	115524557	7.70
VWC	23102 DECORO DR	2811051017	11.36
VWC	23100 DECORO DR	2811051016	6.25
VWC	27501 MCBEAN PKWY	115535264	2.74
VWC	27508 GRANDVIEW DR	2811047063	4.17
VWC	23700 DECORO DR	2811045062	10.06
VWC	27502 HILLSBOROUGH PKWY	115515087	16.05
VWC	27397 MCBEAN PKY	2811043037	14.84
VWC	27399 MCBEAN PKY	2811044072	12.47
VWC	27302 MCBEAN PKY	115515051	4.30
VWC	23698 DECORO DR	2810071271	56.89
Alignment F Demands			**199**

Table A-14: Potential Future Alignment Recycled Water Demands - Alignment G

Purveyor	Address	Meter No.	Demand (AF)
VWC	22605 COPPER HILL DR	3244108018	4.21
VWC	23199 COPPER HILL DR	3244108011	2.25
VWC	23201 COPPER HILL DR	115527603	5.37
VWC	27795 MCBEAN PKWY	2810041040	14.95

Table A-14: Potential Future Alignment Recycled Water Demands - Alignment G (con't)

Purveyor	Address	Meter No.	Demand (AF)
VWC	27857 MCBEAN PKY	115525441	4.50
VWC	27745 MCBEAN PKWY	2810071271	0.97
VWC	28069 SUNSET HILLS DR	2810044096	4.13
VWC	27857 MCBEAN PKWY	115525441	4.50
VWC	27855 MCBEAN PKWY	2810046058	9.87
VWC	28575 SECO CANYON RD	115515204	18.08
VWC	28573 SECO CYN RD	3244027034	18.15
VWC	28600 SECO CYN RD	115515205	8.08
VWC	22650 HAZEL ST	115515202	9.79
VWC	28250 NORTHPARK DR	2810056034	12.05
VWC	27970 NORTHPARK DR	2810050021	5.88
VWC	27969 NORTHPARK DR	2810046055	6.14
VWC	27969 NORTHPARK DR	115526485	9.47
VWC	27810 AMBERWOOD LN	115526295	9.49
VWC	27810 AMBERWOOD LN	115526296	5.05
VWC	28023 NORTHPARK DR	2810044099	7.05
VWC	28113 NORTHPARK DR	115527600	6.95
VWC	28399 SECO CANYON RD	115515224	9.64
VWC	28344 SECO CANYON RD	115515227	20.42
VWC	22809 BANYAN PL	115515228	10.13
VWC	22828 BANYAN PL	115515239	9.41
VWC	27915 NORTHPARK DR- #1713665	2810055013	16.50
VWC	28117 SECO CYN RD	115515241	9.12
VWC	28025 SECO CYN RD	3244070003	2.06
VWC	28122 SECO CYN RD	115515183	5.38
VWC	28048 SECO CANYON RD	111215243	1.37
VWC	27915 NORTHPARK DR	115526436	10.21
VWC	27915 NORTHPARK DR	2810055013	16.50
VWC	28053 TUPELO RIDGE DR	2810056036	2.81
VWC	28249 NORTHPARK DR	2810060021	3.50
VWC	28501 MCBEAN PKY	115536787	7.45
VWC	22591 PECAN PL	115515186	2.79
VWC	28131 TAMARACK LN	115515188	20.14
NCWD		68389255	5
NCWD		1105512	0
NCWD		1105515	0
	Alignment G Demands		**319**

Table A-15: Potential Future Alignment Recycled Water Demands - Alignment H

Purveyor	Address	Meter No.	Demand (AF)
SCWD		70487889	0.13
SCWD		65652901	0.42
SCWD		67246068	0.16
SCWD		720030187	2.76
SCWD		66214836	1.22
SCWD		1623300	88.23
SCWD		62558851	0.84
SCWD		69676646	27.75
SCWD		68529604	2.28
SCWD		70237827	1.84
SCWD		71447009	1.50
SCWD		720030184	0.93
SCWD		65068863	1.10
SCWD		65068860	2.39
SCWD		68529605	1.93
SCWD		68165270	2.39
SCWD		58902560	0.25
SCWD		72030170	2.83
SCWD		70237797	0.00
SCWD		70237808	11.57
SCWD		65652902	19.01
SCWD		70237807	23.46
SCWD		64244304	12.86
SCWD		70066796	9.83
SCWD		71904550	4.32
SCWD		72050863	5.45
SCWD		62699139	2.34
SCWD		67246061	5.15
SCWD		68837404	3.43
SCWD		1066379	0.00
SCWD		69568549	4.42
SCWD		71904659	12.67
SCWD		61676853	1.37
SCWD		62124816	8.71
SCWD		70066805	6.29
SCWD		62720448	8.96
SCWD		67246055	8.85
SCWD		62720446	9.77
SCWD		68837460	10.13
SCWD		62720444	7.70

Table A-15: Potential Future Alignment Recycled Water Demands - Alignment H (con't)

Purveyor	Address	Meter No.	Demand (AF)
SCWD		68529574	9.19
SCWD		68529589	8.48
SCWD		70237786	4.09
SCWD		70487890	6.83
SCWD		71446979	5.59
SCWD		65670328	1.05
SCWD		67250421	7.07
SCWD		68165252	8.62
SCWD		71468733	2.04
SCWD		63454356	0.79
SCWD		65651663	1.37
SCWD		71904547	4.90
SCWD		62124799	3.37
SCWD		64288169	0.98
SCWD		65651693	2.26
SCWD		63416975	14.32
SCWD		63416941	14.25
SCWD		65670324	8.85
SCWD		71367006	0.24
	Alignment H Demands		**419**

Appendix B: Summary of Recycled Water Regulations

B.1. Federal Requirements

Federal requirements relevant to the discharge of recycled water, or wastewater, and any other liquid wastes to "navigable waters" are contained in the 1972 amendments to the Federal Water Pollution Control Act of 1956, commonly known as the federal Clean Water Act (CWA) (Public Law 92-500). The CWA created the U.S. Environmental Protection Agency (USEPA) and established the National Pollutant Discharge Elimination System (NPDES), a permit system for discharge of contaminants to navigable waters. NPDES requires that all municipal and industrial dischargers of liquid wastes apply for and obtain a permit prior to initiating discharge.

There are no federal regulations governing water reuse in the United States, thus regulations (or guidelines) for recycled water are developed and implemented at the state government level. The lack of federal regulations has resulted in differing standards among states that have developed recycled water regulations (WateReuse 2009). This appendix focuses on recycled water regulations in the State of California.

Recognizing the need to provide national guidance on water reuse regulations and program planning, the U.S. Environmental Protection Agency (USEPA) has developed comprehensive, up-to-date water reuse guidelines in support of regulations and guidelines developed by states, tribes, and other authorities (USEPA 2012). The 2012 USEPA Guidelines for Water Reuse provides support for both project planners and state regulatory officials by providing a national overview of the status of reuse regulations and clarifying some of the variations in the regulatory frameworks that support reuse in different states and regions of the United States

B.2. State Requirements

In the State of California, recycled water requirements are administered by the State Water Resource Control Board (SWRCB) - Division of Drinking Water (DDW), formerly under California Department of Public Health (CDPH), and individual Regional Water Quality Control Boards (RWQCBs). The regulatory requirements for recycled water projects in California are contained in the following sources[7,8] :

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[7] State requirements for production, discharge, distribution, and use of recycled water are contained in the California Water Code, Division 7-Water Quality, Sections 1300 through 13999.16 (Water Code); the California Administrative Code, Title 22-Social Security, Division 4 Environmental Health, Chapter 3-Reclamation Criteria, Sections 60301 through 60475 (Title 22); and the California Administrative Code, Title 17-Public Health, Chapter 5, Subchapter 1, Group 4-Drinking Water Supplies, Sections 7583 through 7630 (Title 17).

- California Code of Regulations (CCR) -Title 22 and Title 17
- California Health and Safety Code
- California Water Code.

Title 22 State Clean Water Act (CWA)

In 1975, Title 22 was prepared by the CDPH (now DDW[9]) in accordance with the requirements of Division 7, Chapter 7 of the Water Code. In 1978, Title 22 was revised to conform with the 1977 amendment to the federal CWA. The requirements of Title 22, as revised in 1978, 1990, and 2001, regulate production and use of recycled water in California.

The DDW regulates the treatment, quality, and use of recycled water, as well as the proper separation of recycled water and drinking water systems. Title 22 stipulates the levels of treatment for different uses of recycled water, permissible types of reuse, and minimum recycled water quality requirements. Water meeting these standards is considered safe for non-drinking purposes. Routine monitoring is required to ensure that the intended quality is consistently being produced.

Figure B.1 illustrates the allowable uses of recycled water for each level of treatment. Most recycled water used in California meets the Title 22 standards for "disinfected tertiary recycled water", which has the most stringent requirements for non-potable reuse. "Disinfected tertiary recycled water" means a filtered and subsequently disinfected wastewater that meets certain total coliform concentration, turbidity, and disinfection requirements. A lower degree of treatment, "disinfected secondary recycled water", is allowed for specified irrigation, non-irrigation and environmental uses, and is less frequently used. In some cases, a higher degree of treatment beyond Title 22 requirements is performed to meet more stringent requirements for salt and nutrient-sensitive uses.

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[8] Applicable excerpts from Title 22, Title 17, and the Health and Safety Code are documented in "The Purple Book", which provides a single source of guidelines and requirements for recycled water use in California (CDPH 2001).

[9] The Drinking Water Program for CDPH moved to the SWRCB and was renamed the Division of Drinking Water (DDW) as of July 1, 2014.

Figure B.1 Non-Potable Recycled Water Uses Allowed[1] in California

This summary is prepared by WateRuse Association of California, from the December 2, 2000, Title 22 adopted Water Recycling Criteria, and supersedes all earlier versions.

Recycled Water Use	Treatment Level: Disinfected Tertiary Recycled Water	Disinfected Secondary 2.2 Recycled Water	Disinfected Secondary 23 Recycled Water	Undisinfected Secondary Recycled Water
Irrigation for:				
Food crops where recycled water contacts the edible portion of the crop, including all root crops	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Parks and playgrounds	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
School grounds	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Residential landscaping	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Unrestricted-access golf courses	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Any other irrigation uses not specifically prohibited by other provisions of the *California Code of Regulations*	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Food crops, surface-irrigated, above-ground edible portion, not contacted by recycled water	ALLOWED	ALLOWED	NOT ALLOWED	NOT ALLOWED
Cemeteries	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Freeway landscaping	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Restricted-access golf courses	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Ornamental nursery stock and sod farms with unrestricted public access	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Pasture for milk animals for human consumption	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Nonedible vegetation with access control to prevent use as a park, playground or school grounds	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Orchards with no contact between edible portion and recycled water	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Vineyards with no contact between edible portion and recycled water	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Non food-bearing trees, including Christmas trees not irrigated less than 14 days before harvest	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Fodder and fiber crops and pasture for animals not producing milk for human consumption	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Seed crops not eaten by humans	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Food crops undergoing commercial pathogen-destroying processing before consumption by humans	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Ornamental nursery stock, sod farms not irrigated less than 14 days before harvest	ALLOWED	ALLOWED	ALLOWED	ALLOWED
Supply for impoundment:				
Nonrestricted recreational impoundments, with supplemental monitoring for pathogenic organisms	ALLOWED[2]	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Restricted recreational impoundments and publicly accessible fish hatcheries	ALLOWED	ALLOWED	NOT ALLOWED	NOT ALLOWED
Landscape impoundments without decorative fountains	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED
Supply for cooling or air conditioning:				
Industrial or commercial cooling or air conditioning involving cooling tower, evaporative condenser, or spraying that creates a mist	ALLOWED[3]	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Industrial or commercial cooling or air conditioning not involving cooling tower, evaporative condenser, or spraying that creates a mist	ALLOWED	ALLOWED	ALLOWED	NOT ALLOWED

Recycled Water Use	Treatment Level: Disinfected Tertiary Recycled Water	Disinfected Secondary 2.2 Recycled Water	Disinfected Secondary 23 Recycled Water	Undisinfected Secondary Recycled Water
Other Uses:				
Groundwater Recharge	ALLOWED under special case-by-case permits by RWQCB[4]			
Flushing toilets and urinals Priming drain traps Industrial process water that may contact workers Structural fire fighting Decorative fountains Commercial laundries Consolidation of backfill material around potable water pipelines Artificial snow making for commercial outdoor use Commercial car washes, not heating the water, excluding the general public from the washing process	ALLOWED	NOT ALLOWED	NOT ALLOWED	NOT ALLOWED
Industrial process water that will not come into contact with workers Industrial boiler feed Nonstructural fire fighting Backfill consolidation around nonpotable piping Soil compaction Mixing concrete Dust control on roads and streets Cleaning roads, sidewalks and outdoor work areas		ALLOWED	ALLOWED	
Flushing sanitary sewers				ALLOWED

1 Refer to the full text of the version of California Department of Public Health's "Regulations Related to Recycled Water", published on January 1, 2009. This chart is only an informal summary of uses allowed in that publication. The most current Title 17 and Title 22 regulations can be downloaded from: http://www.waterboards.ca.gov/drinking_water/certlic/drinkingwater/documents/lawbook/RWregulations_20150625.pdf

2 With "conventional tertiary treatment." Additional monitoring for two years or more is necessary with direct filtration.

3 Drift eliminators and/or biocides are required if public or employees can be exposed to mist.

4 Refer to the June 18, 2014 final Groundwater Recharge Guidelines, available from the DDW website at: http://www.cdph.ca.gov/services/DPOPP/regs/Pages/DPH14-003EGroundwaterReplenishmentUsingRecycledWater.aspx

In addition to recycled water uses and treatment requirements, Title 22 addresses sampling and analysis requirements at the treatment plant, preparation of an engineering report prior to production or use of recycled water, general treatment design requirements, reliability requirements, and alternative methods of treatment.

Title 17 State Drinking Water Code

The focus of Title 17 is protection of drinking (potable) water supplies through control of cross-connections[10] with potential contaminants, including non-potable water supplies such as recycled water. Title 17, Group 4, Article 2 - Protection of Water System, Table 1, specifies the minimum backflow protection required on the potable water system for situations in which there is potential for contamination to the potable water supply. Recycled water is addressed in Title 17 as follows:

- An **air-gap separation** is required on "Premises where the public water system is used to supplement the recycled water supply."
- A **reduced pressure principle backflow prevention device** is required on "Premises where recycled water is used…and there is no interconnection with the potable water system."
- A **double-check valve assembly** may be used for "Residences using recycled water for landscape irrigation as part of an approved dual plumbed use area established pursuant to Sections 60313 through 60316 unless the recycled water supplier obtains approval for the local public water supplier, or (DDW) if the water supplier is also the supplier of the recycled water, to utilize an alternative backflow prevention plan that includes an annual inspection and annual shutdown test of the recycled water and potable water systems pursuant to subsection 60316(a)."

Title 17 specifies the minimum backflow protection on the potable water system for situations in which there is potential for contamination to the potable water supply. In conjunction with local health agencies, DDW reviews and approves final onsite (customer) system plans for cross-connection control in accordance with Title 17, and inspects each system prior to operation. Backflow prevention and cross-connection testing would be performed for each site in accordance with DDW requirements before the recycled water supply is connected to that site.

B.3. State Guidelines

To assist in compliance with Title 22, DDW has prepared a number of guidelines for production, distribution, and use of recycled water. Additionally, DDW recommends use of guidelines prepared by the California-Nevada Section of the American Water Works Association (AWWA). These guidelines are summarized below.

Guideline for the Preparation of an Engineering Report on the Production, Distribution, and Use of Recycled Water. According to Title 22, prior to implementation of a water reclamation

[10] A cross-connection is an unprotected actual or potential connection between a potable water system used to supply water for drinking purposes and any source or system containing unapproved water or a substance that is not or cannot be approved as safe, wholesome, and potable, which in this case will be recycled water. By-pass arrangements, jumper connections, removable sections, swivel or changeover devices, or other devices through which backflow could occur, shall be considered to be cross-connections

project (production, distribution, or use) an engineering report must be prepared and submitted to DDW. This guideline, prepared by DDW and dated March 2001, specifies the contents of an engineering report. The report should describe the production process, including the treated (effluent) water quality, the raw water quality, the treatment process; the plant reliability features the supplemental water supply, the monitoring program, and a contingency plan to prevent distribution of inadequately treated water. The report should include maps of the distribution system and describe how the system will comply with DDW and AWWA guidelines and Title 17. The report should include maps of proposed use areas and should describe the use areas, the types of uses proposed, the people responsible for supervising the uses, the design of the user systems, and the proposed user inspection and monitoring programs.

Manual of Cross Connection Control/Procedures and Practices. This manual, dated July 1981, focuses on establishing a cross-connection control program to protect the public against backflow and back-siphonage of contamination. Main elements of the manual include areas where protection is required; causes of backflow; approved backflow preventers; procedures, installation, and certification of backflow preventers; and water shutoff procedures (for conditions which pose a hazard to the potable water supply).

Guidelines for the Distribution of Non-potable Water. These guidelines were prepared by the California-Nevada Section of AWWA in 1992. The purpose of these guidelines is to provide guidance for planning, designing, constructing, and operating non-potable water systems, including recycled water systems. Distribution lines, storage and supply, pumping, on-site (user) applications, and system management are discussed. DDW guidelines reference these guidelines.

Guidelines for the On-Site Retrofit of Facilities Using Disinfected Tertiary Recycled Water. The California-Nevada Section of AWWA prepared these guidelines in 1997 to provide guidance on modifying existing on-site facilities for conversion to use of recycled water, including recommendations for signage, backflow prevention, and separation standards, for landscape irrigation, agricultural irrigation, industrial uses, and impoundments.

B.4. State Recycled Water Policy

The SWRCB adopted a Recycled Water Policy (RW Policy) in 2009 to establish more uniform requirements for water recycling throughout the State and to streamline the permit application process in most instances. The RW Policy includes a mandate that the State increase the use of recycled water over 2002 levels by at least 200,000 AFY by 2030. Also included are goals for stormwater reuse, conservation, and potable water offsets by recycled water. The onus for achieving these mandates and goals is placed both on recycled water purveyors and potential users. Absent unusual circumstances, the RW Policy puts forth that recycled water irrigation projects that meet DDW requirements and other State or Local regulations be adopted by RWQCBs within 120 days. These streamlined projects will not be required to include a monitoring component.

The RW Policy requires that salt/nutrient management plans be developed for every basin in California and adopted as Basin Plan Amendments by 2015. These Management Plans are to be developed by local stakeholders and funded by the regulated community.

The RW Policy also required the formation of a Blue-Ribbon Advisory Panel (Panel) to guide future actions with respect to contaminants of emerging concern (CECs). CECs include chemicals and other substances that have no regulatory standard, have recently been "discovered" in natural streams, and potentially cause deleterious effects in aquatic life at environmentally relevant concentrations. The Panel was convened in May 2009 and completed in May 2010. A final report was issued in June 2010. The recommendations of the Panel resulted in the finalization of the Groundwater Recharge and Reuse Regulations in June 2014, which incorporated the Panel's recommendations.

B.5. Indirect Potable Reuse Regulations

The California Water Code addresses the use of recycled water for IPR via groundwater recharge and reservoir augmentation.

Groundwater Recharge Reuse Regulations

Regulations for groundwater replenishment using recycled water became effective on June 18, 2014. These regulations define full advanced treatment (FAT) as the treatment of an oxidized wastewater (wastewater in which the organic matter has been stabilized) using a RO and oxidation treatment process meeting certain minimum criteria. FAT (also referred as Advanced Water Purification (AWP)) is required in the case of groundwater replenishment via injection (subsurface application), but not necessarily for surface spreading. Key aspects of these regulations are summarized Appendix C: Potable Reuse Evaluation.

Reservoir Augmentation Regulations

A recycled water reservoir augmentation project is defined as a project that plans to use recycled municipal wastewater for the purpose of augmenting a reservoir that is designated as a source of domestic water supply. A significant degree of regulatory uncertainty exists with respect to the overall implementation of a reservoir augmentation project. Chief among these uncertainties is the fact that (1) DDW regulations for such a project have not yet been developed, and (2) DDW has not yet convened the required expert panel to assess reservoir augmentation public safety needs. Appendix C discusses probable DDW reservoir augmentation requirements.

B.6. Direct Potable Reuse Regulations

The California Water Code was modified by legislative statute to require DDW, in consultation with the SWRCB, to investigate and report on the feasibility of developing uniform water recycling criteria for DPR by December 31, 2016. Preliminary DPR regulations may not be available in California until 2020. In addition to FAT or AWP of the recycled water, an "engineered buffer" (storage tank) would need to be provided for a DPR project to ensure that water quality leaving the facility always met regulatory standards. Future DPR regulations, compared to IPR, are anticipated to include additional monitoring and/or treatment requirements to ensure the overall reliability of the treatment scheme, with a focus on acute risks (i.e., pathogens), critical control points, and continuous verification of treatment performance (NWRI 2014). The two major alternatives for the safe design of DPR are 1) focus on the engineered storage buffer that provides time for sample analysis, such as real-time pathogen log reduction monitoring, to ensure water meets quality requirements before distribution, or 2) emphasis on increased advanced treatment to meet the same goals (i.e., treatment redundancy). The required treatment technologies may be similar to the IPR regulations, i.e., RO and AOP. Appendix C provides additional information on potential DPR regulations.

Appendix B References

DDW. 2001. California Health Laws Related to Recycled Water "The Purple Book" Excerpts from the Health and Safety Code, Water Code, and Titles 22 and 17 of the California Code of Regulations. California Department of Public Health [Available at: http://www.cdph.ca.gov/certlic/drinkingwater/Documents/Recharge/Purplebookupdate6-01.PDF, accessed June 30, 2014].

DDW. 2014. California Department of Public Health Regulations Related to Recycled Water – June 18, 2014 (Revisions effective on 6/18/14) [Available at: http://www.waterboards.ca.gov/drinking_water/certlic/drinkingwater/documents/lawbook/RWregulations_20140618.pdf, accessed October 9, 2014].

Trussell RR, Salveson A, Snyder SA, Trussell RS, Gerrity D, Pecson BM. 2013. Potable Reuse: State of the Science Report and Equivalency Criteria for Treatment Trains. WateReuse Research Foundation, Alexandria, VA.

USEPA. 2012. Guidelines for Water Reuse. EPA/600/R-12/618. United States Environmental Protection Agency and National Risk Management Research Laboratory. http://nepis.epa.gov/Adobe/PDF/P100FS7K.pdf

WateReuse Association. 2009. Manual of Practice – How to Develop a Water Reuse Program. Principal authors: Thomas Holliman, Richard Atwater, Dr. James Crook and Lois Humphreys.

Appendix C: Potable Reuse Technical Assessment

The following study "Potable Reuse Technical Assessment" (Trussell Technologies, 2016) supports the evaluation of:

(1) groundwater replenishment (surface spreading and direct injection),
(2) surface water augmentation (at Castaic Lake), and
(3) direct potable reuse.

CASTAIC LAKE WATER AGENCY

Potable Reuse Alternative Technical Assessment

April 2016

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LIST OF ABBREVIATIONS

Description	Abbreviation
Advanced Oxidation Process	AOP
Advanced Water Treatment Facility	AWTF
Castaic Lake Water Agency	CLWA
Contaminants of Emerging Concern	CECs
Direct Potable Reuse	DPR
Division of Drinking Water	DDW
Enhanced Brine Concentration	EBC
Full Advanced Treatment	FAT
Groundwater Replenishment Reuse Regulations	GRR
Indirect Potable Reuse	IPR
Inland Empire Utilities Agency	IEUA
Ion Exchange	IX
Los Angeles County Department of Public Works	LACDPW
Los Angeles Regional Water Quality Control Board	LARWQCB
Maximum Contaminant Levels	MCLs
Membrane Filtration	MF
Modified Ludzack Ettinger	MLE
Nanofiltration	NF
National Pollutant Discharge Elimination System	NPDES
N-nitrosodimethylamine	NDMA
Notification Levels	NLs
Recycled Water	RW
Recycled Water Contribution	RWC
Reverse Osmosis	RO
Salt and Nutrient Management Plan	SNMP
Sanitation Districts of Los Angeles County	LACSD
Santa Clara River	SCR
Santa Clarita Valley Sanitation District	SCVSD
Santa Clarita Water District	SCWD
Soil Aquifer Treatment	SAT
Soil Aquifer Treatment Factor	STF
State Water Project	SWP
Surface Water Augmentation	SWA
Surface Water Treatment Rule	SWTR
Total Dissolved Solids	TDS
Total Maximum Daily Load	TMDL
Total Organic Carbon	TOC
Ultraviolet	UV
Valencia Water Company	VWC
Waste Discharge Requirement	WDR
Water Reclamation Plant	WRP

1. POTABLE REUSE OVERVIEW

The continuing drought in California has depleted surface water supplies to communities across the state. As a result, groundwater use has increased to compensate for this deficiency. The Santa Clara River Valley (Valley) experiences fluctuations in its supply of imported State Water Project (SWP) water annually, which has led to pumping more water from the Alluvial Aquifer and Saugus Formation. To offset future declines in SWP availability and reduce pumping in the two aquifers, potable reuse projects need to be considered as source alternatives.

The goal of this section is to present three types of potable reuse projects – Groundwater Replenishment (surface spreading and direct injection), Surface Water Augmentation, and Direct Potable Reuse – and evaluate them based on water quality and regulatory requirements. This technical assessment will provide CLWA and the purveyors a guideline in deciding on the implementation of a reuse project, which has the potential of enhancing local water supplies for residents of the Valley.

1.1 Potential Source Waters for Potable Reuse

The Santa Clarita Valley Sanitation District (SCVSD) owns and operates two treatment plants in the Valley, namely the Valencia Water Reclamation Plant (Valencia WRP) and the Saugus Water Reclamation Plant (Saugus WRP). The treatment processes for the Valencia and Saugus WRPs are the same and are shown in Figure 1. Both plants undergo biological treatment through a Modified Ludzack Ettinger (MLE) nitrification/denitrification process. The biological treatment is followed by a secondary settling tank, to remove suspended particles. The wastewater is then subject to filtration through the use of dual-media pressure filters. The filtered effluent is then chlorinated for disinfection. Both plants meet the Title 22 recycled water (RW) criteria.

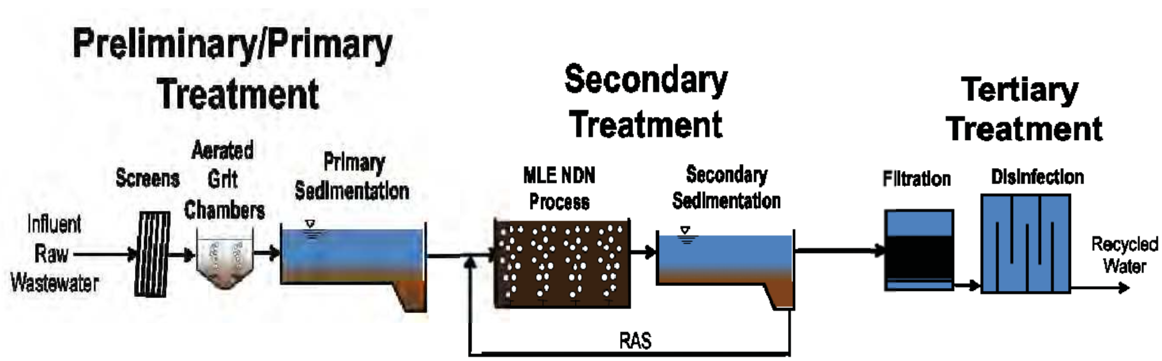

Figure 1 Treatment Process of the Valencia and Saugus WRP

1.1.1 Flow Availability

A flow analysis was performed using future RW flow data from Valencia WRP to determine the amount of water available for potable reuse. Table 1 summarizes the potential available supply of RW from the Valencia WRP in the year 2050. Due to additional conveyance costs, the Saugus WRP was not considered for this analysis. It was assumed that the available supply of RW must first fulfill the demands associated with RW customers in the Valley and the continuing environmental discharge into the Santa Clara River (SCR), a commitment made by SCVSD with the Los Angeles Regional Water Quality Control Board (LARWQCB) and assumed to be 8.5 MGD from the Valencia WRP for planning purposes.

Table 1 Projected RW Production and Discharge in 2050

SCVSD Treatment Plant	Projected Recycled Water Production (MGD)[a]	Anticipated Discharge Requirement (MGD)
Valencia WRP	**18.7**	**8.5**

[a]Based on a 65 gpcd wastewater generation rate multiplied by the projected population

The flows from Table 1 will first account for the 8.5 MGD of RW released into the SCR. Currently, the RW being released into the river is violating the chloride Total Maximum Daily Load (TMDL) of 100 mg/L set by the LARWQCB. This prompted the implementation of the Chloride Compliance Project that will include an Advanced Water Treatment Facility (AWTF) at the Valencia WRP. In addition, the existing chlorine disinfection will be replaced with an ultraviolet light (UV) disinfection system. As a planning tool, SCVSD provided guidance that 4.5 MGD of AWTF product water could be used as part of any potable reuse project, if needed.

The flows will then be utilized to meet the irrigation demands of customers under the existing Phase 1 and planned Phase 2 of the RW Master Plan, as well as the planned Newhall Ranch and Westside Communities developments (herein referred to as Newhall Ranch). Phase 1, Phase 2, and Newhall Ranch require 0.40 MGD, 1.89 MGD, and 3.05 MGD, respectively, or a total of 5.3 MGD. The remainder of the RW can then be used for a potable reuse project and is 4.9 MGD, as shown in Table 2.

Table 2 RW Available in 2050 from Valencia WRP

Projected Combined Flows for Valencia WRP (MGD)	Required River Discharge (MGD)	RW Demand (MGD) [1]	RW Available for Potable Reuse (MGD)
18.7	8.5	5.3	**4.9**

[1] RW demand for Valencia WRP only Phases 1, 2a, 2c, 2d and that portion of planned Newhall Ranch development demands that are not met by the Newhall Ranch WRP.

1.1.2 Existing Water Quality

SCVSD provided water quality monitoring data from 2012-2014 for the tertiary effluent produced from the Valencia WRP. The water quality data is shown in Table 3, along with corresponding regulatory requirements.

Table 3 Water Quality Data from 2012 to 2014 for Final Effluent from Valencia WRP

Constituent	Units	Valencia WRP Effluent	Regulatory Requirement
pH	-	7.43	6.0 - 9.0[1]
Turbidity	NTU	0.50	2[1]
Total Coliform	org./100 mL	<1	2.2[1]
Temperature	°F	77.8	-
Total Suspended Solids	mg/L	<2.5	-
Settleable Solids	mL/L	<0.1	-
Total Dissolved Solids	mg/L	690	800[2]
Total BOD	mg/L	<0.6	-
Ammonia (as nitrogen)	mg/L	0.95	-

Organic Nitrogen	mg/L	1.07	-
Nitrate (as nitrogen)	mg/L	2.60	10[2]
Nitrite (as nitrogen)	mg/L	0.0029	1[3]
Total Nitrogen	mg/L	4.62	10[3]
Fluoride	mg/L	0.367	2[3]
Total Cyanide	mg/L	0.0013	0.15[3]
Chloride	mg/L	126	150[2]
Sulfate	mg/L	178	150[2]
Total Hardness	mg/L	259	-
Antimony	mg/L	4.70E-04	0.006[3]
Arsenic	mg/L	1.25E-04	0.01[3]
Barium	mg/L	0.00995	1[3]
Beryllium	mg/L	<5.00E-04	0.004[3]
Boron	mg/L	0.53	1[4]
Cadmium	mg/L	<2.50E-04	0.005[3]
Chromium VI	mg/L	<4.80E-06	0.01[3]
Total Chromium	mg/L	<7.00E-05	0.05[3]
Copper	mg/L	0.003	1[5]
Iron	mg/L	0.072	0.3[5]
Lead	mg/L	<3.00E-05	0.05[1]
Mercury	mg/L	4.57E-07	0.002[3]
Nickel	mg/L	0.0027	0.1[3]
Selenium	mg/L	1.70E-04	0.01[1]
Silver	mg/L	<3.00E-05	0.05[1]
Thallium	mg/L	<2.00E-05	0.002[3]
Zinc	mg/L	0.033	5[5]
Oil and Grease	mg/L	<0.8	-
Radioactivity (gross alpha + gross beta)	pCi/L	14.9	65[1]
Strontium-90	pCi/L	0.30	-
Diazinon	mg/L	2.54E-04	0.0012[4]
1,4-Dioxane	mg/L	8.60E-04	0.001[4]
Naphthalene	mg/L	<1.80E-04	0.017[4]
N-Nitrosodimethylamine (NDMA)	mg/L	1.21E-04	1.00E-05[4]
N-Nitrosodi-n-propylamine	mg/L	<1.20E-04	1.00E-05[4]
1,2,3,-Trichloropropane	mg/L	<1.20E-06	5.00E-06[4]
Perchlorate	mg/L	9.43E-04	0.006[3]
Total trihalomethanes (TTHM)	mg/L	0.050	**0.08[6]**
Bromodichloromethane	mg/L	0.020	
Bromoform	mg/L	0.0027	
Chloroform	mg/L	0.016	
Dibromochloromethane	mg/L	0.012	
Methyl-tert-butyl ether (MTBE)	mg/L	<1.60E-04	0.005[5]
Benzo(a)pyrene	mg/L	<7.00E-06	0.0002[7]

Chlordane	mg/L	<3.00E-05	0.0001[7]
2,4-D	mg/L	NM	0.07[7]
Endrin	mg/L	<2.00E-06	0.002[7]
Heptachlor	mg/L	<1.00E-06	0.00001[7]
Heptachlor Epoxide	mg/L	<1.00E-06	0.00001[7]
Hexachlorobenzene	mg/L	<1.80E-04	0.001[7]
Hexachlorocyclopentadiene	mg/L	<7.50E-04	0.05[7]
Lindane	mg/L	<1.00E-06	0.0002[7]
Methoxychlor	mg/L	NM	0.03[7]
Pentachlorophenol	mg/L	<3.80E-04	0.001[7]
2,3,7,8-TCDD (Dioxin)	mg/L	<4.80E-10	3.00E-08[7]
2,4,5-TP (Silvex)	mg/L	NM	0.05[7]
Benzene	mg/L	<1.00E-04	0.001[8]
Carbon Tetrachloride	mg/L	<7.00E-05	0.0005[8]
1,2-Dichlorobenzene	mg/L	<1.20E-04	0.6[8]
1,4-Dichlorobenzene	mg/L	<7.00E-05	0.005[8]
1,1-Dichloroethane	mg/L	<7.00E-05	0.005[8]
1,2-Dichloroethane	mg/L	<9.00E-05	0.0005[8]
1,2-Dichloropropane	mg/L	<9.00E-05	0.005[8]
1,3-Dichloropropene	mg/L	*<5.00E-04*[9]	0.0005[8]
Ethylbenzene	mg/L	<6.00E-05	0.3[8]
1,1,2,2-Tetrachloroethane	mg/L	<1.00E-04	0.001[8]
Toluene	mg/L	<6.00E-05	0.15[8]
1,2,4-Trichlorobenzene	mg/L	<1.70E-04	0.005[8]
1,1,1-Trichloroethane	mg/L	<7.00E-05	0.2[8]
1,1,2-Trichloroethane	mg/L	<1.00E-04	0.005[8]
Foaming Agents (MBAS)	mg/L	<0.03	0.5[5]
Toxaphene	mg/L	<4.00E-05	0.003[7]
Vinyl Chloride	mg/L	<1.20E-04	0.0005[8]

[1] RW as specified in RWQCB-LA Order No. 89-129 (Valencia WRP). Trace constituent concentration limits obtained from California Department of Health Services, California Administrative Code, Title 22, Division 4, Chapter 15, "Domestic Water Quality and Monitoring" (1989).
[2] Groundwater quality objectives (GWQO) as stated in the Salt and Nutrient Management Plan (SNMP) of the Santa Clara River Valley East Subbasin.
[3] Table 64431-A (Inorganic Chemicals) of the Title 22 California Code of Regulations.
[4] California notification limits (NLs) set by the Department of Drinking Water (DDW).
[5] Table 64449-A of the Title 22 California Code of Regulations.
[6] Table 64533-A (Disinfection Byproducts) of the Title 22 California Code of Regulations.
[7] Table 64444-A(b) (Non-Volatile Organic Chemicals) of the Title 22 California Code of Regulations.
[8] Table 64444-A(a) (Volatile Organic Chemicals) of the Title 22 California Code of Regulations.
[9] No method of detection limit (MDL) provided in WQ data, so used the reporting detection limit (RDL) to specify the non-detected concentration range.
mg/L: milligrams per liter
NTU: Nephelometric Turbidity Units
pCi/L: Picocuries per liter

For all potable reuse alternatives, the RW must comply with existing Title 22 drinking water maximum contaminant levels (MCLs). The reclaimed water must meet primary and secondary MCLs for drinking water as defined in the Title 22 California Code of Regulations Tables 64444-A(a), 64444-A(b), 64449-A, 64449-B, 64533-A, and 64431-A.

For certain chemicals with no MCLs, the Division of Drinking Water (DDW) has established health-based advisory levels known as notification levels (NLs). Among this list of chemicals, there are two contaminants of emerging concern (CECs) that are of interest: N-nitrosodimethylamine (NDMA) and 1,4-Dioxane. While the levels of 1,4-Dioxane are within the acceptable range, the levels of NDMA for both plants are above the 0.000010 mg/L (10 ng/L) NL.

A Salt and Nutrient Management Plan (SNMP) was prepared for the Santa Clara River Valley East Subbasin, with the guidance of the LARWQCB, to establish water quality objectives that will help sustain and protect the local water supply. The RW will need to satisfy the SNMP water quality requirements for total dissolved solids (TDS), chloride, nitrate, and sulfate. From the data presented in Table 3, it is evident that all the groundwater quality objectives of the SNMP are met, with the exception of sulfate.

2. GROUNDWATER REPLENISHMENT ALTERNATIVES

2.1 Alternatives Overview

The following groundwater replenishment alternatives are utilized to augment groundwater supplies with RW: (1) surface spreading and (2) direct injection.

In surface spreading, reclaimed water is discharged into spreading basins, where it vertically percolates through the vadose (unsaturated) zone until it joins native groundwater and travels horizontally (saturated zone). The water naturally filters through the vadose and saturated zones achieving additional purification. This geopurification system is known as soil aquifer treatment (SAT). Per the Groundwater Replenishment Reuse Regulations (GRR), the wastewater needs to be treated to meet the criteria for Title-22 RW unrestricted use (eg. tertiary, disinfected with Total Coliform of <2.2 Most Probable Number /100 milliliters (mL)). A schematic of a common surface spreading project is shown in Figure 2.

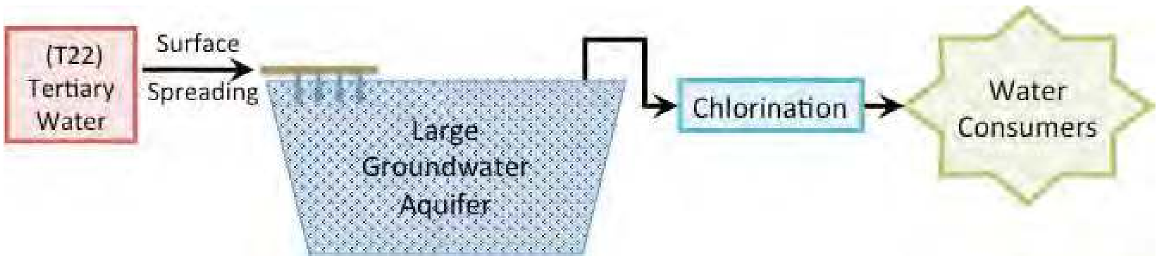

Figure 2 Schematic of a Typical Surface Spreading Project

In direct injection, RW that has gone through a full advanced treatment (FAT) process is directly injected into the saturated groundwater zone. While the implementation of FAT (i.e. membrane filtration (MF), reverse osmosis (RO), and an advanced oxidation process (AOP)) allows for the use of up to 100% RW (eg. no dilution requirement), the cost associated with the capital infrastructure, maintenance and operation of the technology, as well as the brine disposal, is significant. A schematic of a common direct injection project is shown in Figure 3.

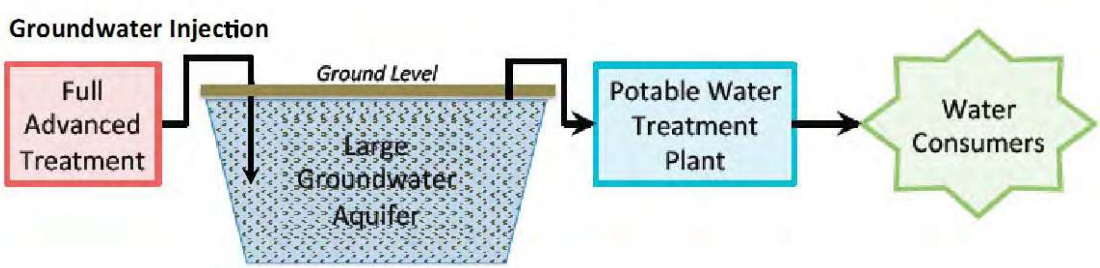

Figure 3 Schematic of a Typical Direct Injection Project

Both of these groundwater replenishment alternatives are governed by the GRR.

2.2 Groundwater Replenishment Reuse Regulations

The GRR of California's DDW, which were promulgated on June 18, 2014, govern surface spreading and direct injection recharge projects. The GRR define specific treatment requirements that both methods must meet:

- Title 22 Criteria
- Pathogenic Microorganism Control
- Total Organic Carbon (TOC) Requirement
- Total Nitrogen Requirement
- All Regulated Contaminant Limits

While most of the requirements are similar across both groundwater replenishment alternatives, there are some key differences. These will be discussed in further detail in the following sections.

2.3 SURFACE SPREADING

2.3.1 Treatment Requirements

For the surface spreading alternative, the GRR requires that the water meet Title 22 RW unrestricted use standards: the wastewater is subject to oxidation (biological treatment), filtration (dual-media pressure filters), and (chlorine) disinfection. As described previously, the Valencia WRP already has this level of treatment and no further treatment is explicitly required in the GRR.

2.3.2 Proposed Treatment Train

No additional treatment train is proposed for the surface spreading project alternatives. However, the inclusion of an ozonation step could provide significant destruction of CECs and help allay public perception concerns regarding trace pollutants. It would also improve the removal of organic matter through the SAT process, allowing more water to be spread as discussed further in Section 2.3.9 Total Organic Carbon and Ultimate Utilization.

2.3.3 RW Quality

According to the GRR, the total nitrogen concentration in RW must be less than 10 mg/L. Figure 4 shows the total nitrogen data from 2012-2014 for the Valencia WRP. The tertiary effluent from the Valencia WRP meets the total nitrogen requirement in the GRR.

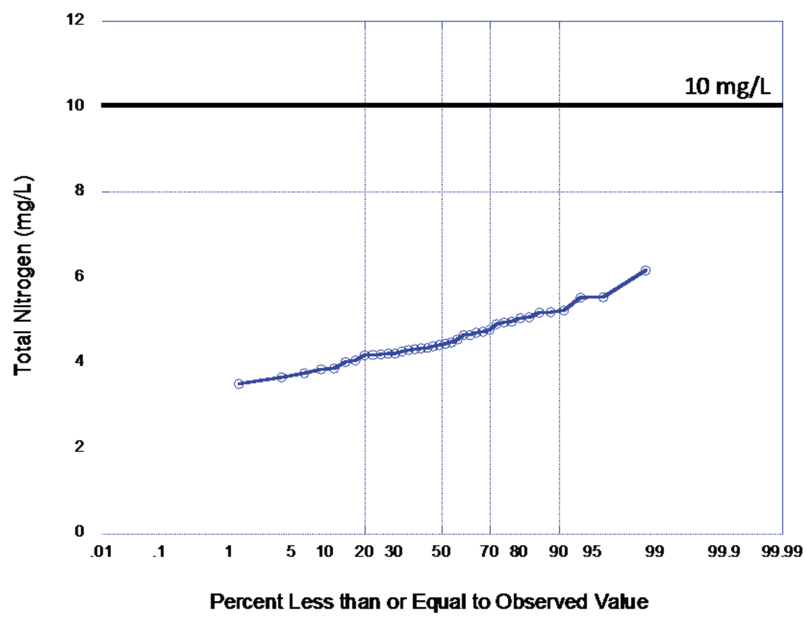

Figure 4 Total Nitrogen from 2012 to 2014 for Final Effluent from Valencia WRP

As previously discussed, the sulfate concentration in the effluent of the Valencia WRP is above the SNMP water quality objective in the Santa Clara River Valley East Subbasin. Any surface spreading project using the Valencia WRP effluent would require mitigation by blending with the planned Valencia AWTF or by providing additional treatment. Blending with the AWTF water was only considered in this analysis and a 70/30 blend of tertiary effluent to RO permeate was assumed based on input from SCVSD.

NDMA concentrations are above the NL established by the DDW and will require discussion on how SAT will aid the removal for a spreading project. The Montebello Forebay has been operating since the 1960's by spreading water that undergoes a similar level of treatment and contains NDMA levels above the NL. Research projects focused on NDMA at the Montebello Forebay, as well as other research, have indicated that NDMA is well removed by SAT (Trussell 2014, Drewes, 2006, Nalinakumari, 2010).

The RW from the Valencia WRP has no other constituents that are above their respective regulatory limits. One possible challenge could be the chloride TMDL in the SCR. While the RW would be spread and percolated into the ground (where the chloride limit is 150 mg/L), given the strict chloride limit in the SCR of 100 mg/L, special attention to prevent upwelling of the groundwater into the river will need to be addressed.

2.3.4 RW Availability

As discussed previously, any IPR scenario first must meet the minimum river discharge of 8.5 MGD and the RW demand of 5.3 MGD (see Table 2). After these demands, the Valencia WRP has 4.9 MGD of available RW.

2.3.5 Potential Recharge Locations

The document “Castaic Lake Water Agency – Water Resources Reconnaissance Study” (Recon Study) provided CLWA and the local water purveyors with water supply augmentation strategies to deal with future dry years and the resulting decrease in SWP water availability. In the groundwater replenishment analysis of the study, three recharge locations (shown in Figure 5) were considered as potential spreading basins based on the six-month retention time requirement used in the GRR to achieve 10-log removal of *Cryptosporidium* and *Giardia*. This will be discussed further in Section 2.3.6 Retention Time and Microorganism Control.

Figure 5 Potential Recharge Location (blue triangles) in Recon Study

In the Recon Study, Recharge Location #2 was eliminated as an option due to its proximity to existing drinking water wells, which would result in retention times below 6-months. For this analysis, the location of Recharge Location #1 was moved out of the river to the riverbank for further analysis. Having an in-river basin presents the challenge of managing the spreading facility operation during storm events to prevent discharge into the river itself. Moving the recharge location to the riverbank considerably simplifies this operation.

Consistent with the Recon Study, an infiltration rate of 3 feet per day was used for all spreading basins. The infiltration rate for any given spreading basin is site specific and can range from 0.5 feet per day to greater than 5 feet per day. An infiltration rate of 3 feet per day is consistent when compared with several active spreading basins owned and operated by the Los Angeles County Department of Public Works, Flood Control District (LACDPW) with similarly sandy soils (Table 4).

Table 4 Reference Infiltration Rates in Existing Spreading Basins

Existing Spreading Basin	Infiltration Rate (ft/d)	Reference
Montebello Forebay	2-3	Laws, et.al., 2010
Santa Fe Spreading Grounds[1]	4.7	via LACDPW website www.ladpw.org/wrd/spreadingground/information/facdept.cfm%3Ffacinit=1
Hansen Spreading Grounds[1]	2.5	via LACDPW website www.ladpw.org/wrd/spreadingground/information/facdept.cfm?facinit=20

[1] Currently only storm water is spread at these facilities, but no change in infiltration rate is anticipated with the implementation of RW for spreading.

2.3.5.1 Recharge Location #1

According to the LACDPW there are currently 53 acres of city-owned parcels available near Recharge Location #1 for use as a potential recharge basin (*SCR Watershed Study*, 2007). For this study, the 21 acres identified in Figure 6 were considered as Recharge Location #1. The 1 acre-basin is envisioned to be used as a settling basin for stormwater flows, which would be diverted to the pond via an inflatable dam across the SCR. A pipeline would connect the 1-acre area and the 20-acre area to maximize reuse. Additional study is required to optimize the location of the inflatable dam and to design the hydraulics and control to maximize recycled water and stormwater recharge. This project would likely require a partnership with LACDPW to operate the in-river and stormwater components of the system.

Figure 6 Proposed Location and Size of Recharge Location #1

2.3.5.2 Recharge Location #2

Recharge Location #2, as identified and discussed in the Recon Study, was eliminated from consideration due to insufficient travel time. No further analysis on this location was considered as part of the RW Master Planning effort.

2.3.5.3 Recharge Location #3

Recharge Location #3 was also considered as described in the Recon Study and the LACDPW's SCR Watershed Study (*SCR Watershed Study*, 2007). Recharge Location #3 is located in-river and would include a recharge area of approximately 28 acres, as shown in Figure 7.

Figure 7 Proposed Location and Size of Recharge Location #3

2.3.5.4 RW Spreading Restrictions

While the potential amount of RW available annually for spreading was developed and shown in Table 2, the actual RW contribution may be limited by seasonal water availability and the capacity of the respective recharge location. Stormwater capture was prioritized and it was assumed that during heavier months of rainfall, spreading RW would be limited. As shown in Table 5, if the average monthly rainfall (2007-2015) was greater than 2-inches, then a 50% usage was assumed. If the average monthly rainfall was greater than 1-inch, then a 75% usage was assumed. If the rainfall was less than 1-inch, the spreading basin was assumed to have full availability. These assumptions are based on an analysis of rain data and storm events. These assumptions are conservative and it's possible that the recharge locations will be available for a higher percentage during winter months.

Table 5 Average Monthly Rainfall (2007-2015) and Assumed Spreading Basin Availability

Month	Average Precipitation (inches/month)	Recharge Availability (%)	Recharge Availability (days)
Jan	2.67	50%	16
Feb	2.40	50%	14
Mar	2.38	50%	16
Apr	1.18	75%	23
May	0.36	100%	31
Jun	0.03	100%	30
Jul	0.02	100%	31
Aug	0.11	100%	31
Sept	0.27	100%	30
Oct	0.27	100%	31
Nov	1.68	75%	23
Dec	1.78	75%	23

Monthly spreading flows of RW were determined for each recharge location based on the spreading area and the limitations caused by precipitation. The maximum RW spread was determined and is the same for both Recharge Location #1 and #3 and is summarized in Table 6 on an annual basis and Figure 8 on a monthly basis. Due to availability restrictions for basins during stormflow and peak summer irrigation demands on the RW supply, not all of the available RW can be spread.

Table 6 Annual RW Contributions for Recharge Locations #1 and #3

Recharge Location	RW Available (MGD)	RW Spread (MGD)
#1	4.9	3.3
#3	4.9	3.3

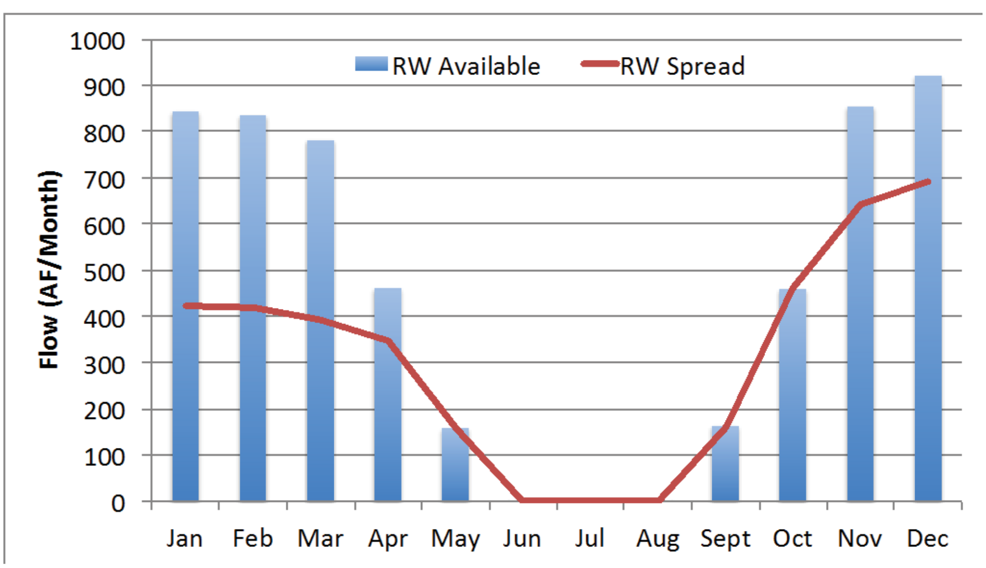

Figure 8 Monthly Comparison of Available RW and RW Spread for Recharge Location #1 and #3

2.3.6 Retention Time and Microorganism Control

The RW discharged will need to satisfy the GRR for pathogen control. Table 7 illustrates the required removal criteria for enteric virus, *Cryptosporidium*, and *Giardia* (V/G/C). For each pathogen, a separate treatment process can only be credited up to a 6-log reduction and at least three processes must each achieve no less than 1-log reduction.

Table 7 GRR Pathogenic Microorganism Control

Pathogen	Removal Criteria
Enteric Virus	12-log
Giardia	10-log
Cryptosporidium	10-log

Removal credit can also be obtained through the amount of time the reclaimed water is maintained underground (e.g., retention time). For a surface spreading project, the following conditions apply:

- 1-log virus credit per month of retention time underground
- 10-log *Cryptosporidium* and *Giardia* credit for 6 months or greater retention time underground

To determine the retention times associated with Recharge Location #1 and Recharge Location #3, groundwater modeling was performed by GSI Water Solutions, Inc. (GSI). Calculated monthly discharge volumes were input into the model for varying groundwater conditions and retention times were calculated.

Figure 9 illustrates the results of the modeling effort for Recharge Location #1 and shows both the capture zones from nearby drinking water wells (indicated in thick yellow and white lines) and the flow path from the spreading basin (indicated with thin red lines). The results show that Valencia Water Company's (VWC) well VWC-U4 captures water in the range of 8-10 months. For planning stages, hydraulic modeling only receives half of the potential log credit. Therefore, a 10-month travel time would result in a 5/0/0 for V/G/C.

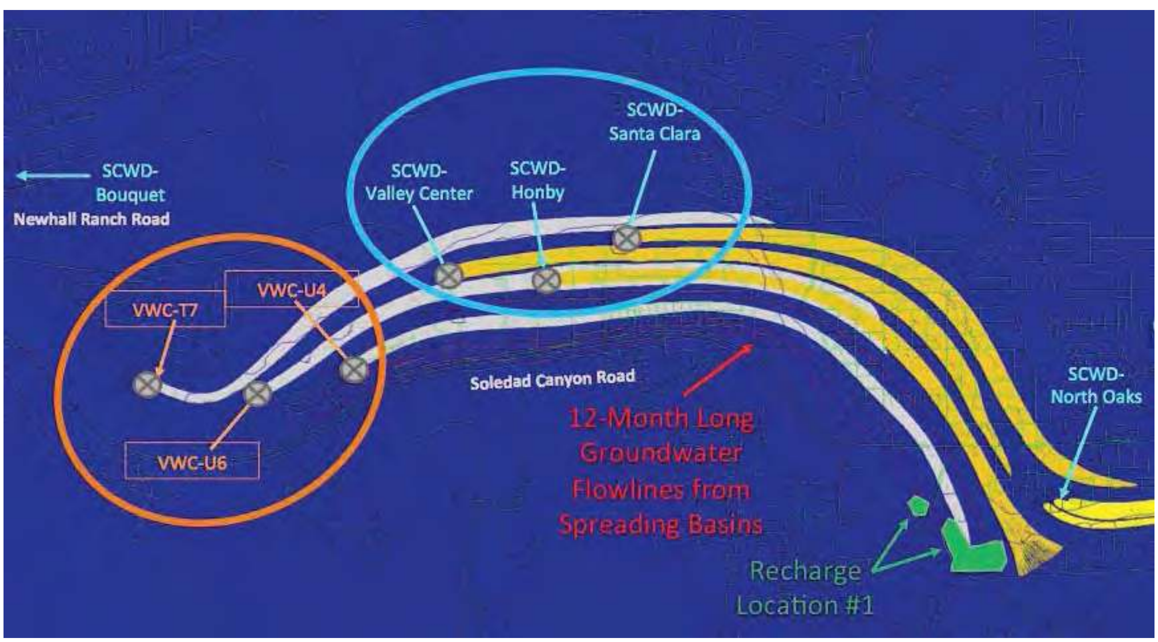

Figure 9 Groundwater Modeling for Recharge Location #1

To validate the retention time of the groundwater and thus increase the associated log credit, an added or intrinsic tracer test is required. An added tracer gets a 1-log reduction credit per month, while an intrinsic tracer gets 0.67-log credits per month. Implementation of an alternative using Recharge Location #1 would require one of two options: 1) Spread potable water spiked with a tracer to verify the travel time or 2) shut down well VWC-U4 for a time period on the order of 6-12 months while the tracer test is performed. If an intrinsic tracer is used, the travel time would need to be confirmed as 9 months or greater to receive 6/10/10 for V/G/C. If an added tracer is used, verification of greater than a 6-month travel time would translate to 6/10/10 for V/G/C.

The remaining 6-log virus credit can be achieved through conventional wastewater treatment processes that exist at the Valencia WRP; 1.9-logs from primary/secondary/tertiary treatment (*Rose et. al., 2004*) and 4-logs from chlorination or 5-logs from the future UV disinfection system.

Table 8 Anticipated Pathogenic Microorganism Control for Recharge Location #1

Pathogenic Microorganism	Goal	Primary, Secondary, Tertiary	Disinfection[1]	Subsurface Travel Time	Total
log virus	12	1.9	5	8	14.9
log Giardia	10	0.8	0	10	10.8
log Crypto	10	1.2	0	10	11.2

[1] Includes entire 5-log filtration disinfection requirement for Title 22 with UV

Additionally, Santa Clarita Water Division's (SCWD) SCWD-Honby well's capture zone is very close to the recharge location. This well would likely be monitored during the in situ tracer test and also has a travel time of near 8-10 months.

Other observations made by GSI include the possibility of groundwater upwelling into the river when the groundwater basin is relatively full and increased pumping by downstream production wells to prevent localized daylighting of groundwater at those wells. These issues will need to be considered and controlled when implementing a surface spreading project.

Figure 10 illustrates the results of the modeling effort for Recharge Location #3. The results show there is an 18-month travel time to the nearest drinking water well in the Pinetree Wellfield. A hydraulic modeling result receives 50% travel time credit, so a 9-month travel time will be credited, resulting in 9/10/10 for V/G/C. In combination with the above ground existing disinfection, this is sufficient to meet the required pathogenic microorganism control log removals as shown in Table 9.

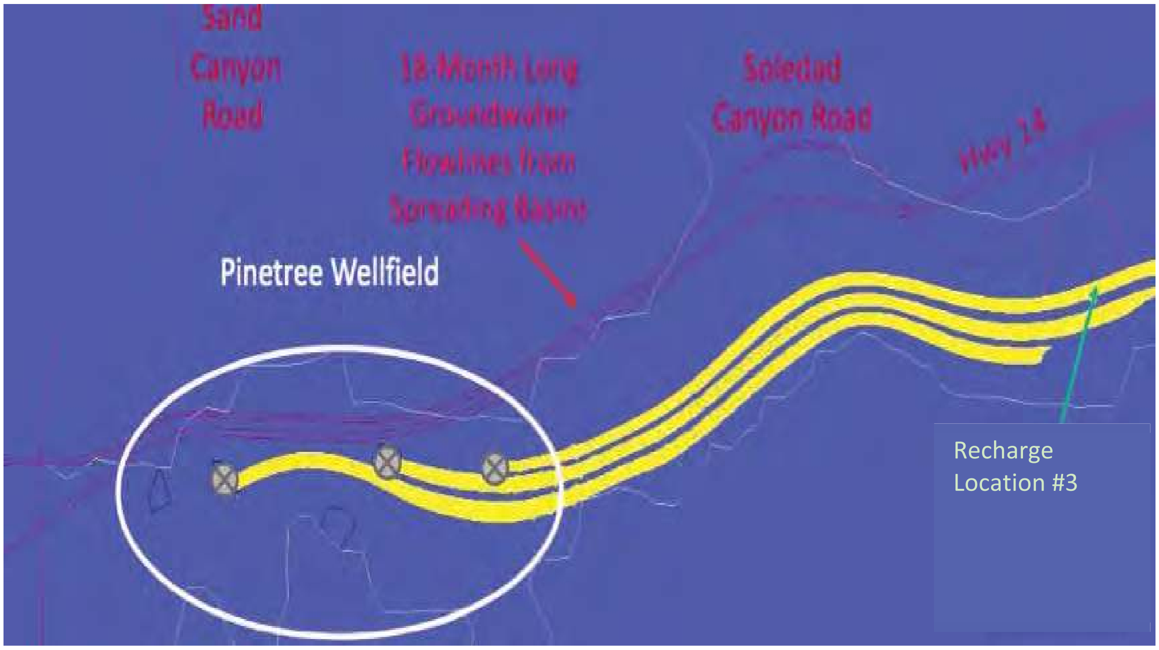

Figure 10 Groundwater Modeling for Recharge Location #3

Table 9 Anticipated Pathogenic Microorganism Control for Recharge Location #3

Pathogenic Microorganism	Goal	Primary, Secondary, Tertiary	Disinfection	Subsurface Travel Time	Total
log virus	12	1.9	5[1]	9	15.9
log Giardia	10	0.8	0	10	10.8
log Crypto	10	1.2	0	10	11.2

[1] Includes entire 5-log filtration disinfection requirement for Title 22 with UV

2.3.7 Diluent Volume

An important parameter in any surface spreading project is the municipal recycled wastewater contribution (RWC) and its closely related TOC requirement in the GRR. The RWC is defined as:

$$RWC = \frac{Recycled\ Water\ Applied}{Recycled\ Water\ Applied + Dilution\ Water} \quad (1)$$

The dilution water is the pre-existing surface or subsurface flow available to blend with the RW. Sources of surface water include rainfall, stormwater, and irrigation runoff, while the category of subsurface water is comprised solely of native groundwater. In the case where surface flow data is absent, such as in Recharge Location #1 and Recharge Location #3, groundwater underflow is relied upon as the dilution water. These values were modeled by GSI as part of the Recon Study and are based on Darcy's Law, which consists of the hydraulic conductivity, cross sectional area, and hydraulic gradient of the desired recharge basin.

In the Recon Study, two cross sectional areas were utilized to obtain the diluent flows; Method 1 used the width of the entire aquifer and Method 2 used the cross sectional area of the recharge basin. For this report, the diluent water calculated via Method 2 was used for both Recharge Location #1 and Recharge Location #3 and was 16.1 MGD and 4.5 MGD, respectively. A higher diluent volume is desirable, since it allows more RW to be spread. While the diluent water calculated in Method 1 was significantly higher and therefore more desirable, DDW may not consider all of the calculated diluent water to be available for mixing with the RW applied.

Table 10 Modeled Diluent Waters

Site	Method #1-Groundwater Basin cross-sectional area (MGD)	Method #2-Recharge Location cross-sectional area (MGD)
Recharge Location #1	51.8	16.1
Recharge Location #3	32.1	4.5

Another method (Method 3), used by the Inland Empire Utilities Agency (IEUA) to calculate the underflow of the Chino Basin aquifer and already approved by the DDW, could be applied to both recharge locations to obtain higher diluent volumes. This technique involves a 45 degree, outward extension from the cross sectional area of the recharge basin, which inherently results in a larger area. Through Darcy's equation, a larger diluent volume could be attained, resulting in a larger allowable RW application.

2.3.8 RW Contribution

Per the GRR, at the beginning of the project, the initial maximum RWC cannot exceed 20% unless specifically pre-approved. A 20% initial RWC would result in a RW application of 4.0 MGD and 1.1 MGD for Recharge Locations #1 and #3, respectively. The diluent volume limitation of Recharge Location #3 is noticeable in the low amount of reclaimed water that can be spread in the initial startup of the groundwater replenishment project.

Table 11 Initial RW Applied[1]

Site	Diluent Volume (MGD)	Initial RW Applied (MGD)
Recharge Location #1	16.1	4.0
Recharge Location #3	4.5	1.1

[1]Assumes 20% recycled water contribution at startup

For the initial RWC of 20%, a maximum TOC concentration of 2.5 mg/L must be achieved in the percolated water from a surface spreading project. This value was found with equation 2:

$$TOC_{max} = \frac{0.5\ mg/L}{RWC} = \frac{0.5\,{}^{mg}/_{L}}{20\%} = 2.5\,{}^{mg}/_{L} \qquad (2)$$

Once an IPR spreading project is underway and has shown itself to be protective of public health and the environment, the sponsor (CLWA) can petition DDW to increase the RWC.

2.3.9 TOC and Ultimate Utilization

TOC is not routinely reported at the Valencia WRP. However, as part of SCVSD's chloride compliance AWTF planning, TOC concentrations in the Valencia WRP effluent were monitored. For planning purposes, SCVSD provided an average TOC value of 4.7 mg/L for the Valencia WRP. This is above the 2.5 mg/L for an initial 20% RWC and as such two mitigation efforts will be utilized: 1) blending of tertiary wastewater with AWTF water to lower the TOC above ground and 2) receiving credit for the TOC removal that naturally occurs via SAT. Typically, an SAT factor (STF) of 60-70% has been observed through other applications and research (Trussell, 2014, Laws, 2011, Ly, 2011, Chino, 2014). Table 12 shows the breakdown of RW sources for surface spreading at Recharge Location #1 and #3.

Table 12 TOC at Recharge Locations #1 and #3

Recharge Location	Possible RW Contribution [1] (MGD)	RW Source	RW Flow from Source (MGD)	TOC (mg/L)	SAT Credited TOC[2] (mg/L)	Ultimate RWC (%)
#1	**3.3**	Valencia Blend	1.8	3.4	**1.20**	**17%**
		Valencia Tertiary	1.5	4.7		
		RW Applied Total	**3.3**	**4.0**		
#3	**3.3**	Valencia Blend	2.0	3.4	**1.18**	**43%**
		Valencia Tertiary	1.3	4.7		
		RW Applied Total	**3.3**	**4.0**		

[1]As developed in Table 6
[2]An assumed SAT factor of 70% was used for this analysis.

The resulting analysis from Table 12 shows that at both Recharge Location #1 and Recharge Location #3, the TOC will be below the required 2.5 mg/L to meet the initial RWC of 20%. The ultimate RWC for Recharge Location #1 is 17% and the ultimate RWC for Recharge Location #3 is 43%. Recharge Location #3 is initially limited by the amount of diluent water (Table 11), but ultimately, both locations are limited by the available RW. Neither location is limited by the TOC requirement.

Table 13 compares the volume spread for the two recharge locations, and shows how much RW could be applied at each location while still meeting the TOC and diluent volume requirements. It is clear from Table 13 that the limitation for both recharge locations is the amount of available RW. If more RW were available, these recharge locations could effectively spread up to the hypothetical ultimate RW shown in Table 13 based on the GRR's RWC and TOC requirements.

Table 13 Flow Comparison at Recharge Locations #1 and #3

Recharge Location	Available RW (MGD)	Initial RW (MGD)	Ultimate RW (MGD)	Hypothetical Ultimate RW[2] (MGD)
#1	3.3	3.3	3.3	10.7
#3	3.3	1.1[1]	3.3	4.2

[1]Based on an initial 20% RW Contribution
[2]RW that could be spread if more RW were available

2.3.10 Alternative Conveyance Concepts

Surface spreading at Recharge Locations #1 and #3 require conveyance to the proposed recharge location, the construction of the recharge basin, diversion facility and maintenance of the conveyance pipe and the recharge basin. The conveyance concept for Recharge Location #1 is shown in Figure 11. Surface spreading at Recharge Location #1 requires the extension of the proposed Phase 2A pipeline for approximately 3.5 miles, and the construction of the spreading basin and a diversion structure (eg. Recharge Location #1). A similar conveyance concept was developed for Recharge Location #3 by extending the pipeline as shown in Figure 12. Facility capital and operations costs for each alternative are presented in the Recycled Water Master Plan.

Figure 11 Conveyance Concept for Surface Spreading at Recharge Location #1

Figure 12 Conveyance Concept for Surface Spreading at Recharge Location #3

2.4 DIRECT INJECTION

The direct injection alternative is also regulated under the GRR and has very similar guidelines to the surface spreading alternative with some very important differences. Notable differences include:

- Full Advanced Treatment requirement
- 100% RWC contribution upon commencement
- 2-month minimum retention time with additional treatment above ground

2.4.1 Treatment Requirements

The direct injection alternative does not benefit from SAT and therefore needs to provide a higher degree of treatment above ground at a treatment facility itself. The GRR requires direct injection projects to have FAT (e.g., MF/RO/AOP). The GRR has specific requirements for the RO and AOP technologies in the FAT train. The RO membranes must achieve a minimum and average sodium chloride rejection of 99.0% and 99.2%, respectively. The initial RO permeate TOC must be less than 0.25 mg/L and not exceed 0.5 mg/L over the long term, based on a 20-week running average of all TOC results and the average of the last four TOC results.

There are two options for demonstrating the performance of the AOP. The first option is to conduct an occurrence study to look at one constituent from each of nine classes of chemicals and demonstrate between 0.3- and 0.5-log reductions of the various classes. The second, simpler option is to demonstrate 0.5-log removal of 1,4-dioxane. 1,4-dioxane was selected as an indicator because it represents the class of low molecular weight, uncharged chemicals that are difficult to remove through RO, and it is one of the more difficult chemicals to remove by advanced oxidation. Processes that can control 1,4-dioxane are assumed to remove numerous additional CECs, and thereby protect public health.

UV/hydrogen peroxide is the most common AOP in place for groundwater replenishment reuse projects. UV/free chlorine offers some unique advantages, and is being implemented as an alternative AOP at the City of Los Angeles, Bureau of Sanitation Terminal Island WRP. There are also situations where ozone/hydrogen peroxide may be an effective AOP for a GRRP though its inability to remove NDMA is often a limiting factor.

2.4.1.1 Brine Disposal

The implementation of an RO process creates brine that will need to be disposed of, a considerable challenge with the chloride TMDL for discharges to the Santa Clara River. A typical recovery for RO is 85% product water with 15% of the feed water being disposed of as brine. This brine is high in salts including chloride, which is well rejected by the RO membrane and builds up to high levels in the brine. Typical disposal methods for brine include truck hauling, ocean disposal, deep well injection, drying beds, and/or maximizing RO recovery.

2.4.1.2 SCVSD Chloride Compliance Project

SCVSD, as part of their chloride compliance project, spent considerable time and energy determining how best to design the optimal AWTF and dispose of the brine in the most economical way. SCVSD is currently in design using a treatment train that includes RO at an anticipated recovery of 99%, thereby minimizing the brine produced. The reduction in brine generation allows SCVSD to truck the brine at an economical rate when compared to other disposal methods. Specifically, SCVSD also studied conveyance to an ocean outfall and deep well injection as alternatives for brine disposal, but found that trucking the brine, along with minimizing its formation, was the most economical decision.

The SCVSD treatment train includes MF, enhanced brine concentration (EBC), RO, and UV for disinfection. The EBC process is designed to pretreat the water prior to RO to reduce certain target constituents that commonly foul RO membranes including calcium, magnesium, and other salts while allowing chloride to pass through to be removed by the RO. Figure 13 shows a schematic of the treatment train. The EBC process consists of nanofiltration (NF), ion exchange (IX) and pH control. The brine from the RO process will be trucked to the Sanitation Districts of Los Angeles County (LACSD) Joint Water Pollution Control Plant in Carson for disposal.

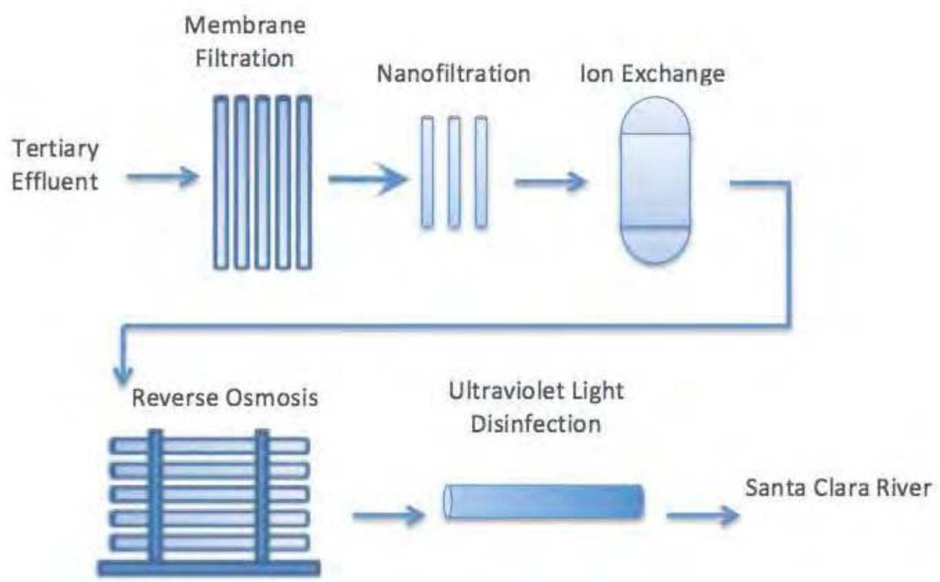

Figure 13 Valencia WRP's AWTF for Chloride Compliance

2.4.2 Proposed Treatment Train

Any advanced treatment train constructed as part of a direct injection IPR project will undergo the same set of challenges regarding brine disposal as those faced by SCVSD. As a result, a modified version of the treatment train selected by SCVSD was used for analysis and consideration for any CLWA AWTF requiring RO to minimize brine generation and disposal. As discussed, this treatment train consists of MF, EBC (NF, IX, pH control), RO, and UV. In the case of a direct injection project, the UV system must be designed for high doses capable of advanced oxidation, not simply for disinfection. This is the one modification from the SCVSD treatment train for the proposed AWTF for CLWA as shown in Figure 14.

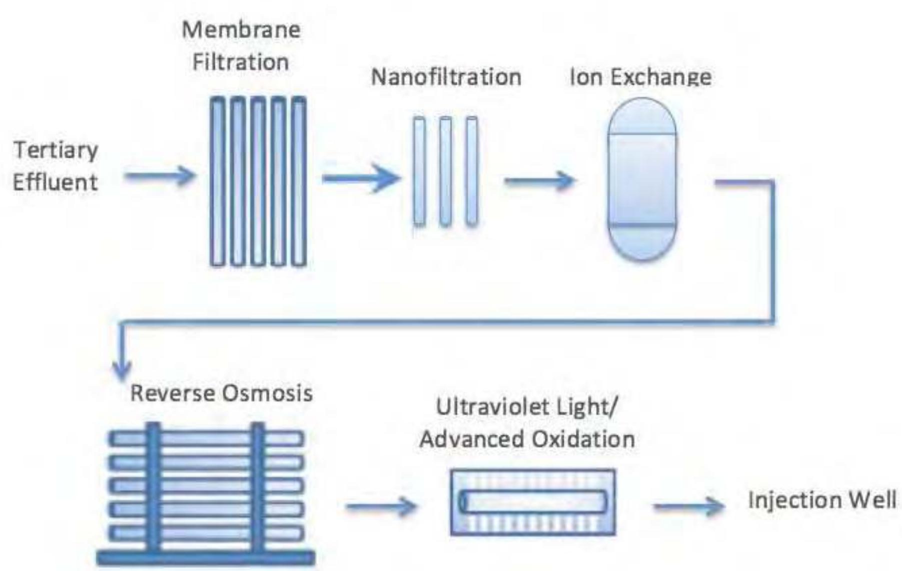

Figure 14 Proposed AWTF for Direct Injection Alternative

The recommended UV AOP could use either hydrogen peroxide or hypochlorous acid as the oxidant to drive the AOP reaction. A conservative estimate of the potential footprint of the AWTF is shown in Figure 15.

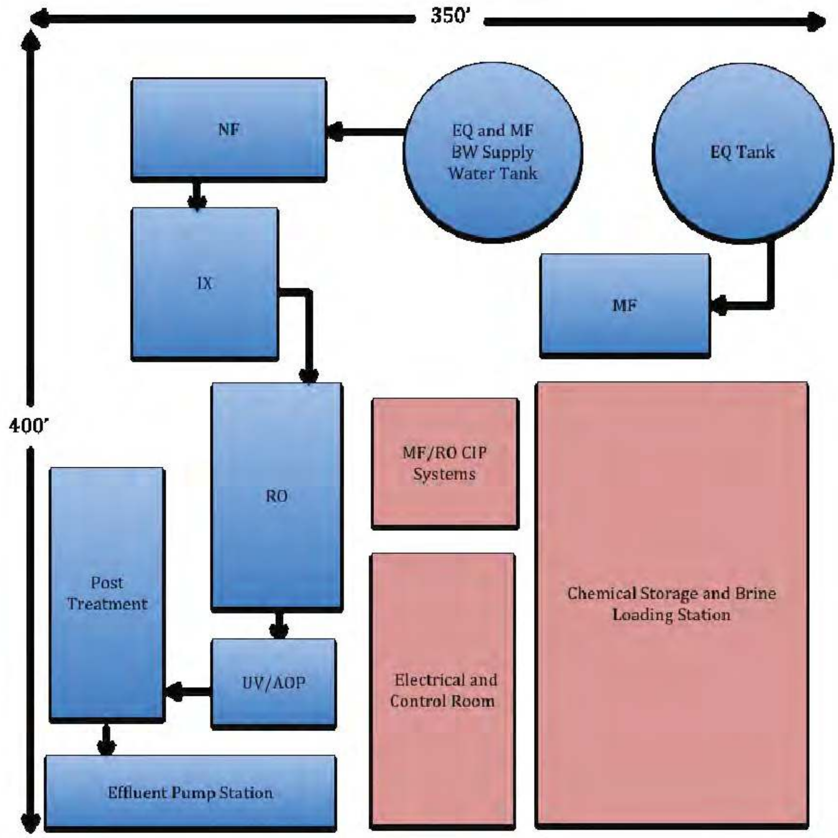

Figure 15 Preliminary AWTF Layout for Direct Injection Alternative

2.4.3 RW Quality

Since the water will be advanced treated through an RO system, it is anticipated that the water quality will be well below any regulated limits. Table 14 shows the anticipated water quality of several key constituents from the AWTF.

Table 14 Key Water Quality Parameters Projected Through AWTF for Direct Injection Alternative

Constituent	Units	VWRP Effluent	AWTF Effluent	Regulatory Requirement
Total Dissolved Solids	mg/L	690	<50	800[2]
Total Organic Carbon	mg/L	4.7	<0.1	0.5[1]
Nitrate (as nitrogen)	mg/L	2.60	<0.1	10[2]
Total Nitrogen	mg/L	4.6	2-4	10[1]
Chloride	mg/L	126	<10	150[2]
Sulfate	mg/L	178	<10	150[2]
1,4-Dioxane	µg/L	0.86	<0.15	1[3]
N-Nitrosodimethylamine (NDMA)	ng/L	121	<2	10[3]

[1] Groundwater quality objectives (GWQO) as stated in the Salt and Nutrient Management Plan (SNMP) of the Santa Clara River Valley East Subbasin.
[2] GRR requirement. Refer to Section 2.3.6.
[3] Table 64431-A (Inorganic Chemicals) of the Title 22 California Code of Regulations
[4] California notification limits (NLs) set by the Department of Drinking Water (DDW).

2.4.4 RW Availability

The Direct Injection alternative is not restricted by the RWC, as the GRR allows for 100% RWC upon commencement of the project (rather than the 20% initial RWC for surface spreading). Therefore, a direct injection project is not restricted by the amount of diluent water. An injection project is also not hindered by inclement weather as water can be injected into the ground regardless of the weather conditions. As such, all of the available RW can be utilized in a Direct Injection project. Furthermore, given the capital investment required for the AWTF, maximizing the usage of all available RW will be critical for creating the most economical alternative possible. Therefore, the AWTF is designed to treat all available RW for potable reuse. The capacity of the AWTF meets the maximum monthly available RW flow as shown in Table 15, or 9.7 MGD.

Table 15 Monthly RW Availability

Month	Monthly RW Availability (MGD)
Jan	8.9
Feb	9.6
Mar	8.2
Apr	5.0
May	1.7
Jun	0.0
Jul	0.0
Aug	0.0
Sept	1.8
Oct	4.8
Nov	9.3
Dec	**9.7**
Annual Average	**4.9**

2.4.5 Potential Injection Well Locations

The injection wells can inject the RW into either the Saugus Formation or the Alluvial Aquifer in the Valley's groundwater basin. The Recon Study identified two potential locations, but considered the use of SWP for injection and as such, did not track the travel time between the injection wells and nearby potable water wells. If this alternative is selected for further consideration, additional modeling of the Saugus Formation and travel times will need to be performed to accurately site the injection well location. Figure 16 shows the recommended location of the wells as discussed in the Recon Study respective to the Valencia and Saugus WRPs. To minimize additional costs, it is assumed that the injection wells could be located onsite at the Valencia WRP, along with the AWTF. SCVSD indicated that they were not sure if there would be available footprint, so additional conveyance costs are possible if the AWTF and injection well needs to be located away from the existing Valencia WRP.

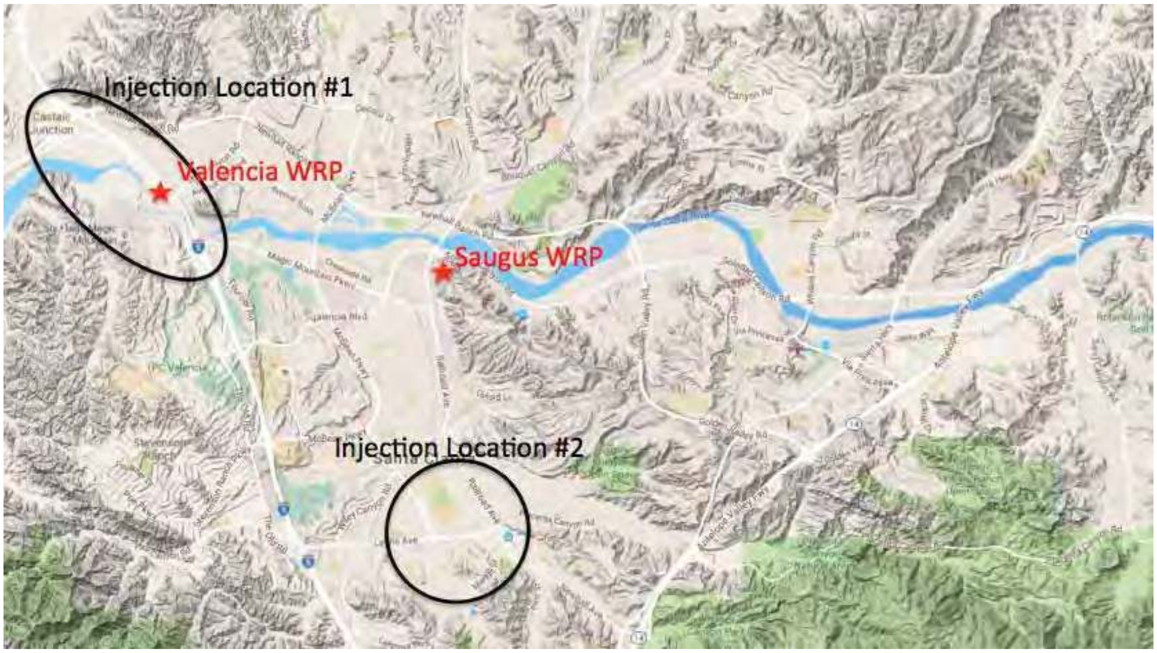

Figure 16 Injection Location Identified in Recon Study for Aquifer Storage and Recovery

2.4.6 Retention Time and Microorganism Control

The GRR mandates a minimum retention time in the groundwater basin of 2 months. No existing facilities currently operate with a retention time under 6 months, although at least four projects in planning stages are proposing such alternatives (Padre Dam, OCWD, WRD and Cambria). Minimizing the travel time underground will likely require that other aspects of the project are enhanced to compensate for the shorter retention times, including the use of enhanced treatment above ground and enhanced monitoring. Enhanced treatment indicates the need for moving beyond the 12/10/10 microorganism removal requirement stipulated in the GRR and would require additional treatment beyond what is stipulated in the currently recommended treatment train. Enhanced monitoring would require identification of additional surrogates or indicators capable of defining the treatment performance in a near time manner, allowing plant operators to notify water purveyors and DDW in a timely manner if a problem with the treatment system is identified.

For this study, it was assumed that a travel time of 6-months could be identified within the aquifer nearby the Valencia WRP. Additional consideration of this alternative should include a detailed analysis of groundwater travel times.

Table 16 Anticipated Pathogenic Microorganism Control for Direct Injection

Pathogenic Microorganism	Goal	Primary, Secondary, Tertiary	MF	NF	RO	UV/AOP	Subsurface Travel Time (6 months)	Total
log virus	12	1.9	0	1	1.5	6	6	16.4
log Giardia	10	0.8	4	1	1.5	6	0	13.3
log Crypto	10	1.2	4	1	1.5	6	0	13.7

2.4.7 Diluent Volume

The GRR stipulates that a direct injection project can have a RWC of 100% upon commencement. This makes the reliance of native groundwater a non-factor and as such is considered no further.

2.4.8 RW Contribution

The GRR stipulates that a direct injection project can have a RWC of 100% upon commencement. This makes the reliance on native groundwater a non-factor and as such is considered no further.

2.4.9 TOC and Ultimate Utilization

As previously indicated, the GRR requires that the RO process meet certain guidelines, including achieving an effluent TOC below 0.5 mg/L, based on a 20-week running average of all TOC results and the average of the last four TOC results. This allows the TOC requirement of 0.5 mg/L of wastewater origins to be met at all times and thus, no background diluent water is required. As such, all available product water from the AWTF can be injected into the groundwater basin and will be able to meet the TOC requirement.

Table 17 Direct Injection Alternative Flow Overview

Potable Reuse Scenario	Available RW (MGD)	Initial RW (MGD)	Ultimate RW (MGD)
Direct Injection	4.9	4.9	4.9

3. Surface Water Augmentation

Senate Bill 918 requires DDW to develop and promulgate regulations for surface water augmentation (SWA) by the end of 2016. SWA projects are similar to groundwater recharge in that they also use an environmental buffer--in this case, a reservoir--in between treatment and distribution. A schematic of a typical SWA project is shown in Figure 17. Key elements of SWA project requirements include pathogen and chemical control at the AWTF and retention time and dilution requirements in the reservoir.

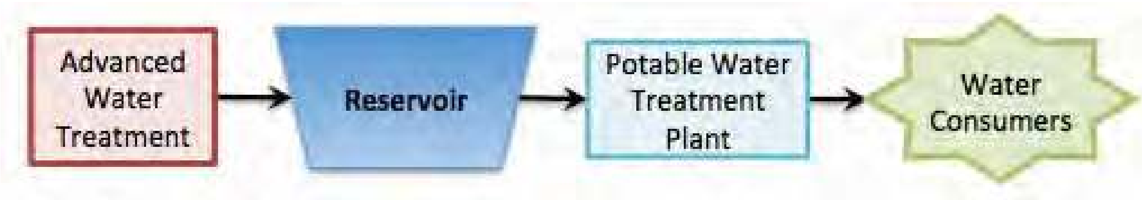

Figure 17 Schematic of a Typical SWA Project

3.1 Treatment Requirements

In the most recent draft SWA regulations, the treatment requirements look very similar to the GRR, particularly with regard to pathogenic microorganism control. Two main treatment pathways are available: (1) 12/10/10 for V/G/C with at least 100:1 dilution achieved in the reservoir, or (2) 12/10/10 for V/G/C with at least 10:1 dilution achieved in the reservoir and an additional 1-log of treatment provided by an additional process[1] - i.e., 13/11/11 for V/G/C. The size of the Castaic Lake reservoir and the anticipated project flow is such that at least 10:1 dilution can likely be achieved in the reservoir; thus, the pathogenic microorganism control requirement for CLWA's SWA project is likely to be 13/11/11 for V/G/C (for further information, see Section 3.7 Diluent Volume).

Where treatment credits are concerned, the principal difference between groundwater recharge and reservoir augmentation is the availability of treatment credit in the conventional drinking water treatment plant. The original surface water treatment rule, promulgated by EPA (EPA 1989), required the water treatment plant to provide treatment to remove 4-log virus and 3-log *Giardia*. This rule has since been updated to include 2-log *Cryptosporidium* removal as well. SWA projects can combine the treatment credit achieved prior to the reservoir and at the conventional drinking water treatment plant to achieve the required pathogen reductions. Assuming a requirement of 13/11/11 for V/G/C in the project overall, taking into account the 4/3/2 removal achieved at the drinking water treatment plant brings the minimum treatment requirements prior to the reservoir to 9/8/9.

3.2 Proposed Treatment Train

The primary purpose of designing the treatment processes will be to design a treatment system that has enough credit to achieve the required 12/10/10 log removal requirement for V/G/C by the draft SWA regulations and considers the drinking water treatment that is received on the downstream side of the reservoir storage. For this application, a similar treatment train is suggested as for the direct injection approach, as was shown in Figure 14. The capacity of the treatment system is the same, treating all available RW and sized at 9.7 MGD. The layout of the facility is the same as for direct injection as was shown in Figure 15.

[1] The process used to provide the additional 1-log of treatment does not need to be a unique type of process, but does need to be independent of and not reliant on the other treatment processes

3.3 RW Quality

The inclusion of an AWTF with an RO system will keep the product water quality well below any current regulatory limits. However, it is possible that the LARWQCB may require strict nutrient limits for environmental reasons, lowering the total nitrogen discharged as low as 1 mg/L.

3.4 RW Availability

Similar to Direct Injection, the SWA alternative is not restricted by the RWC. Therefore, the AWTF is being designed to treat all available RW and will have a capacity of 9.7 MGD to treat the maximum month RW flow (See Section 2.4.4 RW Availability).

3.5 Reservoir Specifications

Figure 18 Aerial of Castaic Lake

CLWA receives their imported SWP water through the Castaic Lake Reservoir. The Castaic Lake Reservoir is a 320,000 acre-foot lake located on the northern edge of the CLWA service area. CLWA owns and operates the Earl Schmidt Filtration Plant, located on the southern border of the Castaic Lake Reservoir, which receives and treats water from the Castaic Lake Reservoir. The Metropolitan Water District of Southern California also uses the Castaic Lake Reservoir as part of its conveyance system for routing SWP water to customers in the Southern California area. As a result, there is a relatively low retention time in the reservoir considering its size.

Due to the ongoing drought, the Castaic Lake Reservoir has seen an unprecedented drop in water storage. This can be seen most clearly in Figure 19, which shows the water level in the reservoir over the past eight years. For dilution and retention time calculations, the ultimate low water height and its corresponding volume that occurred on March 24, 2015 was used.

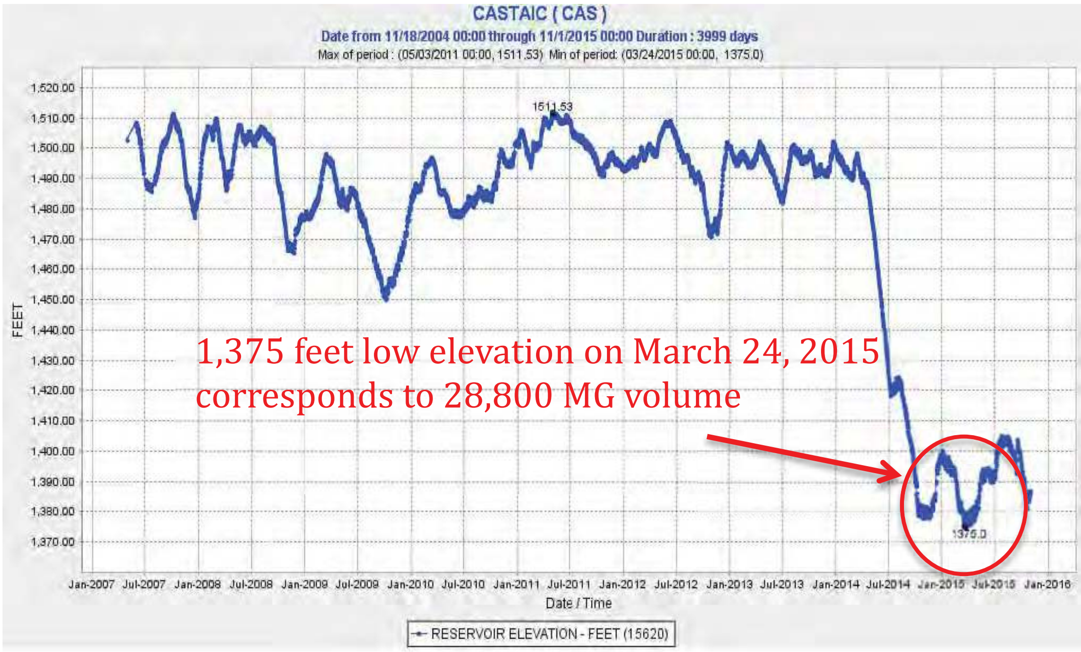

Figure 19 Castaic Lake Historical Elevation (2007-present)
(Source: California Department of Water Resources)

3.6 Retention Time

The draft SWA regulations continue to incorporate the concept of retention time, albeit taking into account the differences in hydrodynamics between an aquifer and a reservoir. The draft regulations stipulate that a reservoir used for SWA must have a minimum theoretical retention time of 6 months, to be measured on a monthly basis.

$$\tau = \frac{V_{total}}{Qout} \geq 6\ months \qquad (3)$$

where V_{total} is the volume in the reservoir at the end of the month and Q_{out} is the total outflow from the reservoir during that month. The California Department of Water Resources tracks the flow out of the Castaic Lake Reservoir and over the past 10-years an average of 475 MGD leaves the reservoir per year (California Department of Water Resources, 2015). Using the low water level previously discussed, the theoretical retention time can be calculated.

$$\tau = \frac{28{,}800\ MG}{475\ MGD} = 2.0\ months\ \ngeq\ 6\ months \qquad (4)$$

As shown, the theoretical retention time is less than 6 months and thus this SWA project does not qualify under the current draft regulations. Because of the large outflows from the reservoir for other purposes, reduction of project flow would not enable this project to qualify. Unlike the groundwater regulations, there is no stipulation in the draft SWA regulations that allows for a project sponsor to petition the DDW for an alternative permitting process *for the reservoir criteria*[2]. Currently, discussions regarding this alternative permitting process are ongoing as many potential project sponsors are finding

[2] An alternative permitting pathway is available for other project components, including treatment, source control, and monitoring.

themselves in a similar situation with a lower retention time than stipulated in the draft regulations. A decision will be made in later 2016 whether to allow some flexibility in this requirement.

3.7 Dilution Requirement and Microorganism Control

The draft regulations stipulate dilution requirements for AWTF water discharged into the reservoir. The basis of these requirements is that any 24-hour input of RW to the reservoir must be mixed such that water withdrawn for use as drinking water will never contain more than 1% (or 10% with an additional log of treatment) of this input. The intent of this requirement is to provide a buffer against off-specification water that enters the reservoir; pathogen concentrations will be reduced by 2 logs, either through 100:1 dilution or 10:1 dilution with 1-log treatment.

Table 18 Draft SWA Regulation Microorganism Control Requirements

Dilution	Enteric Virus Removal	*Cryptosporidium* Removal	*Giardia* Removal
≥100:1	12-log	10-log	10-log
≥10:1	13-log	11-log	11-log
<10:1	Not classified as surface water augmentation		

To demonstrate compliance with this requirement, the draft regulations require hydrodynamic modeling that verifies the ability of the reservoir to meet this requirement under all conditions, as well as completion of a tracer study with added tracer prior to the end of the first six months of operation. The achievable dilution of a 24-hour input to Castaic Lake Reservoir can be estimated using a simplifying assumption of complete mixing; under this assumption, dilution is related to the theoretical retention time and the length of the input (Δt):

$$dilution\ factor = \frac{\tau}{\Delta t} = \frac{60\ days}{1\ day} = 60:1 \qquad (5)$$

This dilution factor means a SWA project using the Castaic Lake Reservoir would be required to achieve 13/11/11 removal for V/G/C, a slightly more stringent requirement than for groundwater recharge. Although no removal credit is given for retention time in the reservoir, the credit received at the Earl Schmidt Filtration Plant can reduce the treatment requirements at the AWTF. Table 19 shows the anticipated microorganism removals based on the developed treatment train. The draft regulation requires that no less than 9/8/9 logs of removal be achieved prior to discharge to the reservoir.

Table 19 Anticipated Pathogenic Microorganism Control for SWA

Pathogenic Microorganism	Goal	Primary, Secondary, Tertiary	MF	NF	RO	UV/AOP	Filtration Plant[1]	Total
log virus	13	1.9	0	1	1.5	6	4	14.4
log Giardia	11	0.8	4	1	1.5	6	3	16.3
log Crypto	11	1.2	4	1	1.5	6	2	15.7

[1] SWTR mandated log removal values are assumed.

3.8 Conveyance Concept

A 9-mile pipeline following Interstate-5 is proposed to convey the advanced treated water from the Valencia WRP to the Castaic Lake Reservoir as shown in Figure 20. Facility capital and operations costs are presented in the Recycled Water Master Plan.

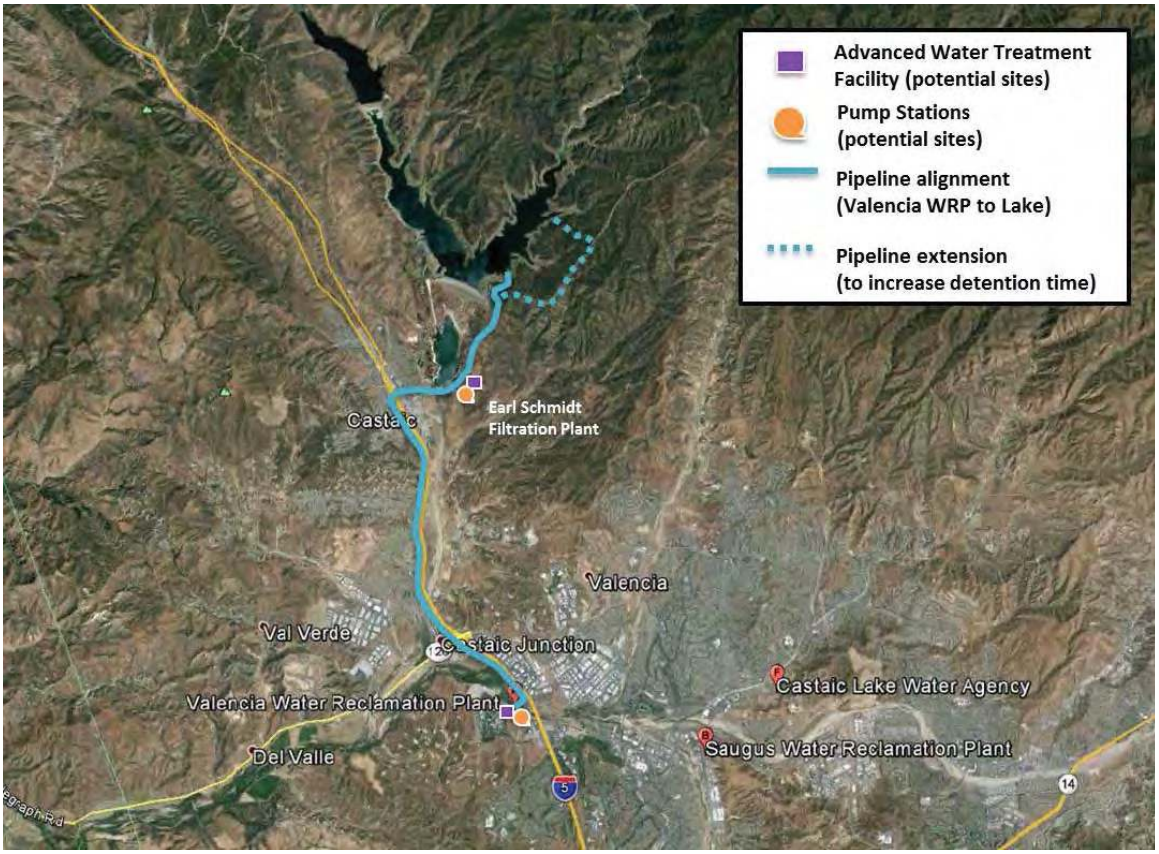

Figure 20 Conveyance Concept for SWA Project

Table 20 SWA Alternative Overview

Potable Reuse Scenario	Available RW (MGD)	Initial RW (MGD)	Ultimate RW (MGD)
SWA	4.9	4.9	4.9

4.0 Direct Potable Reuse

Direct potable reuse (DPR) has a spectrum of alternatives with significant differences in the 'directness' they seek. At one extreme, the finished water production scenario envisions an AWTF piped directly to a distribution system with no intervening barriers, storage, or retention time provided. This is the most direct form of DPR. On the other hand, AWTF water could be piped to a reservoir that is too small to comply with the surface water augmentation criteria. This water could be blended with existing source water, treated through a drinking water treatment plant, and then sent on to distribution. As such, a project classification between DPR and SWA may rely simply on the size and flow through a drinking water reservoir. Figure 21 illustrates the differing degrees of DPR project alternatives.

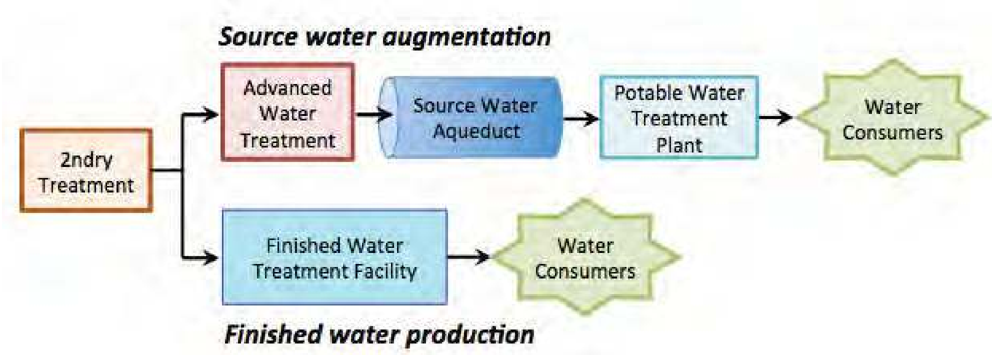

Figure 21 Potential Configurations of DPR Options and Comparison with SWA

SB918 has as its final requirement that DDW assess the feasibility of developing regulations for DPR. It is important to note that SB 918 does not require the development of regulations, but only an assessment of whether or not it is feasible to do so. ***There is no mandated timeline for the state to develop a formal DPR regulatory framework.***

The concept of DPR is fairly new and relatively untested. As a result, there is very little data on DPR design, performance, and safety. Such information is critical to assess DPR feasibility and as a result significant research efforts have recently commenced. Figure 22 provides an overview of the various research themes being pursued primarily by the WateReuse Research Foundation (WRRF), WateReuse California, and Water Research Foundation (WRF), in addition to other international partners.

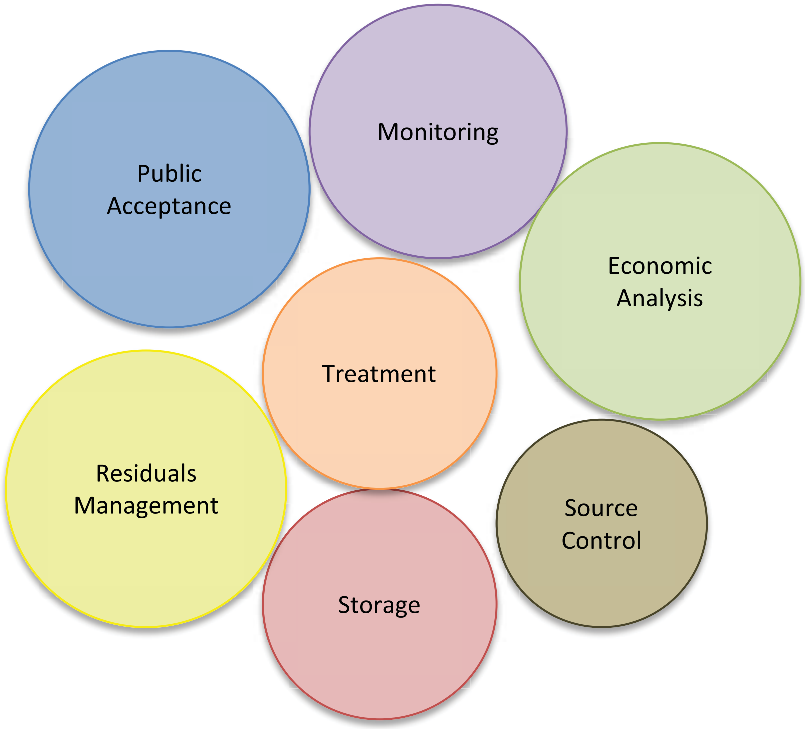

Figure 22 Ongoing Areas of DPR Research

4.1 Treatment Requirements

WRRF has created a keystone project that seeks to tie together many of the findings from the last few years of potable reuse research. This project is WRRF 14-12, entitled "Demonstrating Redundancy and Monitoring to Achieve Reliable Potable Reuse," a 1.6-MGD demonstration project at the City of San Diego's North City Water Reclamation Plant. This project ties together multiple aspects of DPR research on treatment, monitoring, and storage to address the fundamental issue of reliability in public health protection.

One result from recent potable reuse research is that the elements of public health protection--- treatment, monitoring, and storage-- can be balanced in different ways to still provide equal public health protection. For example, as retention time is reduced, increases in treatment and monitoring can compensate for equal protection. This most clearly can be seen with existing GRR, which require a minimum of 6-month retention time for less-treated Title 22 water (see Surface Spreading GWR alternative), yet a 2-month minimum retention times is allowed for full advanced treated water. A similar framework can be seen in the draft surface water augmentation regulations, which require 13/11/11 (V/G/C) logs of pathogen removal (instead of 12/10/10) if the reservoir provides less dilution.

4.2 Proposed Treatment Train

Project 14-12 has developed a DPR concept train that further augments both the treatment protection and the monitoring to provide continuous and demonstrable performance of a DPR train. The treatment train provides redundancy in both treatment and monitoring to reduce the probability that the system will fail to treat the water to the required levels. It also provides new and different barriers in the form of ozone and BAC pre-treatment, offering two new and different mechanisms to control the wide diversity of potential chemical and microbiological threats. Finally, the system has a high degree of monitoring to detect system compromises and failures, and respond accordingly. The treatment train from Project 14-12 is shown in Figure 23.

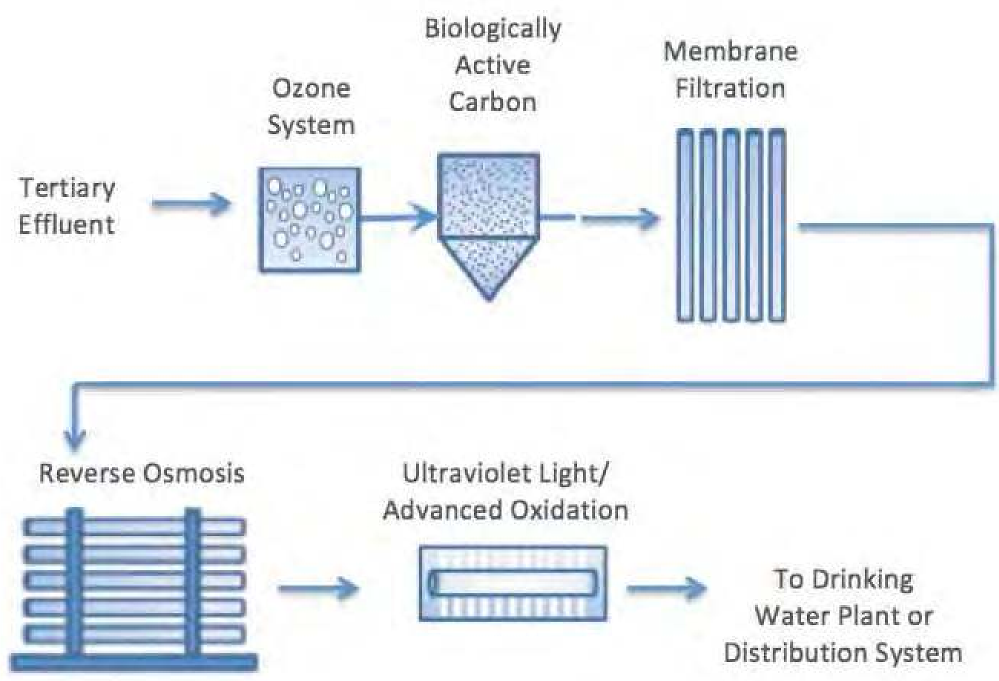

Figure 23 WRRF 14-12 Demonstration Treatment Facility

To maintain the desire to minimize brine, the treatment train used in WRRF 14-12 was modified to mirror the SCVSD chloride compliance project with the addition of ozone and BAC as pretreatment. Figure 24 shows the proposed treatment train. A conservative estimate of the preliminary layout for the proposed AWTF is shown in Figure 25.

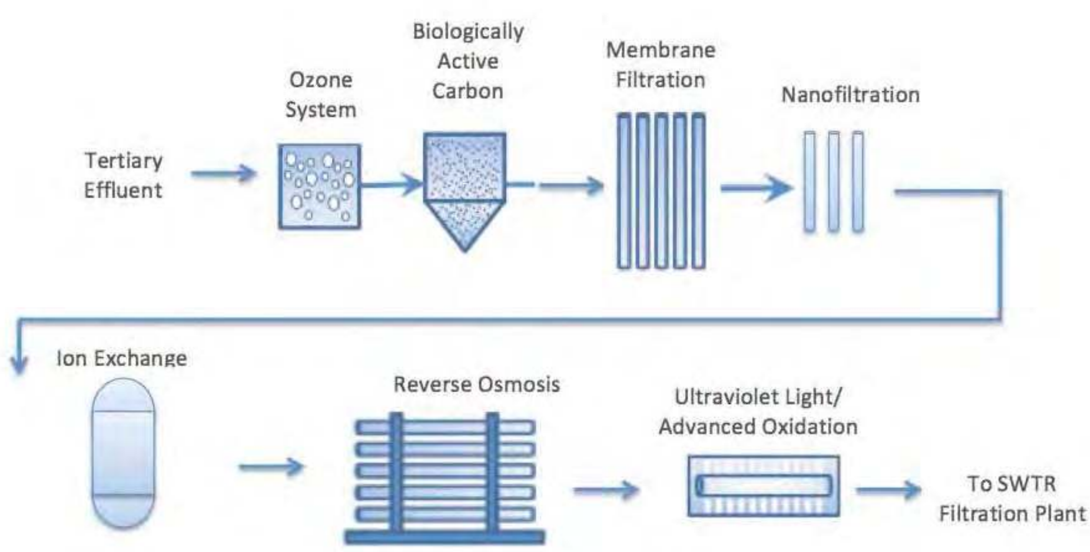

Figure 24 Proposed AWTF for DPR Treatment Alternative

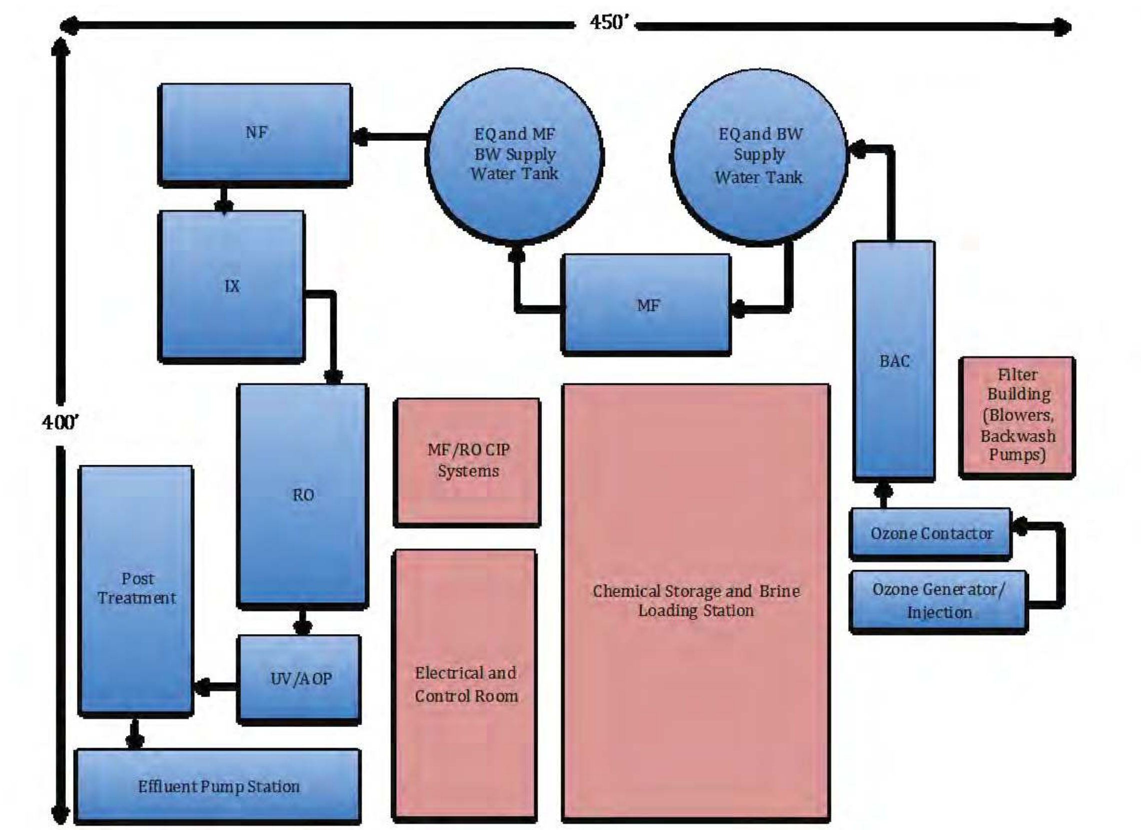

Figure 25 Preliminary AWTF Layout for DPR Treatment Alternative

4.3 Diluent Volume and Microorganism Control

While there is no framework yet for DPR, the effective microorganism control of the proposed treatment train was determined and is shown in Table 21. It is anticipated that a minimum of 13/11/11 will be required, as indicated by the SWA draft regulation where dilution is at a minimum.

Table 21 Anticipated Pathogenic Microorganism Control for DPR

Pathogenic Microorganism	Goal[1]	Primary, Secondary, Tertiary	O3	BAC	MF	NF	RO	UV AOP	Filtration Plant[2]	Total
log virus	≥13	1.9	6	0	0	1	1.5	6	4	20.4
log Giardia	≥11	0.8	6	0	4	1	1.5	6	3	22.3
log Crypto	≥11	1.2	2	0	4	1	1.5	6	0	15.7

[1] The DPR requirements are not developed and it is presumed that they will be no less than 13/11/11 to meet the most stringent requirements of the draft SWA regulations.
[2] SWTR mandated log removal values are assumed.

4.4 Conveyance Concept

The proposed DPR concept alternative involves sending the advanced treated water from Valencia WRP to the Rio Vista Filtration Plant for further treatment prior to distribution. Figure 26 shows the conveyance concept. ***It is important to note that this alternative is speculative as there is neither a developed framework for regulations nor any established timeframe for promulgating DPR regulations.***

Figure 26 Conveyance Concept for DPR

Table 22 DPR Alternative Overview

Potable Reuse Scenario	Available RW (MGD)	Initial RW (MGD)	Ultimate RW (MGD)
Direct Potable Reuse	4.9	4.9	4.9

5. SUMMARY OF POTABLE REUSE ALTERNATIVES

Table 23 summarizes the flows for all considered potable reuse alternatives. For all of the potable reuse scenarios, the available RW is dependent on population growth and water conservation because the projected flows are derived on a per capita basis. Additionally, the available RW is dependent on other non-potable RW demands such as the planned Newhall Ranch development. If additional RW were made available to potable reuse (eg. if purple pipe is not constructed as planned) more RW would be available for spreading (see Table 13 for ultimate spreading capacities). Finally, the addition of ozone as a pretreatment step to spreading would allow additional volume to be spread (even beyond what is stipulated in Table 13) and would assist in alleviating public perception by providing an additional treatment barrier that is effective at the destruction of CECs.

Table 23 Alternative Comparison

Potable Reuse Scenario	INITIAL		ULTIMATE		DESIGN
	Ave Annual Flow (MGD)	Annual Recharge Volume (AFY)	Ave Annual Flow (MGD)	Annual Recharge Volume (AFY)	Peak Flow for Conveyance (MGD)
Recharge Location #1	3.3	3,700	3.3	3,700	9.7
Recharge Location #3	1.1	1,200	3.3	3,700	9.7
Direct Injection	4.9	5,500	4.9	5,500	9.7
Surface Water Augmentation	4.9	5,500	4.9	5,500	9.7
Direct Potable Reuse	4.9	5,500	4.9	5,500	9.7

AFY = acre-feet per year

Note: Average and annual recharge volumes are based on 2050 available recycled water flows from the Valencia WRP.

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Appendix D: Hydraulic Model Information

The following "Technical Memorandum: Recycled Water Model EPS Calibration and System Analysis" (IDModeling 2016) describes the hydraulic modeling to support the evaluation of alternatives.

This study describes:

(1) calibration of the existing Phase 1 Recycled Water System
(2) analysis of the Alternative 1 - Phase 2 projects
(3) analysis of the Alternative 3 – Groundwater Recharge via Surface Spreading projects, and
(4) analysis of the Alternative 2 – Future Expansion projects.

Detailed results are presented in the following attachments to the TM:

Attachment A – Existing System Results
Attachment B – Phase 2A-1 Results
Attachment C – Phase 2A-2 Results
Attachment D – Phase 2A-3 Results
Attachment E – Phase 2B Results
Attachment F – Phase 2C and 2D Results
Attachment G – IPR Scenario 1 Results
Attachment H – IPR Scenario 2 Results
Attachment I – IPR Scenario 3 Results
Attachment J – IPR Scenario 4 Results
Attachment K – Phase 2A + Future Expansion North Results
Attachment L – Phase 2C + Future Expansion South Results

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CC:	Paul Hauffen, IDModeling, Inc.
DATE:	September 2, 2016

1.0 Introduction

IDModeling (IDM) has developed this technical memorandum to report on the results of the Castaic Lake Water Agency (CLWA or Agency) recycled water system model calibration and system analysis (Project). The goal of this Project was to calibrate CLWA's existing recycled water system hydraulic model and analyze the proposed future system as part of the CLWA's master planning process. This document describes the model calibration process, development of the future phases, summarizes the results of the extended period simulation (EPS) calibration, and includes analysis results for future phases.

This TM includes the following sections and attachments:

Section 1 – Introduction
Section 2 – Recycled Water Model Calibration
Section 3 – Phase 2 Analysis
Section 4 – Phase 2A with Indirect Potable Reuse Flows
Section 5 – Future Expansion Analysis

Attachment A – Existing System Results
Attachment B – Phase 2A-1 Results
Attachment C – Phase 2A-2 Results
Attachment D – Phase 2A-3 Results
Attachment E – Phase 2B Results
Attachment F – Phase 2C and 2D Results
Attachment G – IPR Scenario 1 Results
Attachment H – IPR Scenario 2 Results
Attachment I – IPR Scenario 3 Results
Attachment J – IPR Scenario 4 Results
Attachment K – Phase 2A + Future Expansion North Results
Attachment L – Phase 2C + Future Expansion South Results

2.0 Recycled Water Model Calibration

The following sections describe the model calibration process including model facility updates, diurnal pattern development, demand allocation, and development of the EPS calibration scenario.

2.1 System Configuration

CLWA's recycled water system receives its supply from the Valencia Water Reclamation Plant (WRP) located near the intersection of The Old Road and Rye Canyon Road. The system consists of one pump station located at the Valencia WRP and one 1.5 MG storage reservoir located at Woodridge Parkway north of Valencia Boulevard. There are approximately 3.5 miles of pipe in the recycled water distribution system ranging in diameter from 8-inches to 36-inches. **Table 1** below shows a summary of the pipes by diameter within the system. **Figure 1** shows the existing recycled water system.

Table 1 – Recycled Water System Pipelines by Diameter

Diameter	Length (ft)	Length (miles)	Percent of Total (%)	Notes
8	150	0.03	0.80	Pipe located near existing reservoir, no as-built information
12	2,800	0.53	14.97	
18	20	0.00	0.11	Pump station discharge piping
20	11,770	2.23	62.94	
24	3,500	0.66	18.72	
36	370	0.07	1.98	Pump station discharge piping
42	90	0.02	0.48	Pump station suction piping
Total	**18,700**	**3.54**	**100.00**	

In addition, the existing pipeline system was updated to include laterals to the existing Valencia Meters. The Valencia meter data was used to identify the location and lateral size to connect to the existing system.

2.2 Demand Development

The existing system demand was allocated using meter data provided by CLWA as part of the 2015 existing model update. Demand data was processed for each customer account to determine the individual and system-wide average day demand (ADD) using the 2014 total annual usage at each existing metered customer. Demands were assigned to the model node closest to the actual meter point for each account to ensure the most accurate distribution of demands across the system.

To create the maximum day demand (MDD) scenario, the Agency provided SCADA data for both August 2015 and September 2015. The daily demand was calculated for this time period. The MDD of 1.1 mgd occurred on August 6, 2015. However, this data could not be used due to missing pump flow and pressure data for approximately 7 hours over the day. The calibration effort was based on useable SCADA data that occurred between August 16 and 17, 2015.

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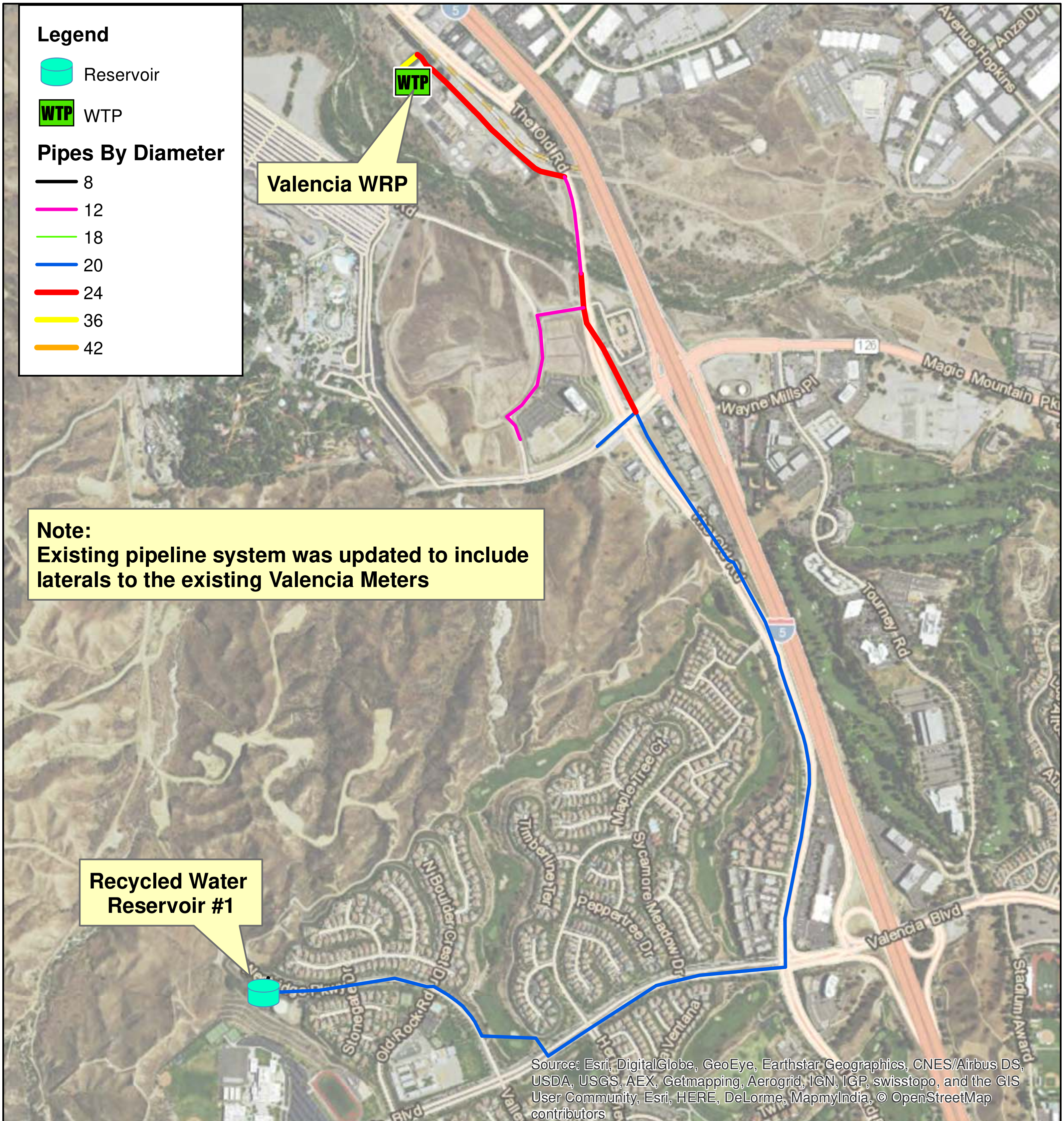

Figure 1
Pipes by Diameter
Existing System

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The following demands were allocated to the model junctions:

- ADD = 0.68 MGD (2014 data)
- MDD = 0.88 MGD (August 16-17; average of 0.75 mgd (August 16) and 1.0 mgd (August 17))
- MDD/ADD multiplier = 1.48 (2015 MDD/ADD used for existing system analysis)

2.2.1 Diurnal Pattern

A diurnal pattern represents the anticipated daily fluctuation in system demand over a specified time period. Patterns are necessary to accurately perform EPS analyses that simulate system performance over the specified time period. A 48-hour diurnal pattern was developed using available SCADA data provided by CLWA. The 48-hour diurnal pattern used 15-minute time intervals. **Figure 2** below shows the diurnal pattern calculated for the existing system.

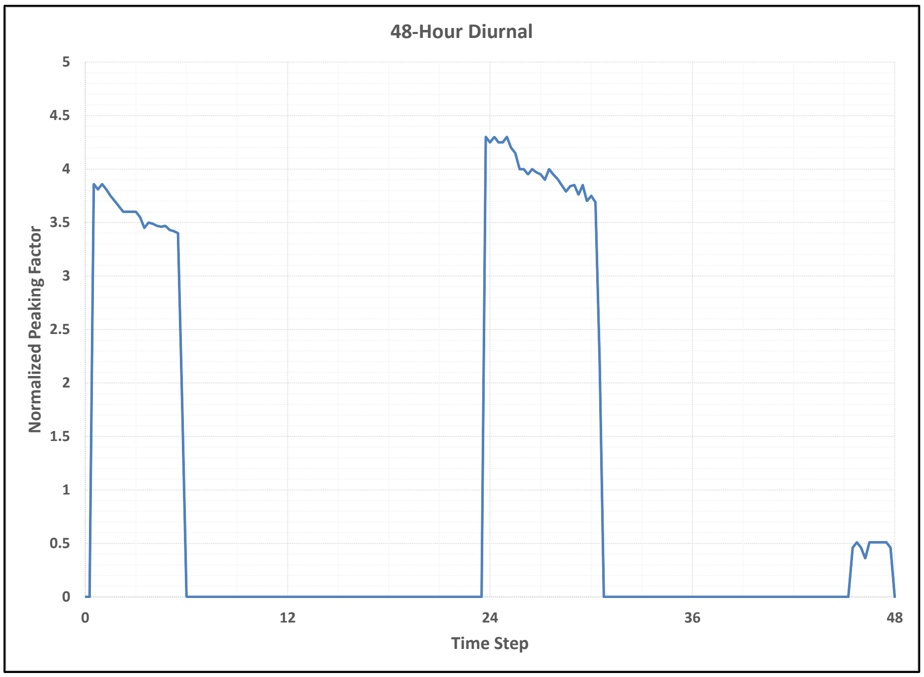

Figure 2 – Diurnal Pattern

2.3 Calibration Criteria

The EPS calibration included comparing modeled results to SCADA data for Storage Reservoir #1 (water level) and the Valencia WRP pump station (discharge pressure and flow). The goal was for modeled data to be within the tolerances listed below:

- Storage Reservoir Levels – Within 1 foot of SCADA data and same trending
- Flows – Within 15 percent of SCADA data at pump station
- Pressures – Within 5 psi of SCADA data at pump station

2.4 Calibration Scenario Setup and Results

2.4.1 Setup

An EPS calibration was performed for a 48-hour duration for the period between August 16 and 17, 2015, based on CLWA provided SCADA data. This time period represented MDD operating conditions. The goal of the calibration was to meet the calibration criteria for all active facilities, with specific focus on correlating storage reservoir water levels and trending. This scenario included all the necessary data sets for performing the calibration, including the MDD data set, operational control set, and the base data sets for all facilities (tanks, valves, reservoirs, pumps, and pipes). Data sets contain the information specific to each facility or demand condition and can be customized for each scenario. Upon completion of the calibration effort an operational scenario was developed to represent current system operations under both MDD conditions.

2.4.2 Results

Model results were compared to SCADA data for the Valencia WRP booster pump station and the storage reservoir. **Figure 3** presents the Valencia WRP booster station calibration results. **Figure 4** presents the storage reservoir calibration results. Additional results are provided in **Attachment A**.

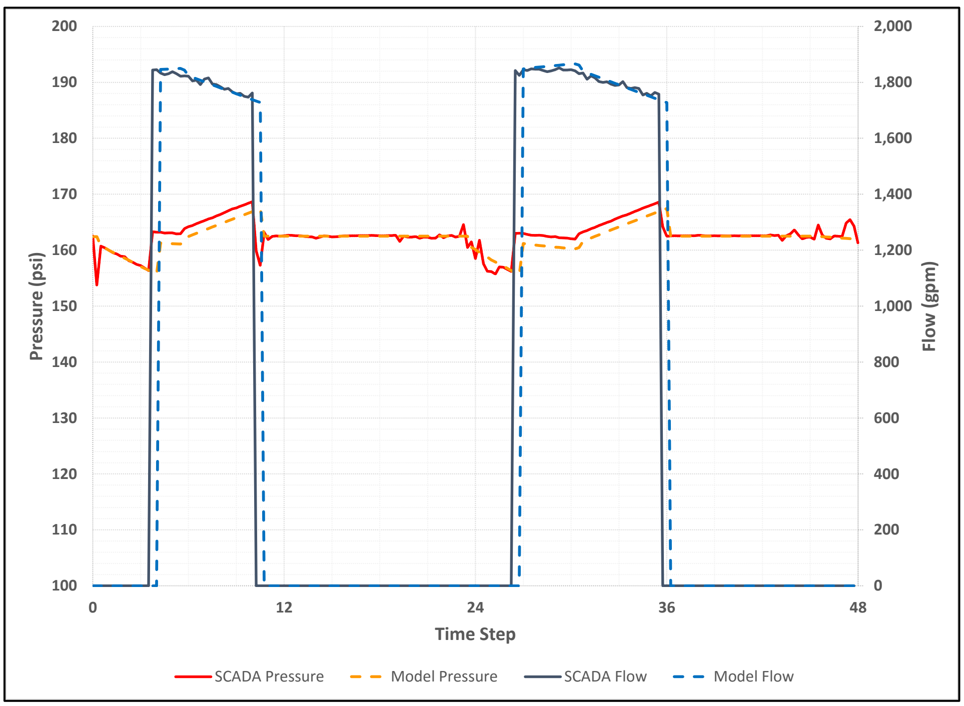

Figure 3 – Valencia WRP Pump Station Result

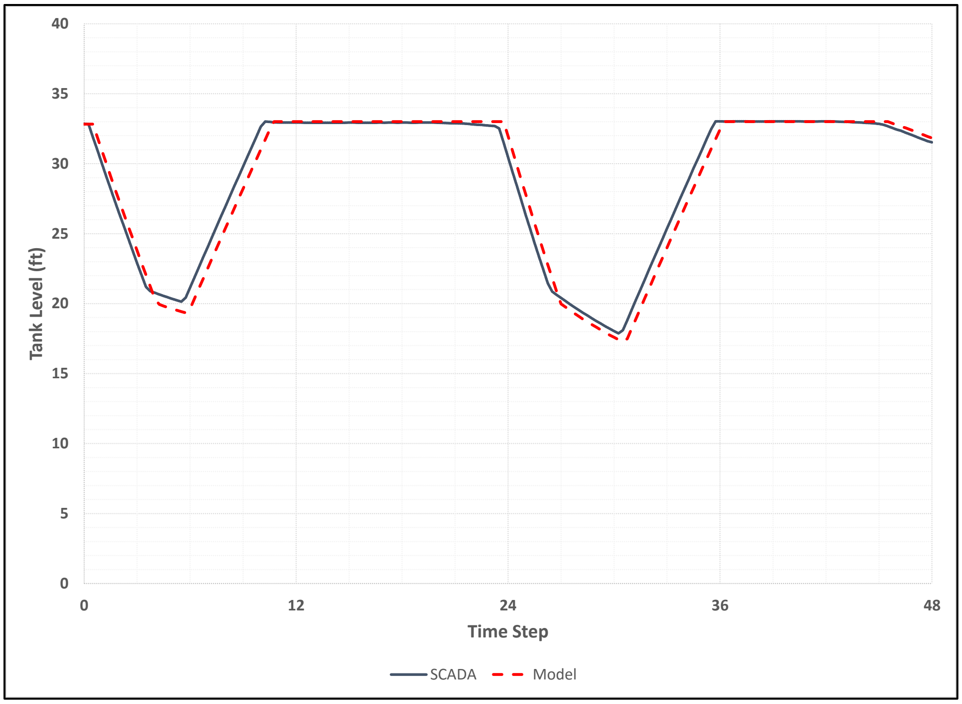

Figure 4 – Storage Reservoir #1 Results

Results indicate a good correlation between the SCADA data and the model. The pump station flows are consistent and the pressure trends are within the established calibration criteria. The modeled storage reservoir water levels are consistent with SCADA data over the calibration period. The main difference seems to be a slight delay for when the model controls activate versus actual operation. This can be attributed to the minor time step differences in the diurnal pattern.

Using the MDD calibration, the minimum pressures and maximum velocities were analyzed to determine where there may be deficiencies within the existing system. **Figure 5** shows the minimum pressures and maximum velocities observed during the calibration. Pressures below 40 psi were identified in the vicinity of Storage Reservoir #1. This is due to the elevation of the pipeline relative to the reservoir water level. Only one pipeline in the system had velocities greater than 5 feet per second (fps) and was located in the 12-inch diameter pipeline segment located at The Old Road along a bridge crossing the Santa Clara River.

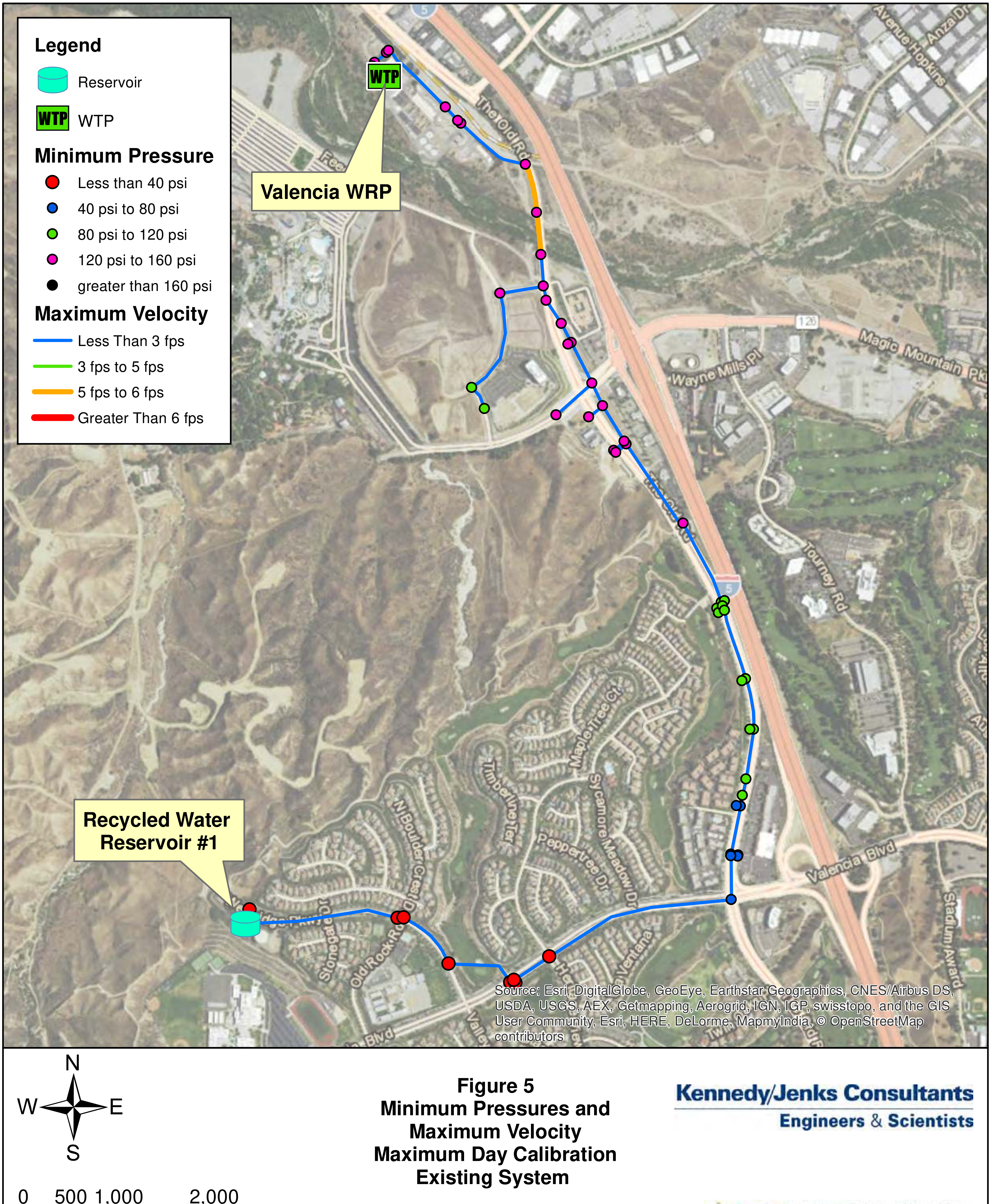

Figure 5
Minimum Pressures and Maximum Velocity Maximum Day Calibration Existing System

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2.5 Conclusions

The EPS calibration included comparing modeled results to SCADA data for storage reservoir levels and the Valencia WRP booster pump station flows. The goal was for modeled data to be within the calibration tolerance, which was achieved. In addition, the existing system was analyzed for deficiencies.

The following deficiencies were identified based on an evaluation of the existing system:

- The 12-inch pipeline across the bridge in The Old Road has a maximum velocity of 5.4 fps, which is acceptable for the current demands, but subsequent modeling showed the velocity increases significantly as demands increase.
- The pressures near the Storage Reservoir #1 are low due to elevation, and it will be difficult to serve new customers in this area. New customers in this area should be allocated to Phase 2D.

3.0 Phase 2 Analysis

The proposed Phase 2 service area includes the expected near term expansion of the recycled water system. All future analyses utilized a pattern with an irrigation window from 10:00 pm to 6:00 am with a peaking factor of 3, as shown in **Figure 6.**

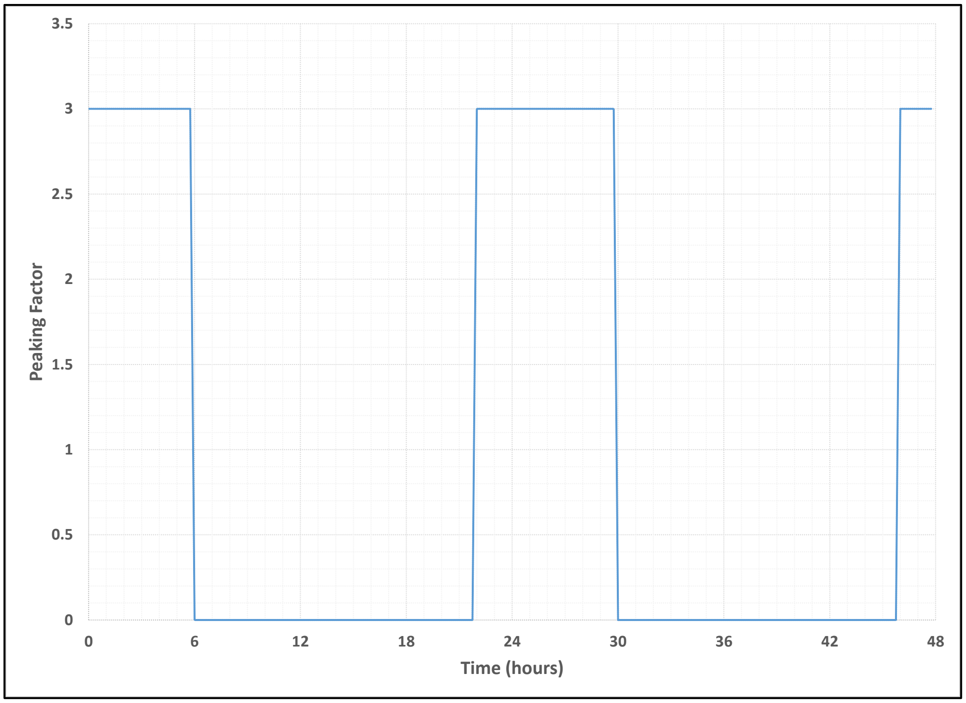

Figure 6 – Irrigation Diurnal

Phase 2 consists of four potential pipeline alignments that are shown in Figure 7 and described below:

- Phase 2A – Located north and east of the Valencia WRP. Analysis includes multiple alignments.

- Phase 2B – Located near the Vista Canyon Water Factory. The service area includes pipelines installed by a local developer (Vista Canyon Development).
- Phase 2C – Located to the south of the existing recycled water system
- Phase 2D – Located near the existing recycled water reservoir.

The following sections describe model setup and summarize the results of the analysis for each Phase 2 pipeline alignment.

3.1 Phase 2A Analysis

3.1.1 Setup

Phase 2A will be served from the booster pump station at the Valencia WRP. The HGL for the existing Valencia WRP booster pump station is too low to accommodate the Phase 2A system. Therefore, a dedicated pump station will be required to serve the proposed Phase 2A system. In addition, Phase 2A had two different alignments (Central Park South and Bouquet Canyon Road) which were analyzed, along with a storage reservoir site.

To analyze the Phase 2A system shown in **Figure 7**, three scenarios were developed: Phase 2A-1, Phase 2A-2, and Phase 2A-3. **Table 2** shows the boundary conditions used for each of these scenarios.

Table 2 – Phase 2A Scenario Summary

Scenario	Maxim Day Demand (gpm)	Description
Phase 2A-1 (Central Park South Alignment with Tank)	826	Phase 2A with the Central Park South Alignment Includes a 1 MG tank within Phase 2A system WRP PS – Design station flow at 2,500 gpm with 490 feet of head
Phase 2A-2 (Central Park South Alignment)	826	Phase 2A with the Central Park South Alignment WRP PS – Design station flow at 2,500 gpm with 490 feet of head
Phase 2A-3 (Bouquet Canyon Road Alignment)	719	Phase 2A with the Bouquet Canyon Road Alignment WRP PS - Design station flow at 2,200 gpm with 450 feet of head

3.1.2 Phase 2A-1 Results and Conclusions

The results for Phase 2A-1 indicate that the maximum observed pressure was 238 psi which occurred at the WRP. The minimum observed pressure at a demand node was 56 psi. Pressures of less than 20 psi were reported at high elevations along the transmission main but did not impact demand nodes. Velocities were below 6 fps using 8-inch to 16-inch diameter pipelines. **Table 3** presents a summary of the pipeline sizes used in this analysis. **Figure 8** shows the minimum pressures and maximum velocities for Phase 2A-1. Additional results are provided in **Attachment B**.

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Figure 7
Proposed Facilities
Phase 2 System

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Figure 8
Minimum Pressures and Maximum Velocity Maximum Day Demands Phase 2A-1 with a Tank Alternative

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Table 3 – Phase 2A-1 Pipeline Summary

Diameter (in)	Length (ft)	Length (miles)
8	8,865	1.68
12	14,915	2.82
16	14,628	2.77
Total	**38,408**	**7.27**

Conclusions

The following conclusions were developed based on the analysis presented above:

- Two constant speed pumps with a design point of 1,250 gpm at 490 feet of head per pump will adequately supply the service area.
- This scenario utilizes only one of the pumps for the majority of the 48-hour MDD scenario.
- The 1 MG storage reservoir provides peak flow allowing the pumps to be operated using a constant speed drive.

3.1.2 Phase 2A-2 Results

The results for Phase 2A-2 indicate that the maximum pressure observed was 190 psi which occurred downstream of the WRP. The minimum pressure observed at a demand node was 55 psi. Pressures of less than 20 psi were reported at high elevations along the transmission main but did not impact demand nodes. Velocities were below 6 fps using 8-inch to 16-inch diameter pipelines. **Table 4** presents a summary of the pipeline sizes used in this analysis. **Figure 9** shows the minimum pressures and maximum velocities for Phase 2A-2. Additional results are provided in **Attachment C**.

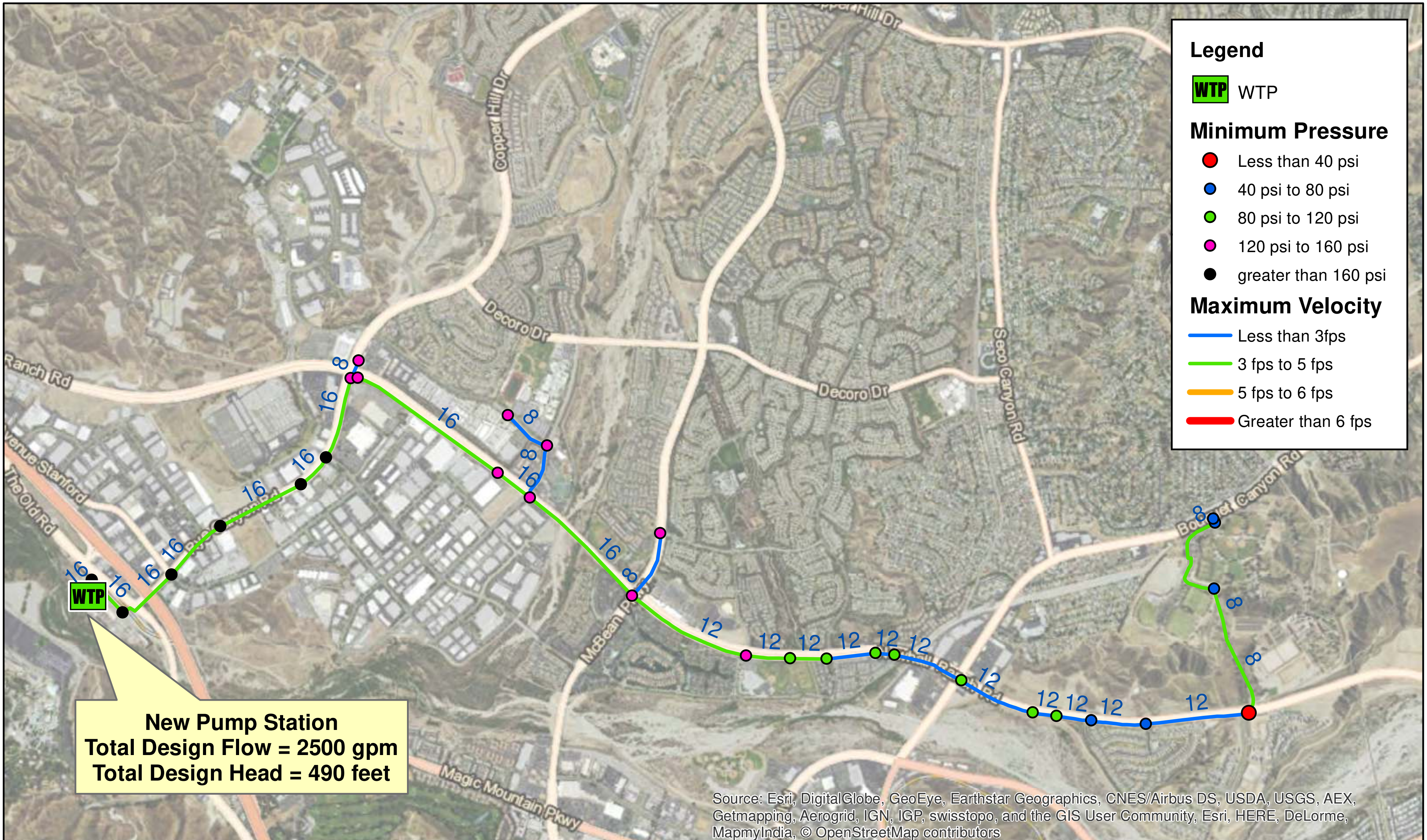
Table 4 – Phase 2A-2 Pipeline Summary

Diameter (in)	Length (ft)	Length (miles)
8	8,865	1.68
12	14,819	2.81
16	14,653	2.78
Total	**38,337**	**7.26**

Conclusions

The following conclusions were developed based on the analysis presented above:

- Two pumps with a design point of 1,250 gpm at 490 feet of head per pump operating with variable frequency drives (VFD) are required to meet the demand fluctuations. The VFD setpoint should be 210 psi.
- With no storage reservoir, the treatment plant and pump station capacity must be adequate to supply peak demands.



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0 1,250 2,500 5,000
Feet
1 inch = 2,500 feet

Figure 9
Minimum Pressures and
Maximum Velocity
Maximum Day Demands
Phase 2A-2

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3.1.2 Phase 2A-3 Results

The results for Phase 2A-3 indicate that the maximum pressure observed was 190 psi located downstream of the WRP. The minimum observed pressure was 65 psi. Velocities were below 6 fps using 8-inch to 16-inch diameter pipelines. **Table 5** presents a summary of the pipeline sizes used in this analysis. **Figure 10** shows the minimum pressures and maximum velocities for Phase 2A-3. Additional results are provided in **Attachment D**.

Table 5 – Phase 2A-1 Pipeline Summary

Diameter (in)	Length (ft)	Length (miles)
8	9,782	1.85
12	7,008	1.33
16	14,628	2.77
Total	**31,417**	**5.95**

Conclusions

The following conclusions were developed based on the analysis presented above:

- Two pumps with a design point of 1,100 gpm at 450 feet of head per pump operating with variable frequency drives are required to meet the demand fluctuations. The VFD setpoint should be 190 psi.
- With no storage reservoir, the treatment plant and pump station capacity must be adequate to supply peak demands.

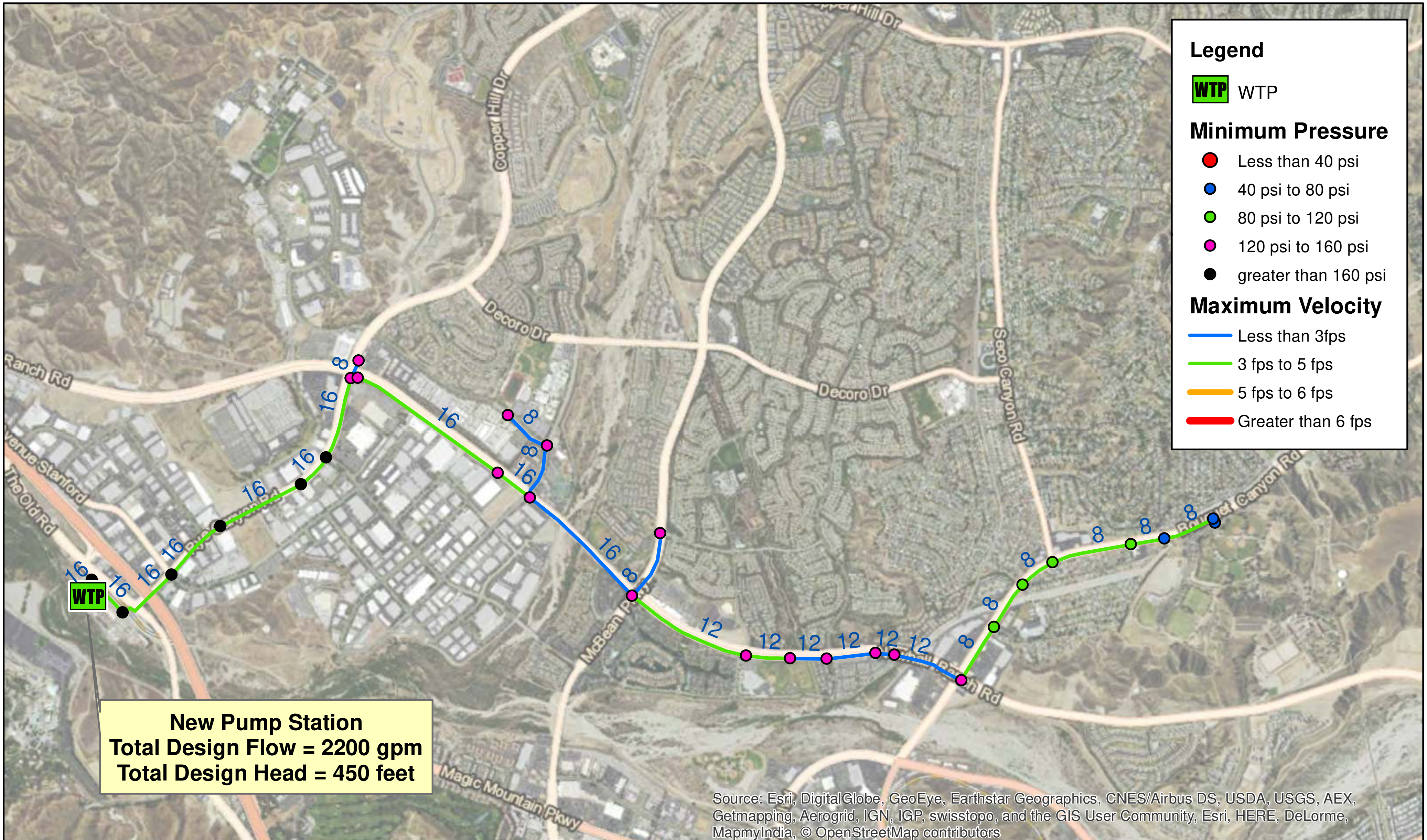
3.1.3 Recommendations

Of the three scenarios analyzed, Scenario 2A-1 provides the most benefits to the system in terms of storage and ease of operation. In addition, the alignment provides service to all potential customers within the phase system.

3.2 Phase 2B Analysis

3.2.1 Setup

Phase 2B will be served from the Vista Canyon Water Factory, which is located at the proposed Vista Canyon Development. The Phase 2B system includes the Vista Canyon development and Santa Clarita Water Division (SCWD) customers to the south of the proposed development. In addition, a 1 MG storage reservoir is located south of the Vista Canyon Water Factory. The total demand for this scenario is 424 gpm; with 234 gpm within SCWD's service area and 190 gpm within the proposed Vista Canyon development.



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Feet
1 inch = 2,500 feet

Figure 10
Minimum Pressures and
Maximum Velocity
Maximum Day Demands
Phase 2A-3 Alternative Alignment

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3.2.2 Phase 2B Results and Conclusions

The results for Phase 2B indicate that the maximum observed pressure was 212 psi which occurred downstream of the WRP. The minimum observed pressure was 44.4 psi which occurred at Junction NOD3348. Velocities were below 6 fps in the 6-inch to 12-inch diameter transmission main. The total length of the pipeline used in Phase 2B is 23,200 feet, or 4.4 miles. **Table 6** presents a summary of the proposed pipeline sizes for Phase 2B used in this analysis. **Figure 11** shows the minimum pressures and maximum velocities for Phase 2B. Additional results are provided in **Attachment E**.

Table 6 – Phase 2B Pipeline Summary

Location	Diameter (in)	Length (ft)	Length (miles)
Pipelines South of Railroad Tracks	6	6,100	1.16
	12	6,600	1.25
	Subtotal	**12,700**	**2.41**
Pipelines North of Railroad Tracks	8	10,500	1.99
Total		**23,200**	**4.40**

Conclusions

The following conclusions were developed based on the analysis presented above:

- Three constant speed pumps with a design point of 136 gpm at 348 feet of head per pump will adequately supply the service area.
- This scenario utilizes only one of the pumps for the majority of the 48-hour MDD scenario.
- The 1 MG storage reservoir provides peak flow allowing the pumps to be operated using a constant speed drive.

3.3 Phase 2C Analysis

Phase 2C and Phase 2D were analyzed concurrently in the same model scenarios because it is anticipated that both projects will be implemented at approximately the same time in the near future. This section describes the Phase 2C system.

3.3.1 Setup

Phase 2C will be served from the Valencia WRP. The total demand for the project is 1,754 gpm. In addition to the proposed alignments, it was assumed that the 12-inch pipeline in The Old Road along the bridge was replaced with a 24-inch pipeline due to observed velocities as high as 18 fps. The Phase 2C alignments are shown on **Figure 7.**

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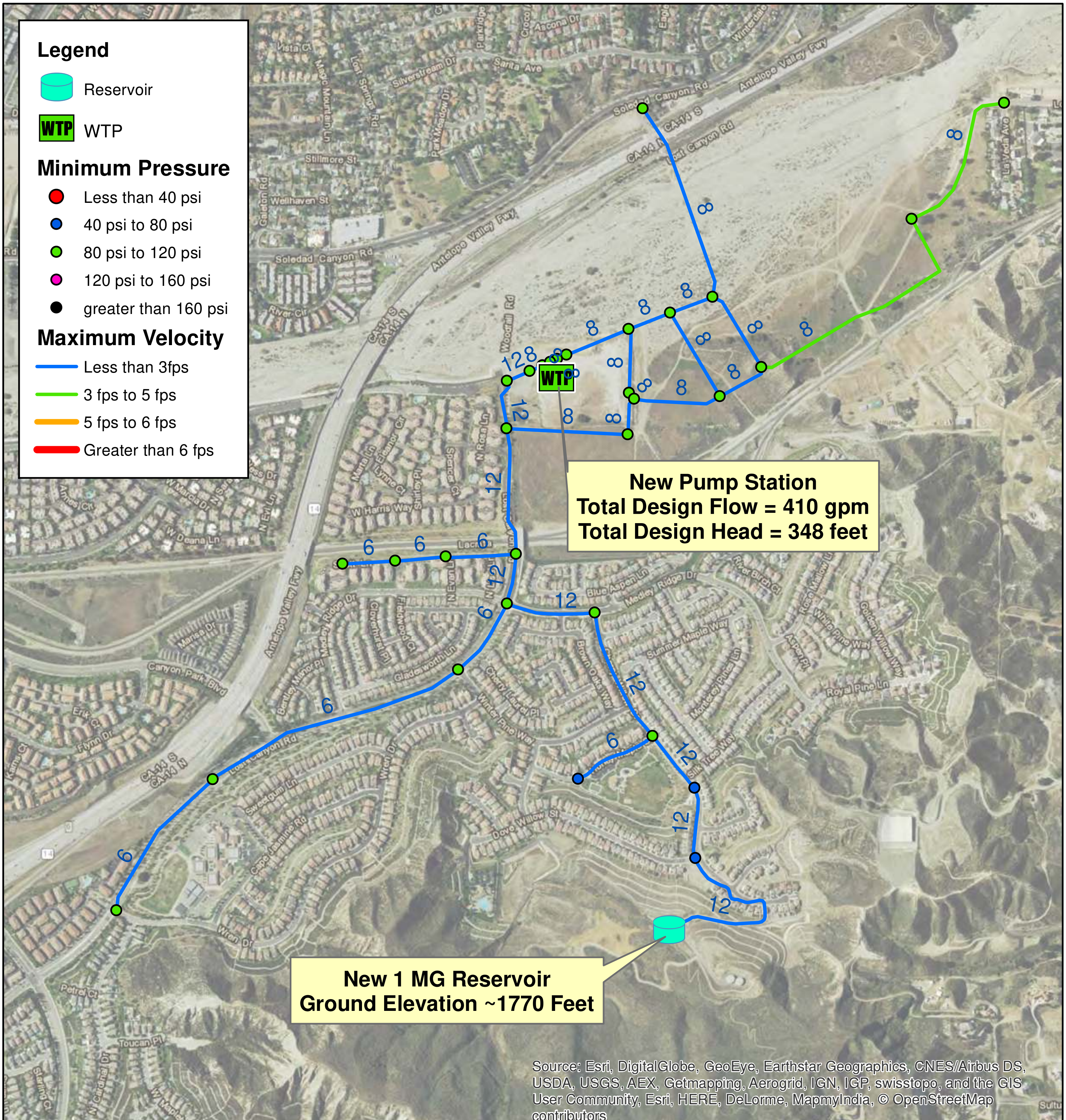

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0 400 800 1,600 Feet

1 inch = 1,000 feet

Figure 11
Minimum Pressures and Maximum Velocity Maximum Day Demands Phase 2 B Results

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3.3.2 Results

Results for Phases 2C indicate that the maximum observed pressure was 212 psi located downstream of the WRP. The minimum observed pressure was 13 psi which occurred at a high elevation along the existing 20-inch diameter pipeline in Valencia Boulevard (Junction NOD3348). Velocities were below 6 fps by utilizing 12-inch to 24-inch diameter pipelines for the proposed phases. However, the existing system velocities frequently violated the 6 fps criteria, with velocities ranging from 0.2 fps to 13 fps. **Table 7** presents a summary of the proposed pipeline sizes for Phase 2C used in this analysis. **Figure 12** shows the minimum pressures and maximum velocities for Phases 2C. Additional results are provided in **Attachment F**.

Table 7 – Phase 2C Pipeline Summary

Phase	Diameter (in)	Length (ft)	Length (miles)
Phase 2C	8	8,710	1.65
	12	5,470	1.04
	16	7,380	1.40
	20	5,250	0.99
	24	4,130	0.78
Total		**30,940**	**5.86**

Conclusions

The following conclusions are based on the Phase 2C and 2D analysis presented above:

- The 12-inch diameter pipeline in The Old Road must be replaced with a 24-inch diameter pipeline to supply enough water to the proposed pipelines. Without replacement, the 12-inch pipeline will see velocities as high as 18 fps and 35 feet of headloss within this scenario.
- There is insufficient storage at the existing reservoir site so the peak demand is met by increased flow from the WRP. Pump station flows were as high as 7,000 gpm – requiring a total of 4 pumps operating (assuming each pump is the same size as the existing pumps with a design point of 2,000 gpm at 380 feet of head). This also assumes that all pumps are operating on their original manufacturer's curve, which may require the existing pumps to be rehabilitated.
- A low pressure of 13 psi was observed at a demand node, which may not be acceptable to this customer.
- Multiple pipelines in the existing system have velocities above 6 fps, with velocities ranging from 0.2 fps to 13 fps.

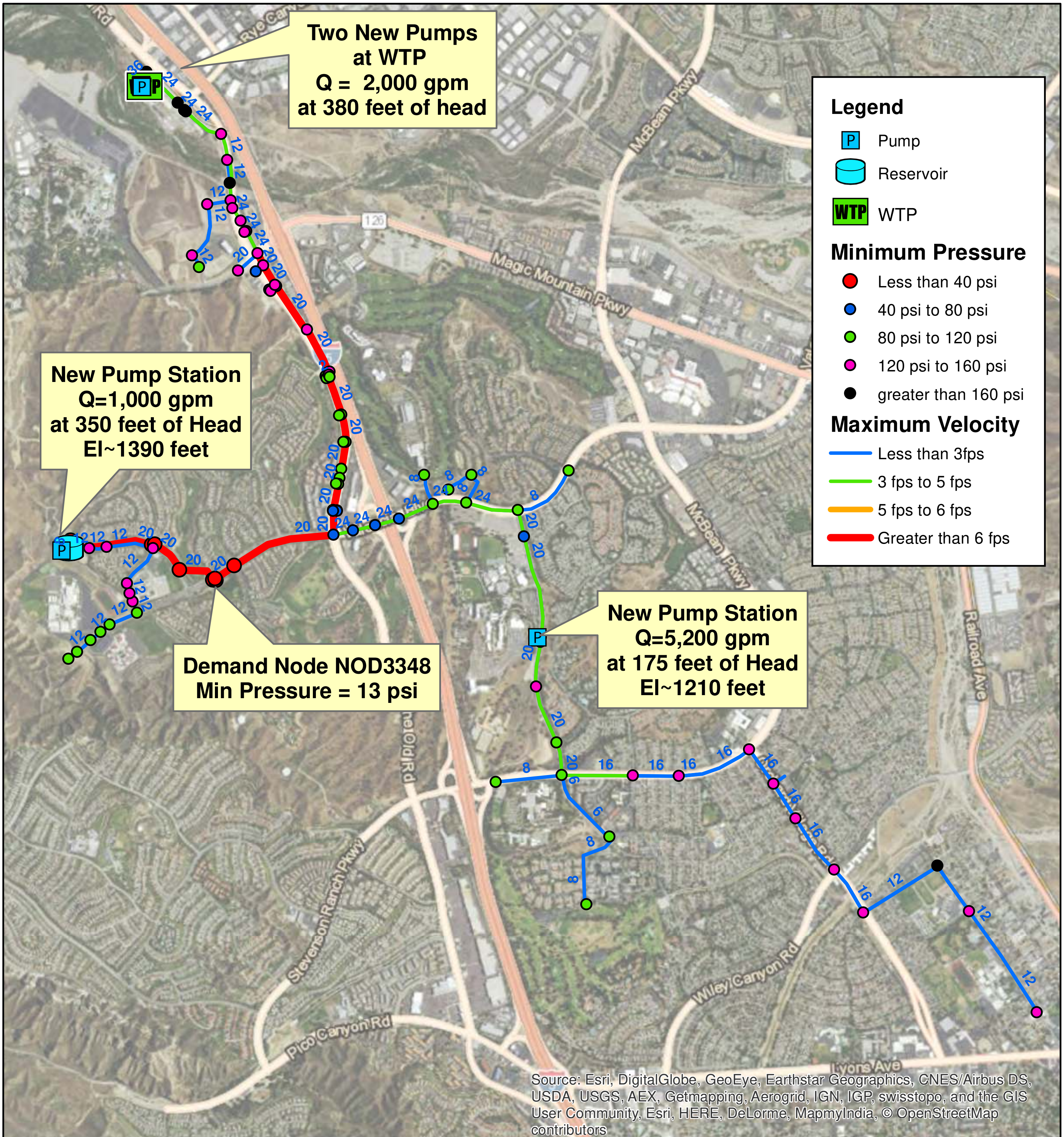

Figure 12
Minimum Pressures and Maximum Velocity Maximum Day Demands Phase 2C and 2D

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3.4 Phase 2D Analysis

Phase 2C and Phase 2D were analyzed concurrently in the same model scenarios because it is anticipated that both projects will be implemented at approximately the same time in the near future. This section describes the Phase 2D system.

3.4.1 Setup

Phase 2C and 2D will be served from the Valencia WRP. The total demand for this project is 333 gpm. In addition to the proposed alignments, it was assumed that the 12-inch pipeline in The Old Road along the bridge was replaced with a 24-inch pipeline due to observed velocities as high as 18 fps. The Phase 2D alignment is shown on **Figure 7** above.

3.4.2 Results

Results for Phase 2D indicate that the maximum observed pressure was 212 psi located downstream of the WRP. The minimum observed pressure was 13 psi which occurred at a high elevation along the existing 20-inch diameter pipeline in Valencia Boulevard (Junction NOD3348). Velocities were below 6 fps by utilizing 12-inch diameter pipelines for the proposed Phase 2D. However, the existing system velocities frequently violated the 6 fps criteria, with velocities ranging from 0.2 fps to 13 fps. The proposed pipeline utilized for the Phase 2D analysis included approximately 5,200 feet of 12-inch pipeline. **Figure 12** above shows the minimum pressures and maximum velocities for Phase 2D. Additional results are provided in **Attachment F**.

Conclusions

There are no conclusions in addition to those identified within section 3.3 (Phase 2C) above.

4.0 Phase 2A with Indirect Potable Reuse Flows

The indirect potable reuse (IPR) scenarios assume minimum day demand for irrigation customers with all excess water production being used for groundwater recharge. Two groundwater recharge spreading basins (basins) were analyzed: Basin 1 and Basin 3. Both basins use the proposed Phase 2A system plus additional pipelines to supply water to the basins. A total of 6,903 gpm was used for the following scenarios: 167 gpm for the minimum day demand in the Phase 2A system and 6,736 gpm is used at the spreading basin. The following scenarios were analyzed.

- IPR Scenario 1 – Phase 2A-1 with Basin 1
- IPR Scenario 2 – Phase 2A-2 with Basin 1
- IPR Scenario 3 – Phase 2A-1 with Basin 1 and no additional pump station
- IPR Scenario 4 – Phase 2A-1 with Basin 3

Figure 13 shows the proposed pipelines serving the two basins.

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0 2,000 4,000 8,000 Feet
1 inch = 5,000 feet

Figure 13
Proposed Facilities
Phase 2 System plus
IPR Basin Pipelines

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4.1 IPR Scenario 1 Results and Conclusions

The results for IPR Scenario 1 indicate that the maximum observed pressure was 209 psi which occurred at the WRP. The minimum observed pressure was 10 psi which occurred at Basin 1 and is acceptable. Velocities were below 6 fps using a 24-inch diameter pipeline. **Table 8** presents a summary of the pipeline sizes used in this analysis. **Figure 14** shows the minimum pressures and maximum velocities for IPR Scenario 1. Additional results are provided in **Attachment G**.

Table 8 – IPR Scenario 1 Pipeline Summary

Phase	Diameter (in)	Length (ft)	Length (miles)
Phase 2A	8	7,432	1.41
	12	757	0.14
	24	30,219	5.72
	Subtotal	**38,408**	**7.27**
IPR Basin 1	24	17,933	3.40
Total		**56,341**	**10.67**

Conclusions

The following conclusion are based on the analysis presented above:

- A 24-inch diameter pipeline and pump station is able to supply adequate flow and pressure to serve IPR Basin 1.
- Two pumps with a design point of 3,500 gpm at 490 feet of head per pump are required at the Valencia WRP.
- Two pumps with a design point of 3,500 gpm at 30 feet of head per pump are required on Newhall Ranch Road.

4.2 IPR Scenario 2 Results and Conclusions

The results for IPR Scenario 2 indicate that the maximum observed pressure was 236 psi which occurred at the WRP. The minimum observed pressure was 20.3 psi which occurred at Basin 1 and is acceptable. Velocities were below 6 fps using a 24-inch diameter pipeline. **Table 9** presents a summary of the pipeline sizes used in this analysis. **Figure 15** shows the minimum pressures and maximum velocities for IPR Scenario 2. Additional results are provided in **Attachment H**.

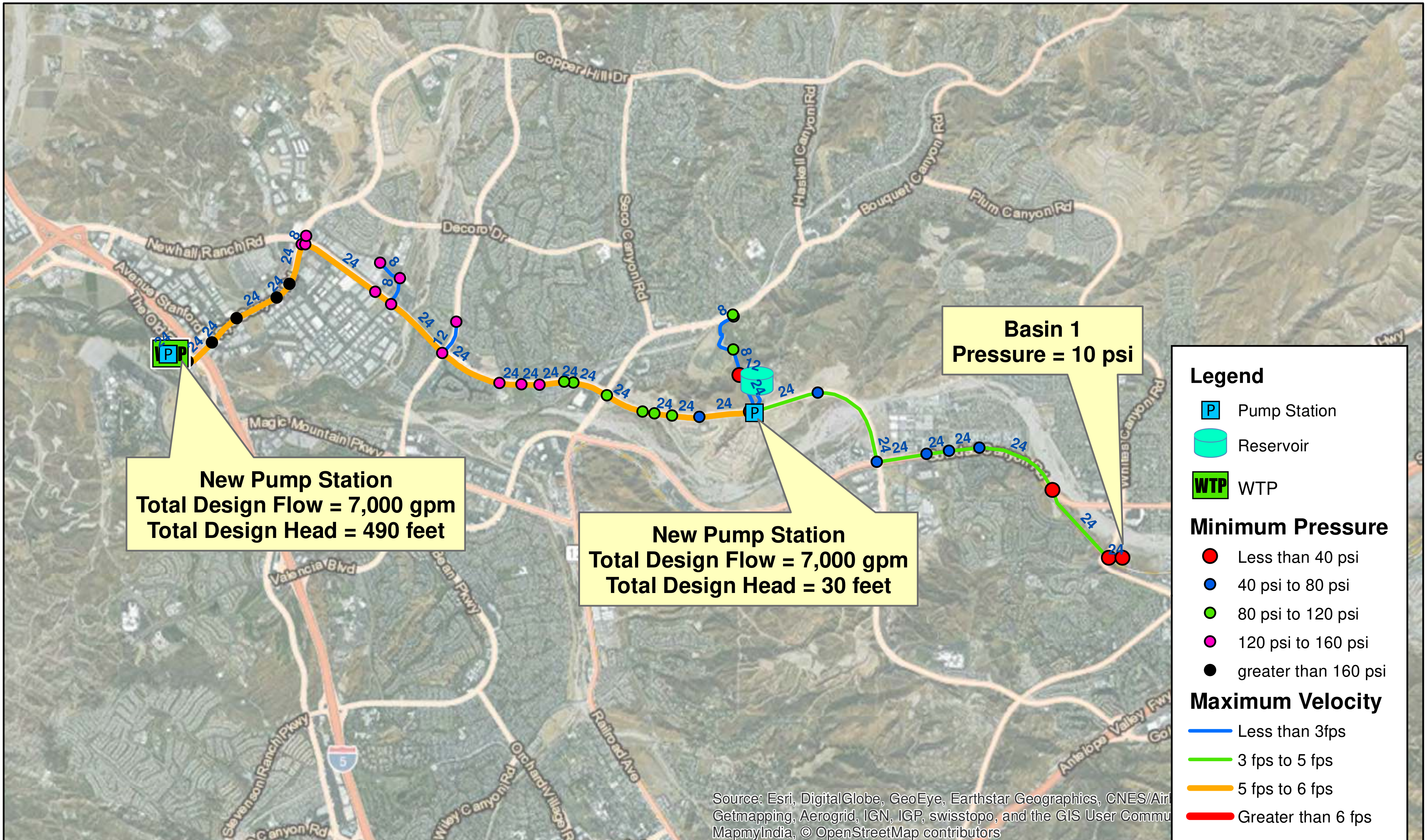

Figure 14
Minimum Pressures and Maximum Velocity
Minimum Day Demands
Phase 2A with a Tank Alternative
IPR Basin 1 with New Pump Station

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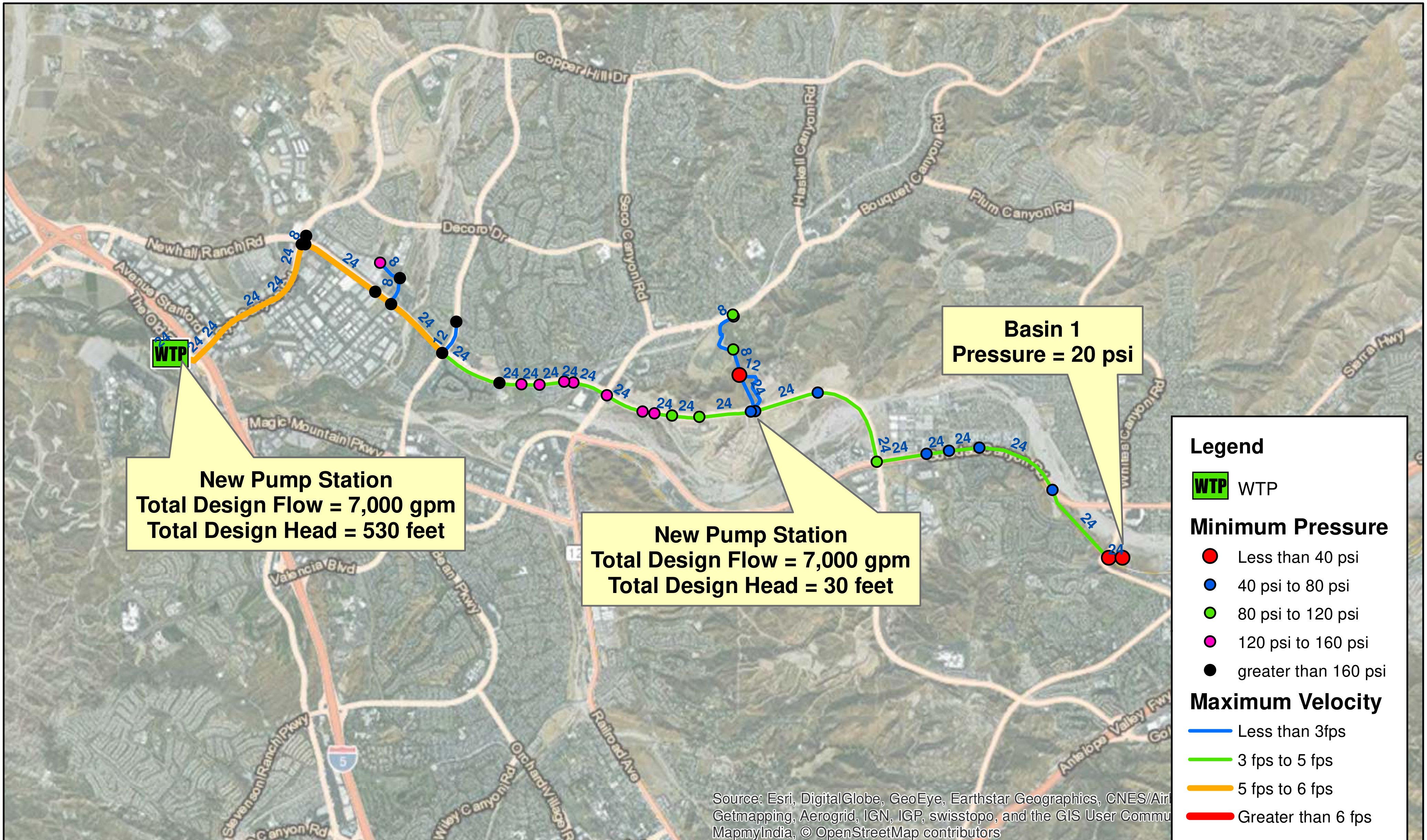

Figure 15
Minimum Pressures and Maximum Velocity
Minimum Day Demands
Phase 2A without a Tank Alternative
IPR Basin 1 Supplied From WTP

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Table 9 – IPR Scenario 2 Pipeline Summary

Phase	Diameter (in)	Length (ft)	Length (miles)
Phase 2A	8	7,432	1.41
	12	757	0.14
	24	30,219	5.72
	Subtotal	**38,408**	**7.27**
IPR Basin 1	24	17,933	3.40
Total		**56,341**	**10.67**

Conclusions

The following conclusions are based on the analysis presented above:

- A 24-inch diameter pipeline and pump station is able to supply adequate flow and pressure to serve IPR Basin 1.
- Two pumps with a design point of 3,500 gpm at 530 feet of head per pump are required at the Valencia WRP.
- Two pumps with a design point of 3,500 gpm at 30 feet of head per pump are required on Newhall Ranch Road.

4.3 IPR Scenario 3 Results and Conclusions

The results for IPR Scenario 3 indicate that the maximum observed pressure was 209 psi which occurred at the WRP. The minimum observed pressure was 11 psi which occurred at Basin 1. Velocities were below 6 fps using 24-inch to 30-inch diameter pipelines. **Table 10** presents a summary of the pipeline sizes used in this analysis. **Figure 16** shows the minimum pressures and maximum velocities for IPR Scenario 3. Additional results are provided in **Attachment I**.

Table 10 – IPR Scenario 3 Pipeline Summary

Phase	Diameter (in)	Length (ft)	Length (miles)
Phase 2A	8	6,089	1.15
	12	2,100	0.40
	24	30,219	5.72
	Subtotal	**38,408**	**7.27**
IPR Basin 1	24	4,278	0.81
	30	13,655	2.59
	Subtotal	**17,933**	**3.40**
Total		**56,341**	**10.67**

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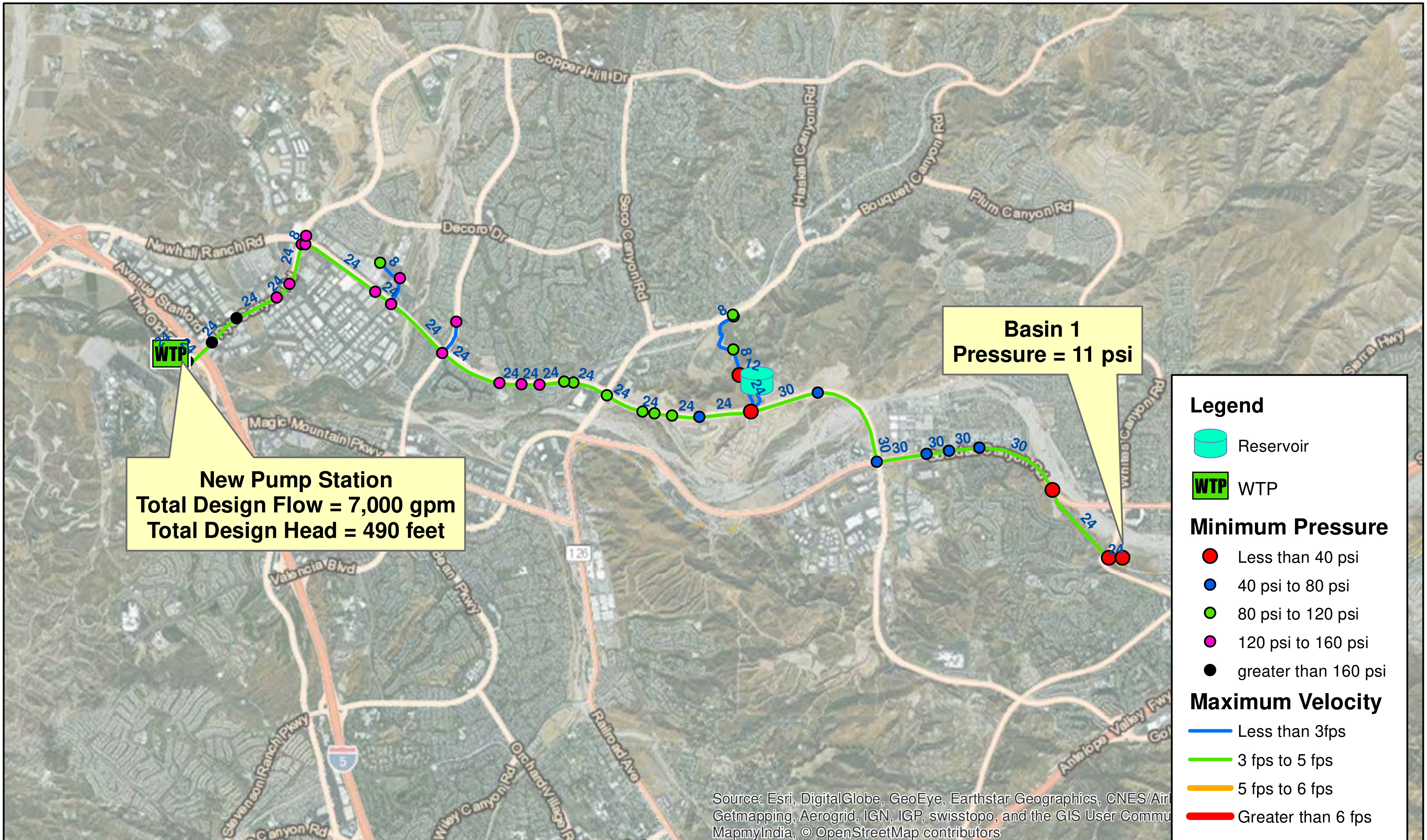

N
W E
S

0 2,500 5,000 10,000
Feet
1 inch = 5,000 feet

Figure 16
Minimum Pressures and Maximum Velocity
Minimum Day Demands
Phase 2A with a Tank Alternative
IPR Basin 1 with No Pump Station

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Conclusions

The following conclusion are based on the analysis presented above:

- A 30-inch diameter pipeline is able to supply adequate flow and pressure to serve IPR Basin 3.
- Two pumps with a design point of 3,500 gpm at 490 feet of head per pump are required at the Valencia WRP.

4.4 IPR Scenario 4 Results and Conclusions

The results for IPR Scenario 4 indicate that the maximum observed pressure was 209 psi which occurred at the WRP. The minimum observed pressure was 10 psi which occurred at Basin 3 and is acceptable. Velocities were below 6 fps using a 24-inch diameter pipeline. **Table 11** presents a summary of the pipeline sizes used in this analysis. **Figure 17** shows the minimum pressures and maximum velocities for IPR Scenario 4. Additional results are provided in **Attachment J**.

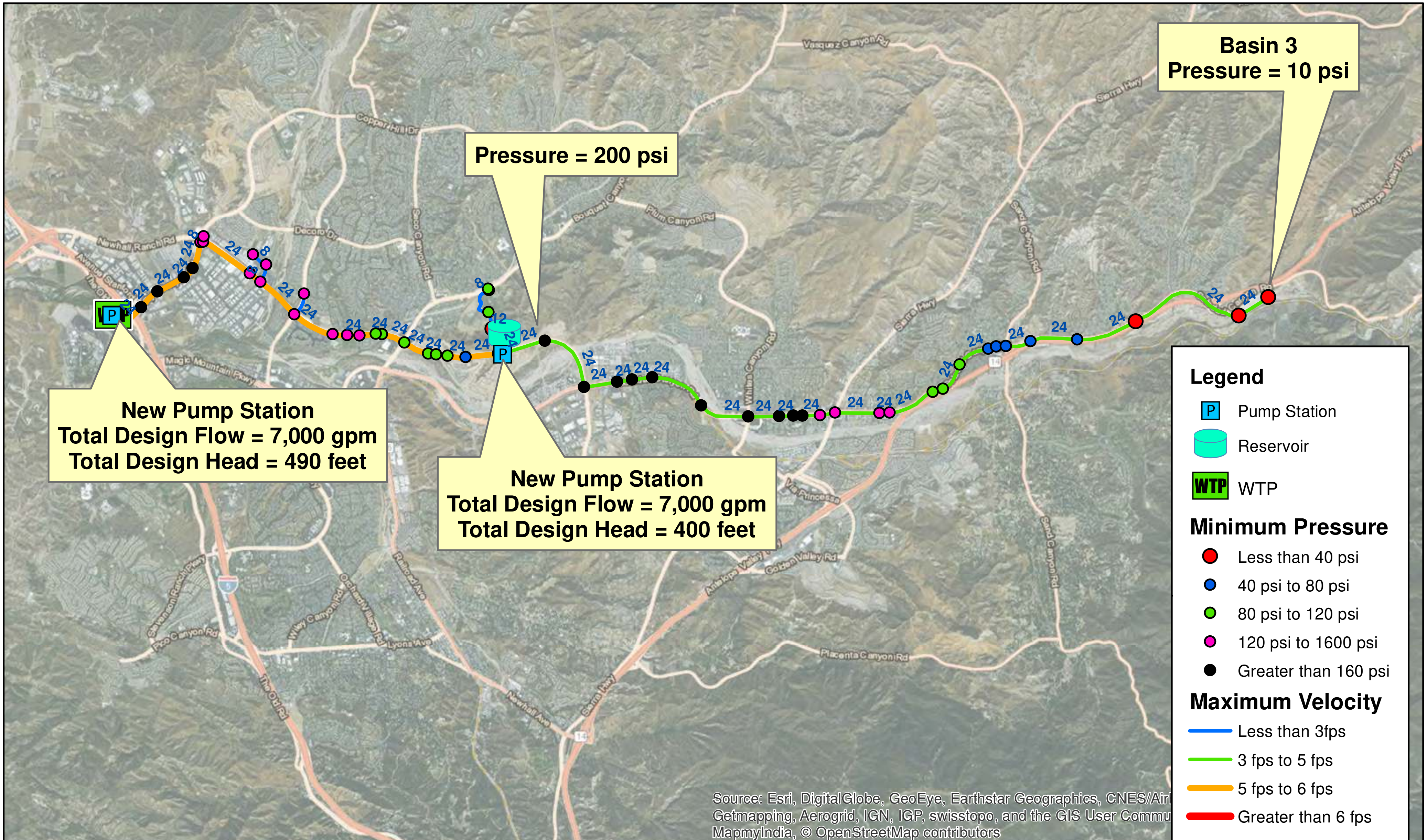
Table 11 – IPR Scenario 4 Pipeline Summary

Phase	Diameter (in)	Length (ft)	Length (miles)
Phase 2A	8	7,432	1.41
	12	757	0.14
	24	30,219	5.72
	Subtotal	**38,408**	**7.27**
IPR Basin 1	24	49,457	9.37
Total		**87,865**	**16.64**

Conclusions

The following conclusions are based on the analysis presented above:

- A 24-inch diameter pipeline and pump station is able to supply adequate flow and pressure to serve IPR Basin 3.



N
W E
S

0 3,750 7,500 15,000
Feet
1 inch = 7,500 feet

Figure 17
Minimum Pressures and Maximum Velocity
Minimum Day Demands
Phase 2A with a Tank Alternative
IPR Basin 3

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5.0 Future Expansion Analysis

The future expansion analysis starts with the Phase 2 system and expands the service area using multiple alignments. Alignments A through D tie into the Phase 2C system. Alignments E through H tie into the Phase 2A system. The alignments may be implemented independently. For the purposes of this analysis, the alignments are organized in two groups. **Figure 18** shows the proposed alignments.

5.1 Future Expansion North

5.1.1 Future Expansion North Setup

The future expansion north area will be served from the Valencia WRP and builds on the Phase 2A system. Alignments E-H were added to the Phase 2A system. The total demand for this scenario is 2,700 gpm: 826 gpm for Phase 2A and 1,874 gpm for Alignments E - H. **Table 12** presents the demands for this scenario.

Table 12 – Future Expansion North Demands

Pipeline Alignment		Demand (gpm)	Demand (mgd)
Phase 2A		826	1.19
Future Expansion North	Alignment E	566	0.82
	Alignment F	279	0.40
	Alignment G	444	0.64
	Alignment H	585	0.84
Total		**2,700**	**3.89**

As part of this analysis, a feasibility of a tank located at the end of Alignment G was analyzed and discussed further below.

5.1.2 Future Expansion North Results and Conclusions

The results for the future expansion north with a tank located at the end of Alignment G indicate that the elevations in the area are not sufficient to maintain a tank and adequate pressure at service nodes. While utilizing a tank with a bottom elevation of 1520 feet, the closest service node was experiencing a pressure as low as 8 psi. This was considered insufficient pressure and no further analysis of a tank along Alignment G was considered.

The results for the future expansion north area indicate that the maximum observed pressure was 238 psi which occurred at the WRP. The minimum observed pressure was 40 psi which occurred at the end of Alignment H. Velocities were below 6 fps using 8-inch to 24-inch diameter pipelines. Phase 2A pipelines were upsized to accommodate the demands for this scenario.

Table 13 presents a summary of the pipeline sizes used in this analysis. **Figure 19** shows the minimum pressures and maximum velocities for Phase 2A-1. Additional results are provided in **Attachment K**.

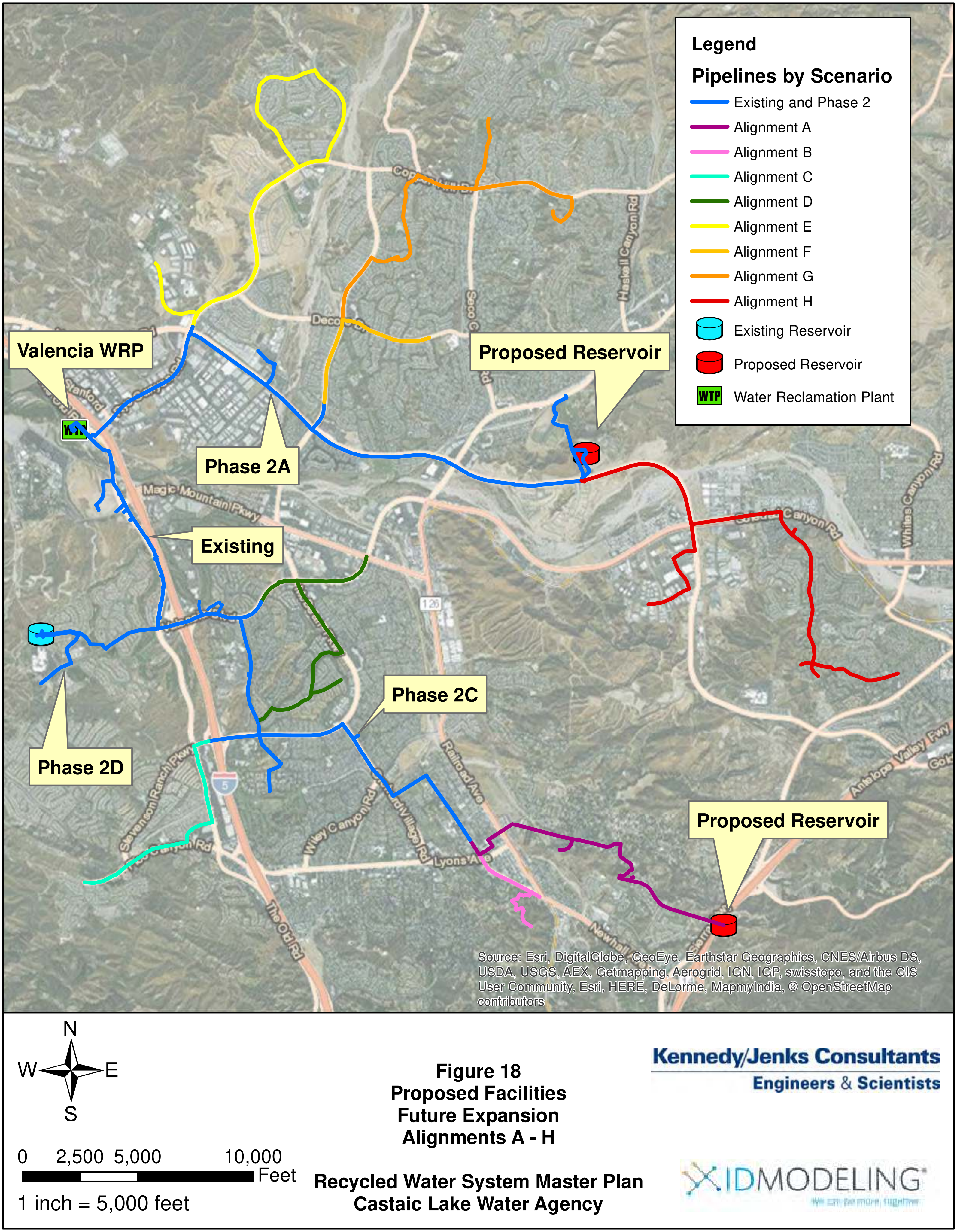

Figure 18
Proposed Facilities
Future Expansion
Alignments A - H

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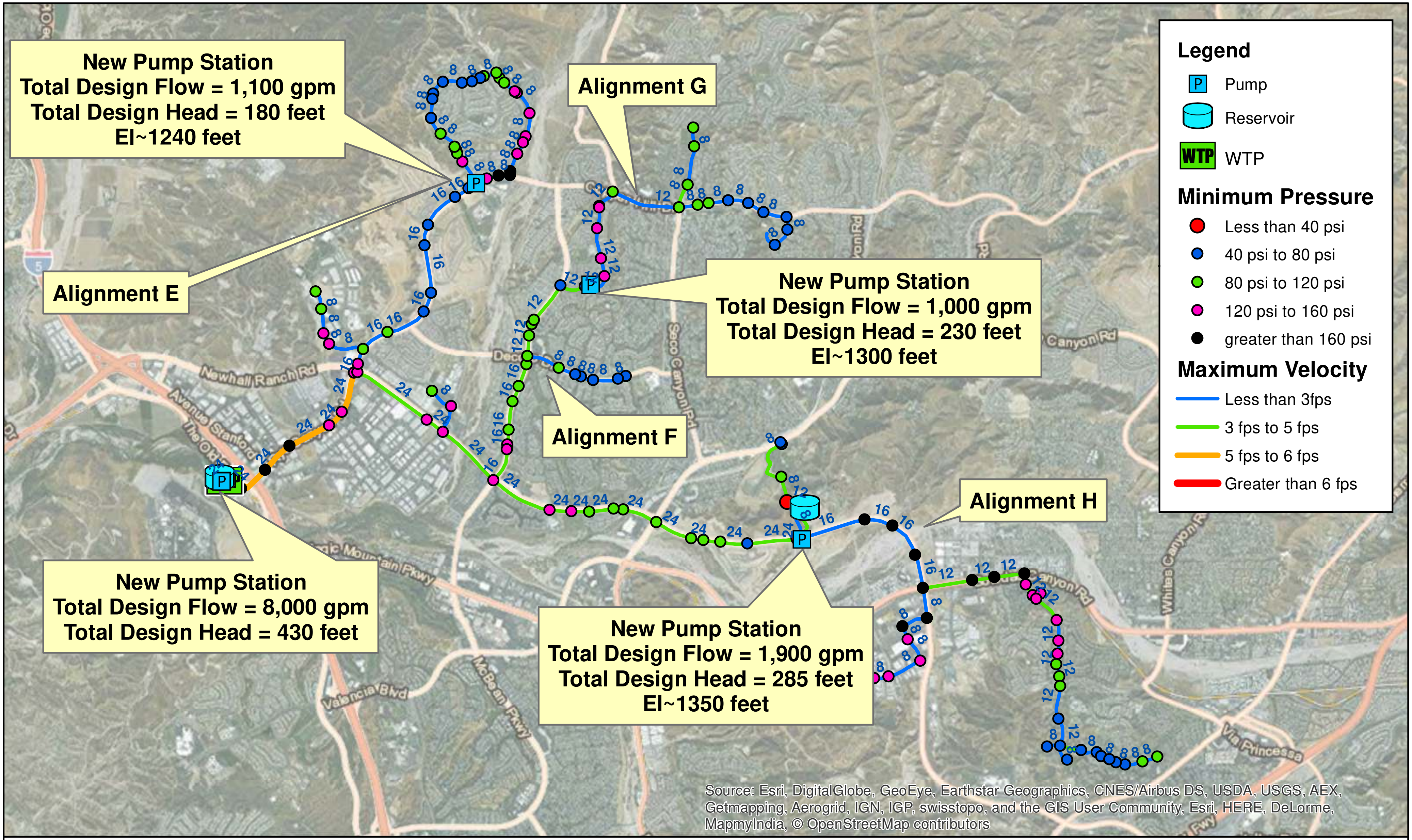
Table 13 – Future Expansion North Pipeline Summary

Phase	Diameter (in)	Length (ft)	Length (miles)
Phase 2A	8	7,153	1.35
	12	870	0.16
	16	1,712	0.32
	24	28,602	5.42
	Subtotal	**38,337**	**7.26**
Alignment 3E	8	16,073	3.04
	12	332	0.06
	16	9,233	1.75
	Subtotal	**25,639**	**4.86**
Alignment 3F	8	3,979	0.75
	16	3,692	0.70
	Subtotal	**7,671**	**1.45**
Alignment 3G	8	9,333	1.77
	12	11,616	2.20
	Subtotal	**20,949**	**3.97**
Alignment 3H	8	10,985	2.08
	12	11,084	2.10
	16	6,500	1.23
	24	205	0.04
	Subtotal	**28,774**	**5.45**
Total		**121,369**	**22.99**

Conclusions

The following conclusions are based on the analysis presented above:

- Two constant speed pumps with a design point of 4,000 gpm at 430 feet of head per pump will adequately supply the service area.
- Three additional pump stations must be added to serve the all proposed alignments:
  - A pump station to serve Alignment E with a design point of 1,100 gpm at 180 feet of head.
  - A pump station to serve Alignment G with a design point of 1,000 gpm at 130 feet of head.
  - A pump station to serve Alignment H with a design point of 1,900 gpm at 185 feet of head.
- There is insufficient storage to serve all demands so the peak flow is served from the WRP.



N
W E
S

0 2,500 5,000 10,000
Feet
1 inch = 5,000 feet

Figure 19
Minimum Pressures and
Maximum Velocity
Maximum Day Demands
Phase 2A with Alignments E-H

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5.2 Future Expansion South

5.2.1 Future Expansion South Setup

Future expansion south will be served from the Valencia WRP and builds upon the Phase 2C system. Phase 3 alignments A - D were added to the Phase 2C system. The total demand for this scenario is 4,023 gpm: 619 gpm within the existing system, 1,754 gpm within the Phase 2C service area, 333 gpm within the Phase 2D service area, and 1,317 within the future expansion north service area. **Table 14** presents the demands for Scenario 3A.

Table 14 – Future Expansion North Pipeline Summary

Pipeline Alignment		Demand (gpm)	Demand (mgd)
Existing		619	0.89
Phase 2C		1754	2.53
Phase 2D		333	0.48
Future Expansion North	Alignment A	581	0.84
	Alignment B	158	0.23
	Alignment C	125	0.18
	Alignment D	453	0.65
	Subtotal	**1317**	**1.90**
Total		**4023**	**5.79**

In addition to the proposed alignments, it was assumed that the 12-inch pipeline in The Old Road along the bridge was upsized to 24-inch (deficiency identified in Section 3.3). The proposed alignments are shown in **Figure 18.**

5.2.2 Results and Conclusions

The results for the future expansion south area indicate that the maximum observed pressure observed pressure was 212 psi which occurred downstream of the WRP. The minimum observed pressure was 15.4 psi which occurred within the existing system on Valencia Boulevard. Velocities were maintained below 6 fps by utilizing 8-inch to 24-inch diameter pipelines for the proposed alignments. Note that it is anticipated that the Phase 2C system will be implemented in the next several years, likely prior to a decision to implement one or more of Alignments A – D. Hence, the Phase 2C pipes were not upsized to meet the maximum velocity criteria for this analysis. **Table 15** presents a summary of the proposed pipeline sizes used in this analysis. **Figure 20** shows the minimum pressures and maximum velocities for Phases 2C and 2D. Additional results are provided in **Attachment L**.

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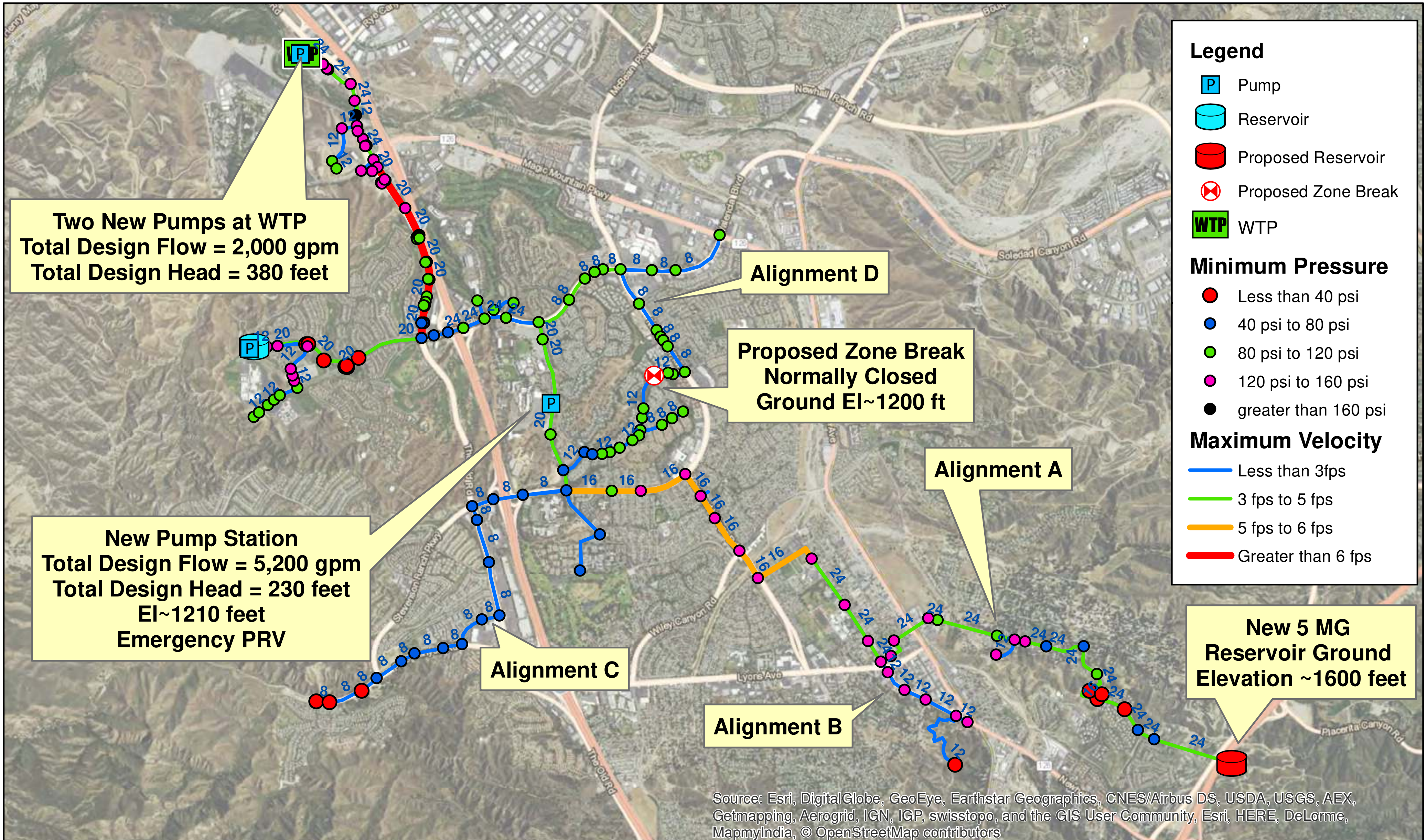

N
W E
S

0 2,000 4,000 8,000
Feet
1 inch = 4,000 feet

Figure 20
Minimum Pressures and
Maximum Velocity
Maximum Day Demands
Phase 2C with Alignments A-D

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Table 15 – Future Expansion South Pipeline Summary

<table>
<tr><th colspan="2">Pipeline Alignment</th><th>Diameter</th><th>Length (ft)</th><th>Length (miles)</th></tr>
<tr><td colspan="2" rowspan="5">Phase 2C</td><td>8</td><td>8710</td><td>1.65</td></tr>
<tr><td>16</td><td>12850</td><td>2.43</td></tr>
<tr><td>20</td><td>5,250</td><td>0.99</td></tr>
<tr><td>24</td><td>4,210</td><td>0.80</td></tr>
<tr><td>Subtotal</td><td>31,020</td><td>5.88</td></tr>
<tr><td colspan="2">Phase 2D</td><td>12</td><td>5,190</td><td>0.98</td></tr>
<tr><td rowspan="9">Future Expansion South</td><td rowspan="4">Alignment A</td><td>12</td><td>820</td><td>0.16</td></tr>
<tr><td>16</td><td>220</td><td>0.04</td></tr>
<tr><td>24</td><td>15,120</td><td>2.86</td></tr>
<tr><td>Subtotal</td><td>16,160</td><td>3.06</td></tr>
<tr><td>Alignment B</td><td>12</td><td>6,190</td><td>1.17</td></tr>
<tr><td>Alignment C</td><td>8</td><td>12,860</td><td>2.44</td></tr>
<tr><td rowspan="3">Alignment D</td><td>8</td><td>11,920</td><td>2.26</td></tr>
<tr><td>12</td><td>5,730</td><td>1.09</td></tr>
<tr><td>Subtotal</td><td>17,650</td><td>3.34</td></tr>
<tr><td colspan="3">Total</td><td>89,070</td><td>16.87</td></tr>
</table>

Conclusions

The following conclusions are based on the analysis presented above:

- Approximately 1,000 feet of 12-inch pipeline in The Old Road must be replaced with a 24-inch pipeline to supply enough water to the new proposed pipelines. Without replacement the 12-inch pipeline will see velocities as high as 18 fps.
- Two additional pumps of the same size as the existing pumps at the Valencia WRP (estimated design point of 2,000 gpm at 380 feet of head) will adequately supply the service area. The manufacturers curve was assumed for all pumps which may necessitate rehabilitation of the existing pumps.
- The Phase 2C pump station will need to be upsized to a design flow of 5,200 gpm at 230 feet of head.
- A new 5 MG storage reservoir at the end of Alignment A is required to serve the entire Phase 2C and future expansion south demand. This 5 MG reservoir serves the entire demand located on the discharge side of the proposed Phase 2C pump station to relieve peak flows at the Valencia

WRP. Without this reservoir, peak hour flows must be provided by the Valencia WRP and the existing recycled water reservoir; which is not possible with the current pipeline sizes, existing reservoir volume, and existing pump station design points. The following controls are recommended for the proposed facilities:

- The Phase 2C pump station should be shut down during irrigation times to ease the peak flow observed at the WRP.
- An emergency pressure reducing station should be constructed to transfer water from the 5MG reservoir back to the existing system. This allows the 5MG reservoir to be utilized as storage for the entire system. This valve should be located at the proposed Phase 2C pump station.

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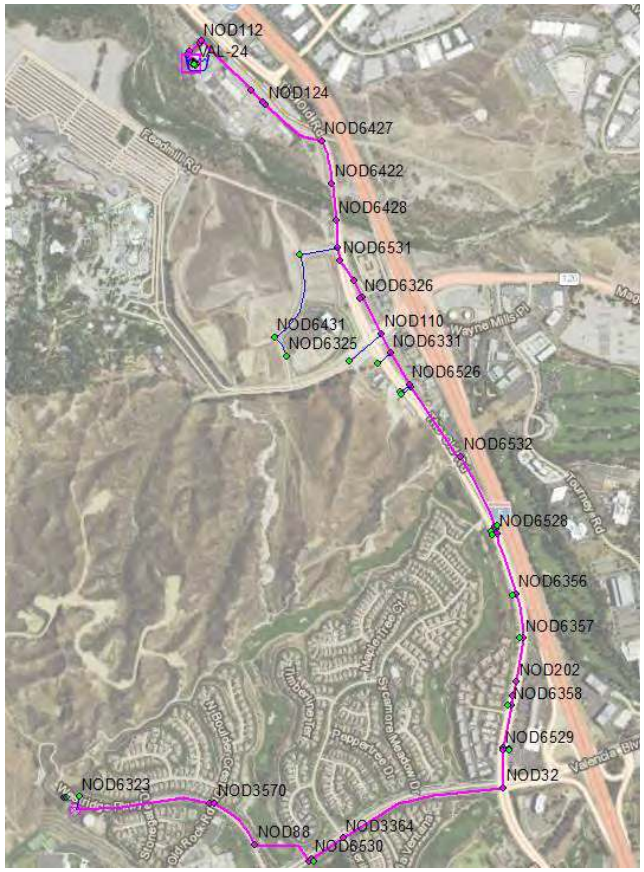
Attachment A - Existing System Results



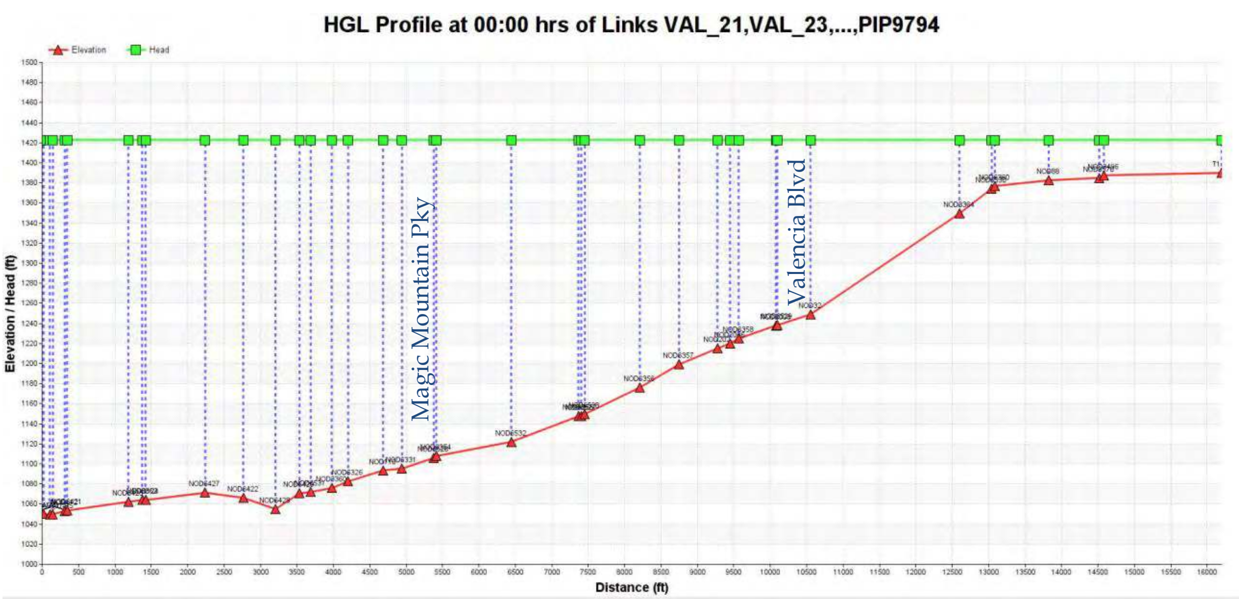


Figure A1 – HGL Profile 1

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Attachment B - Phase 2A-1 Results

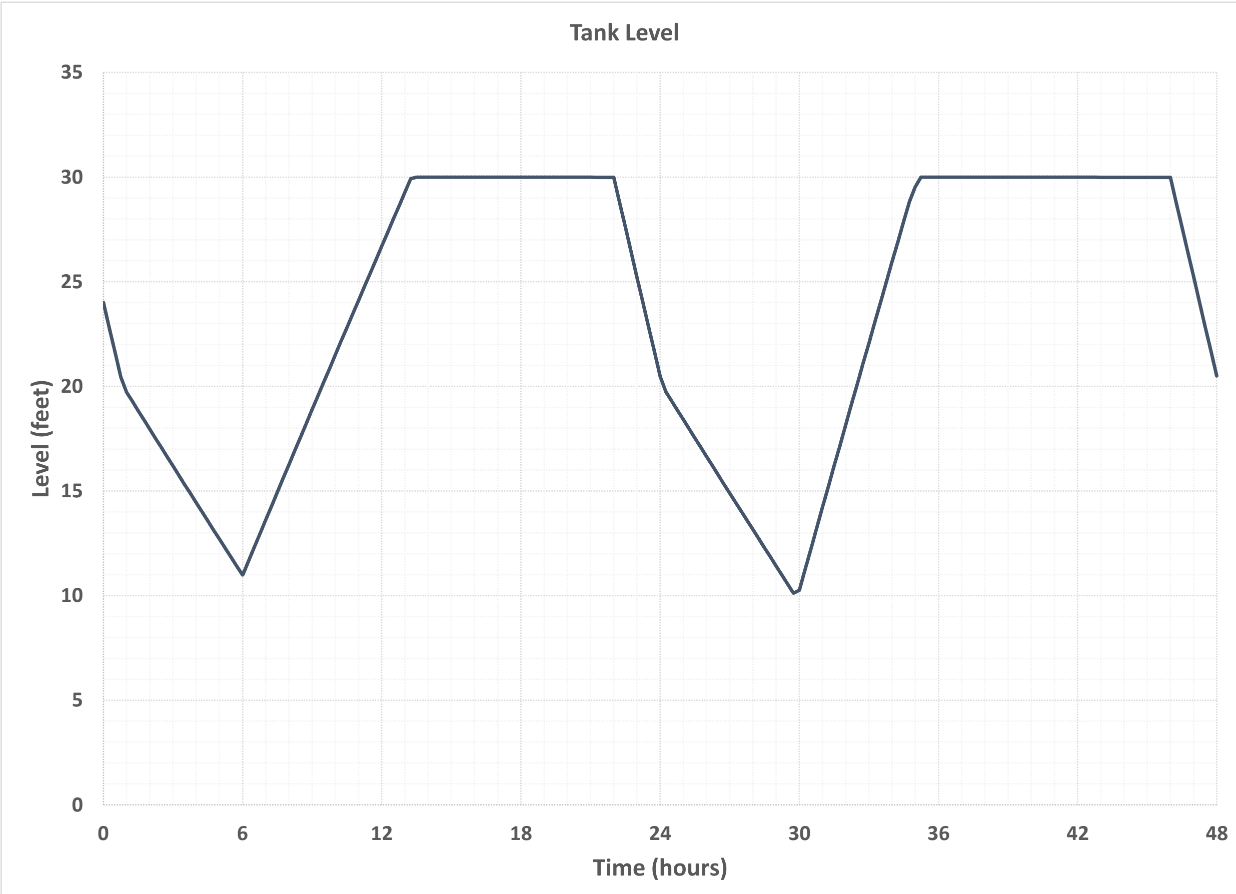
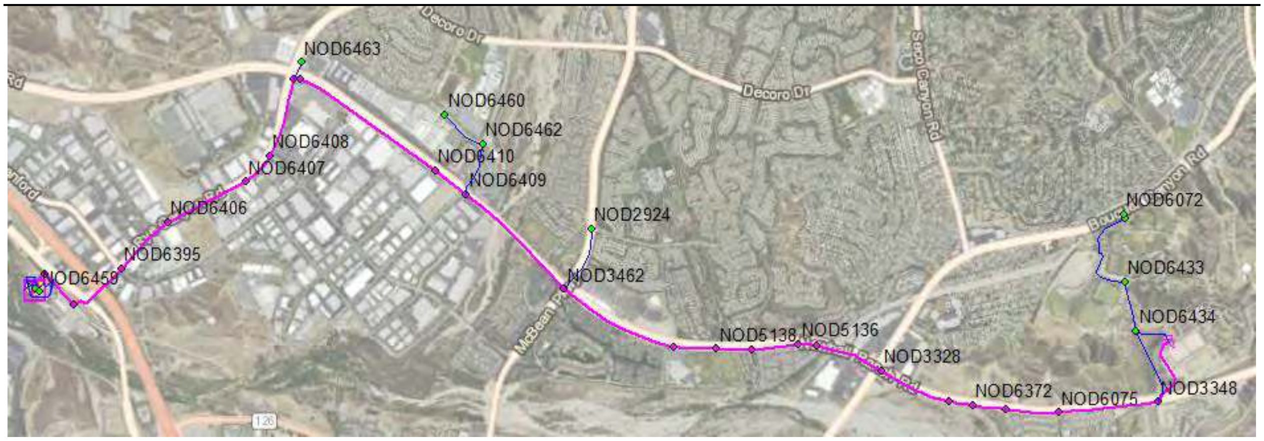


Figure B1 – Proposed Tank Level



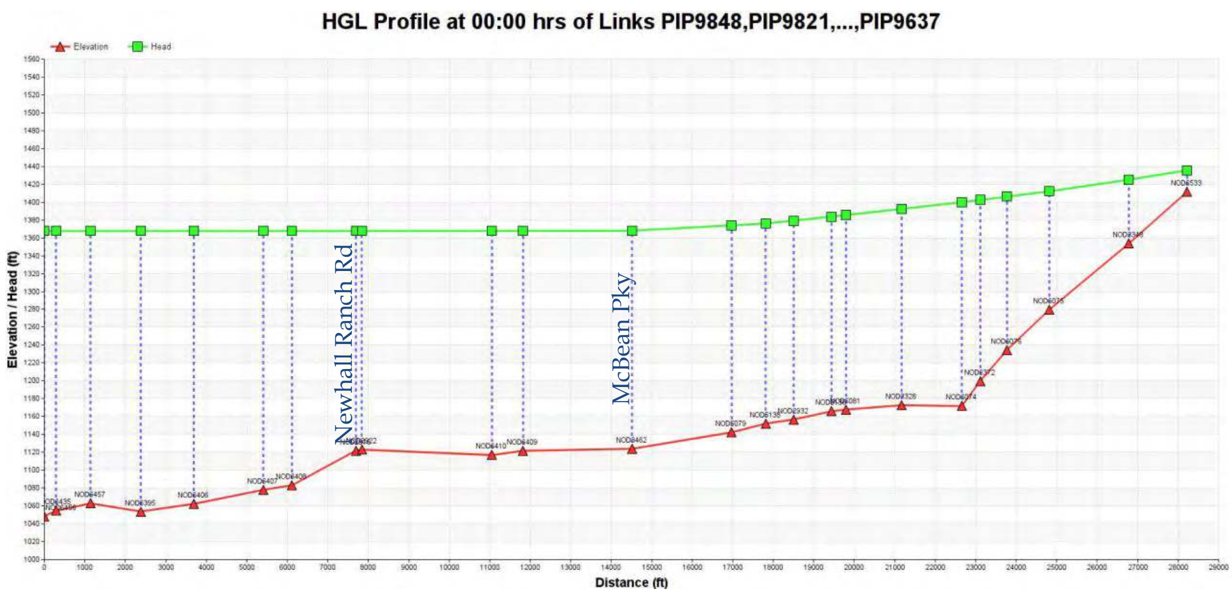


Figure B2 – HGL Profile 1

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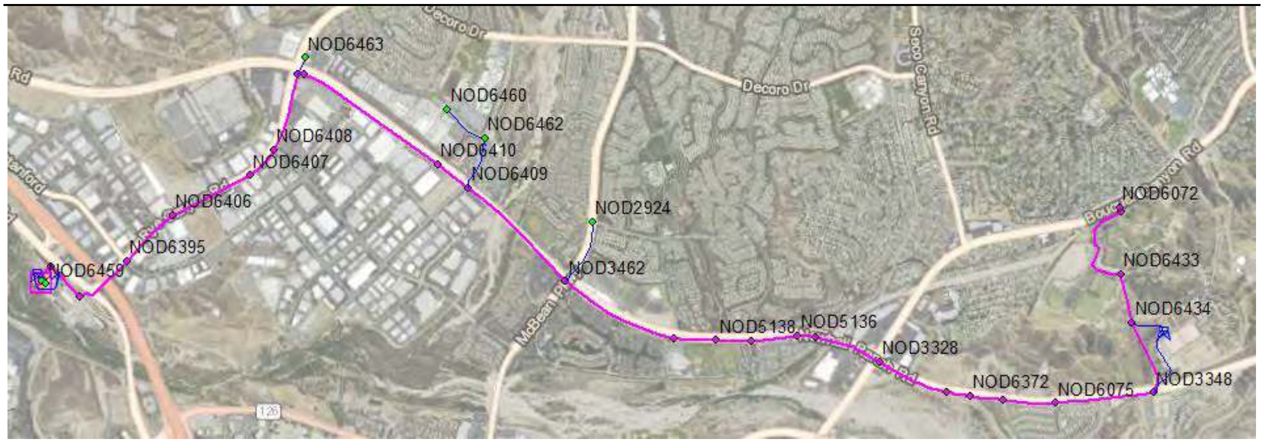
Attachment C - Phase 2A-2 Results



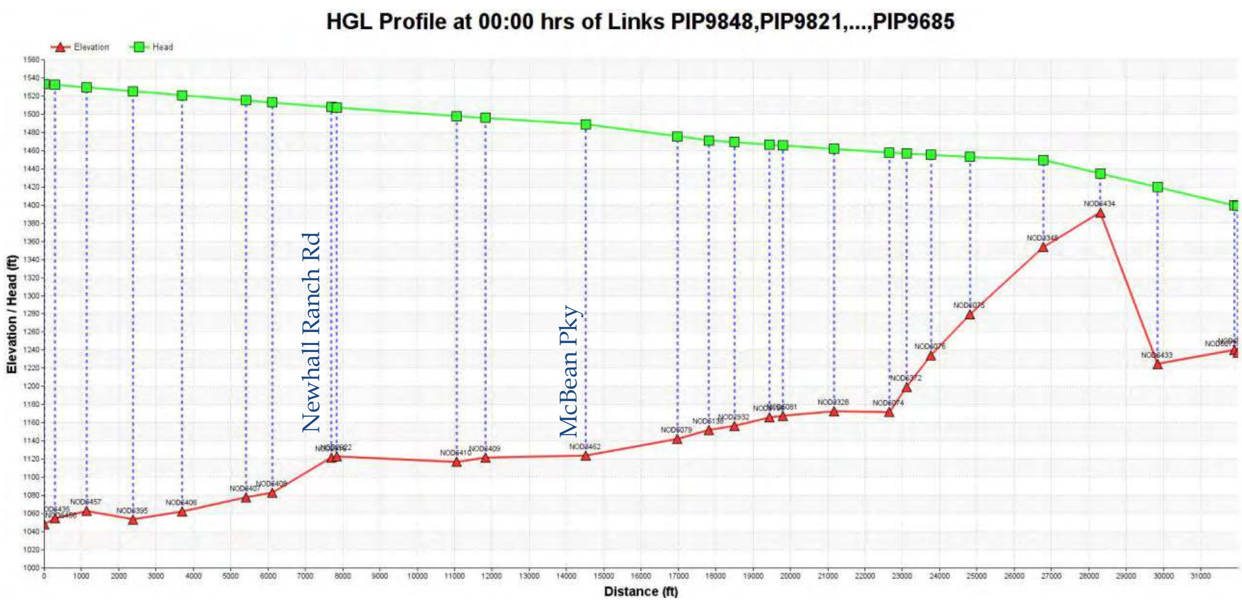


Figure C1 – HGL Profile 1

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Attachment D - Phase 2A-3 Results

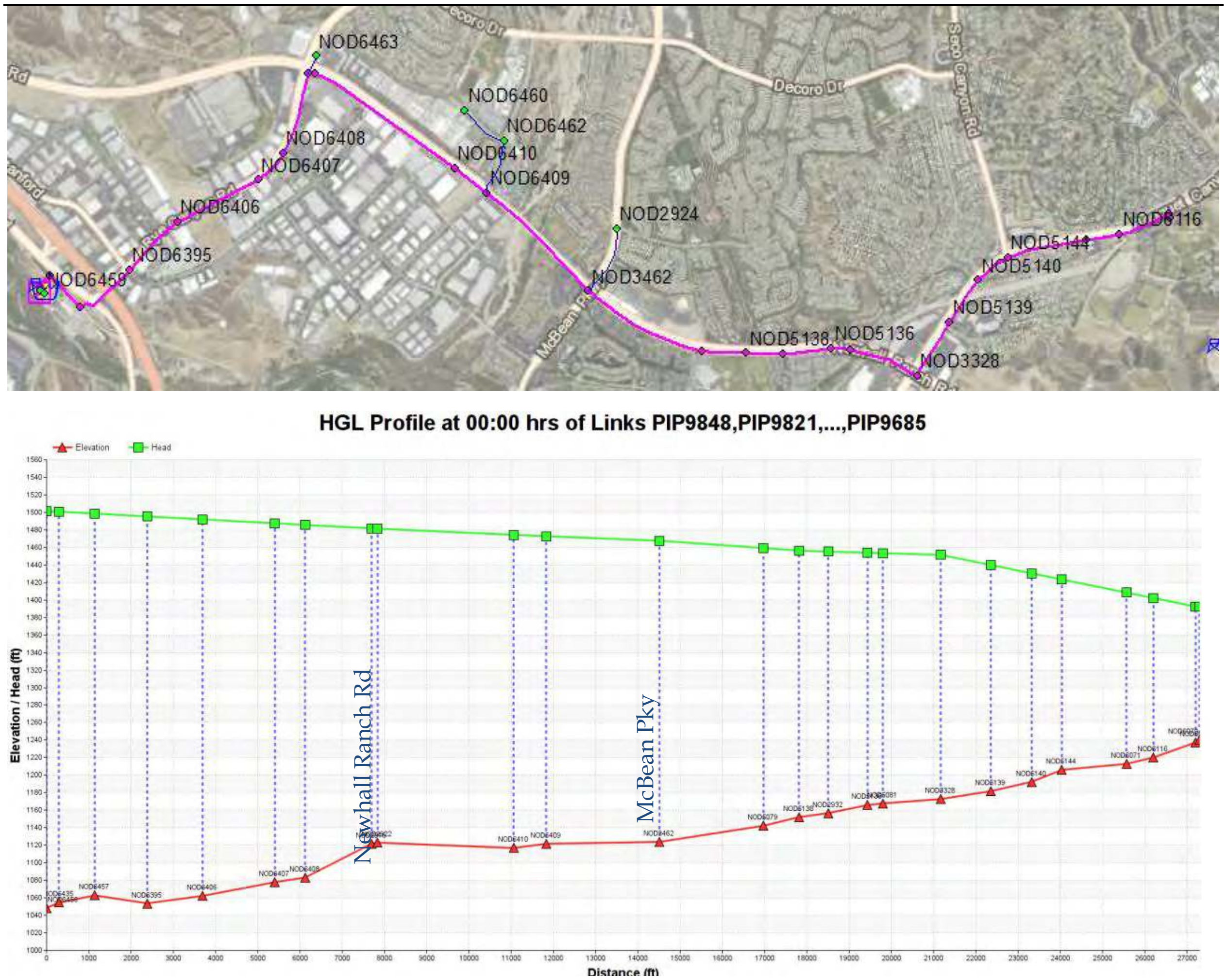

Figure D1 – HGL Profile 1

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Attachment E - Phase 2B Results

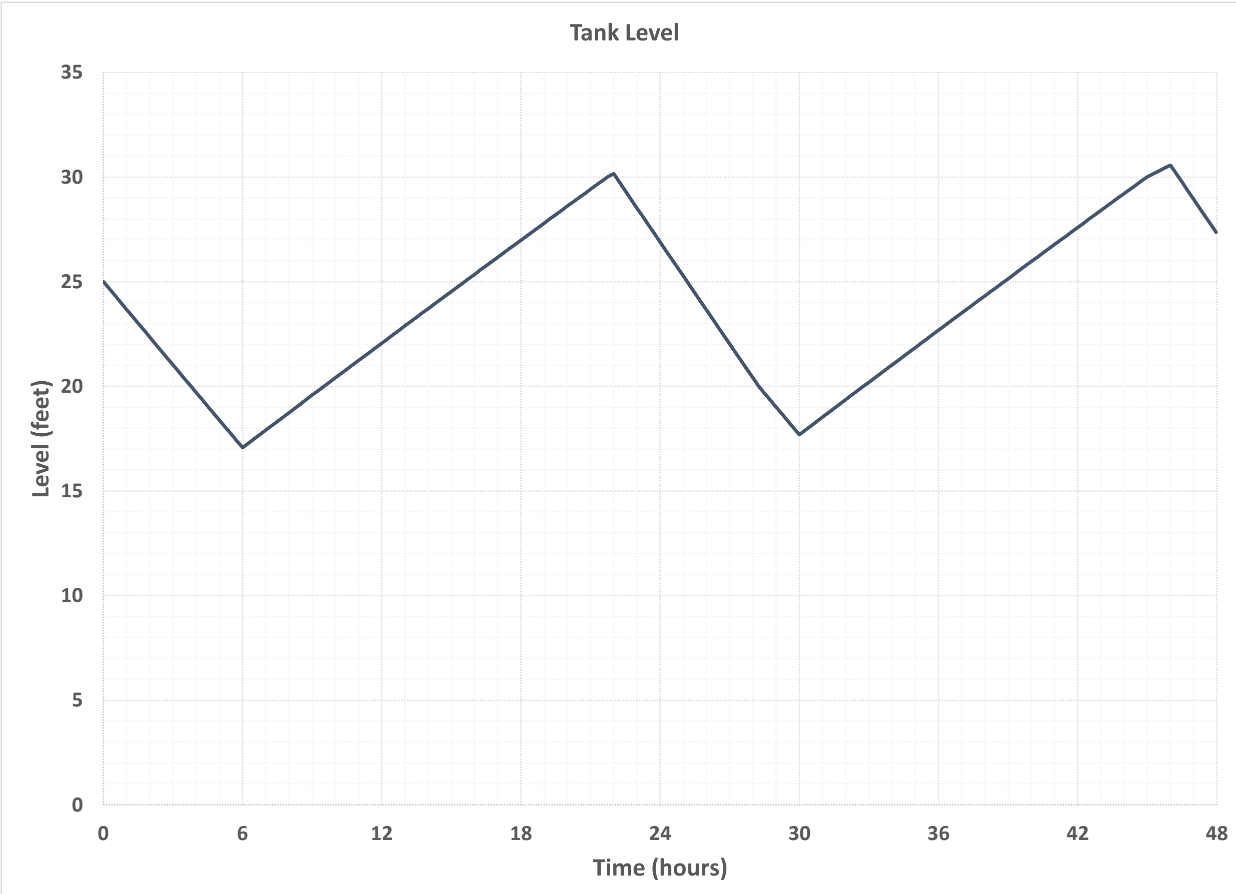


Figure E1 – Proposed Tank Level

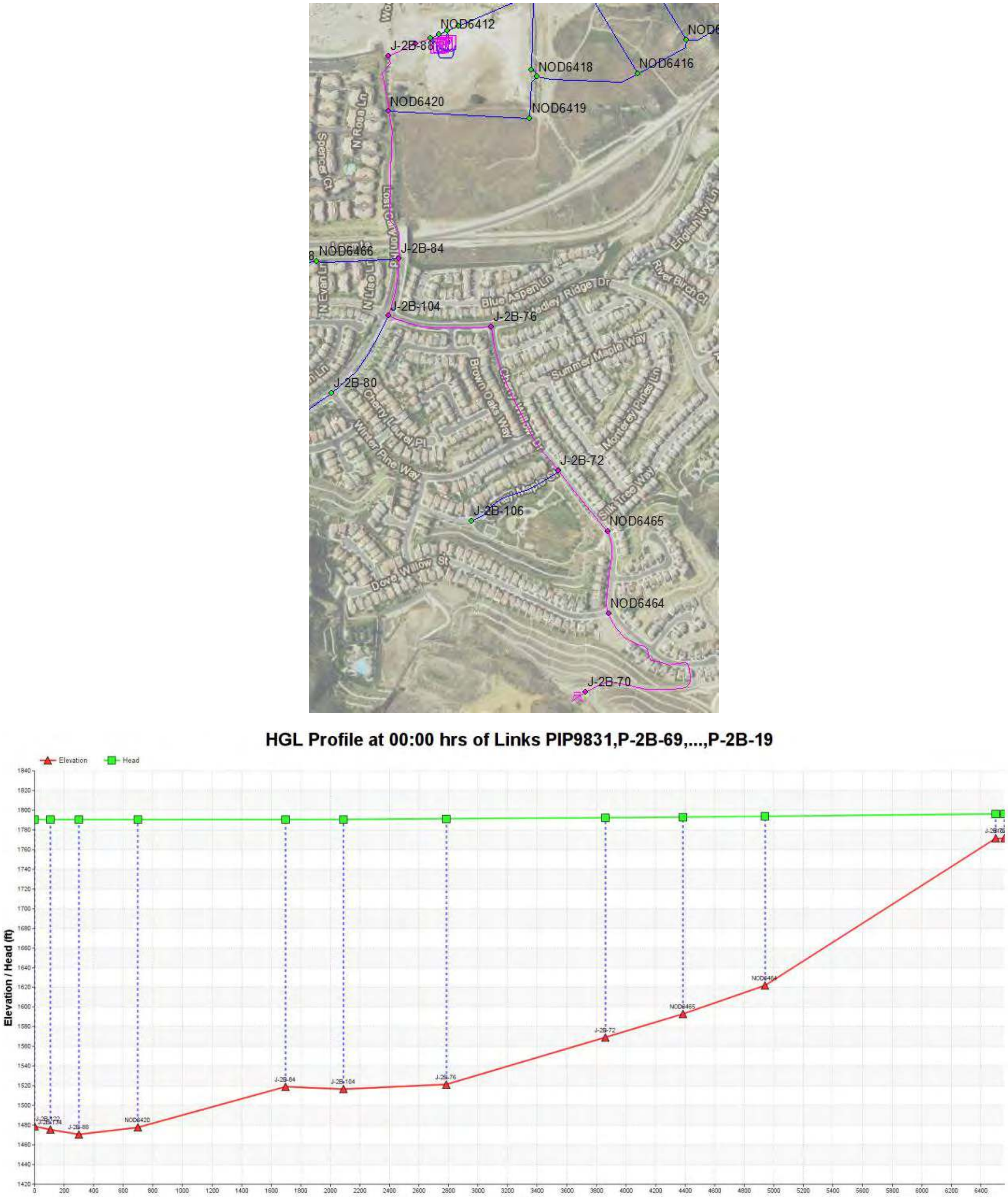
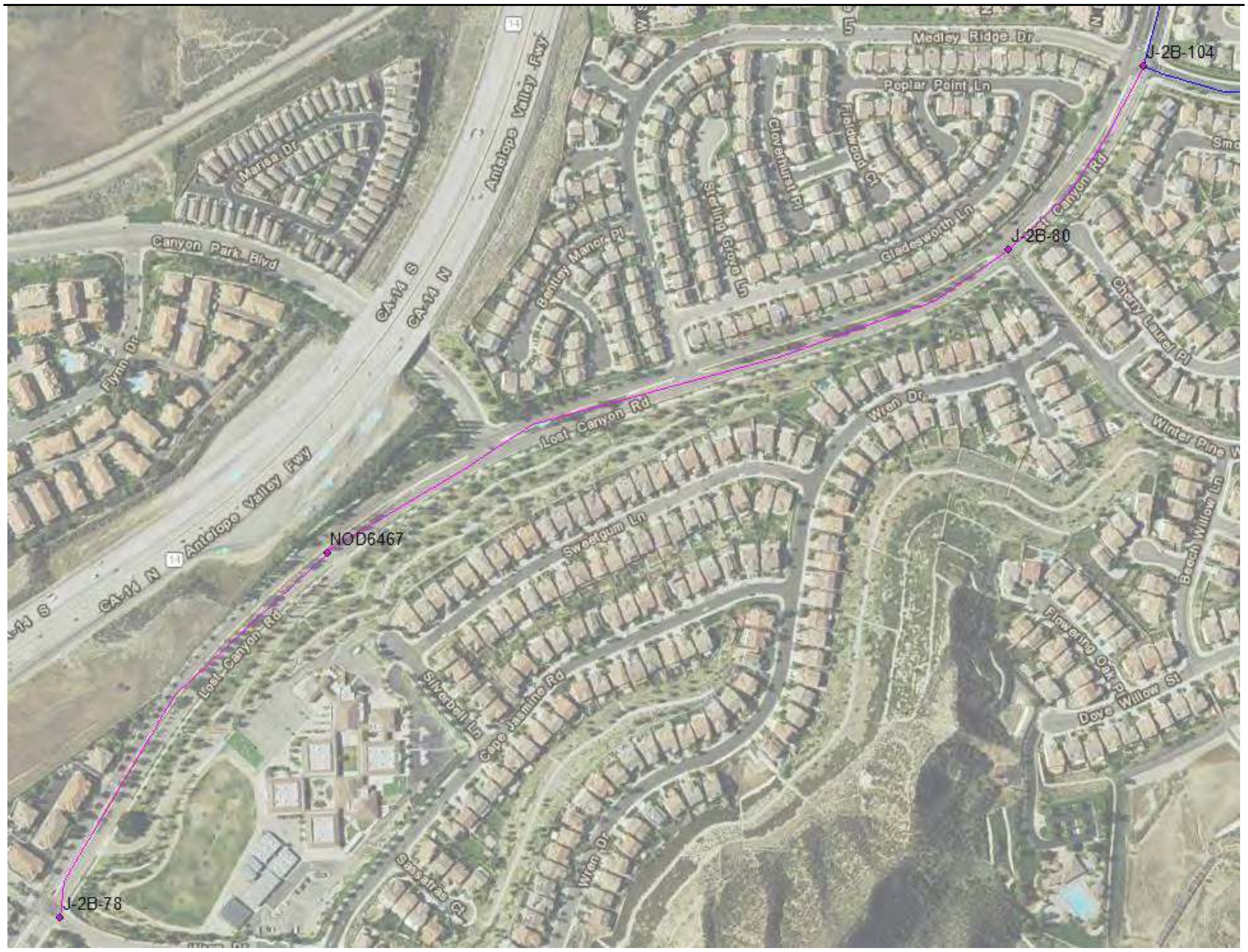


Figure E2 – HGL Profile 1



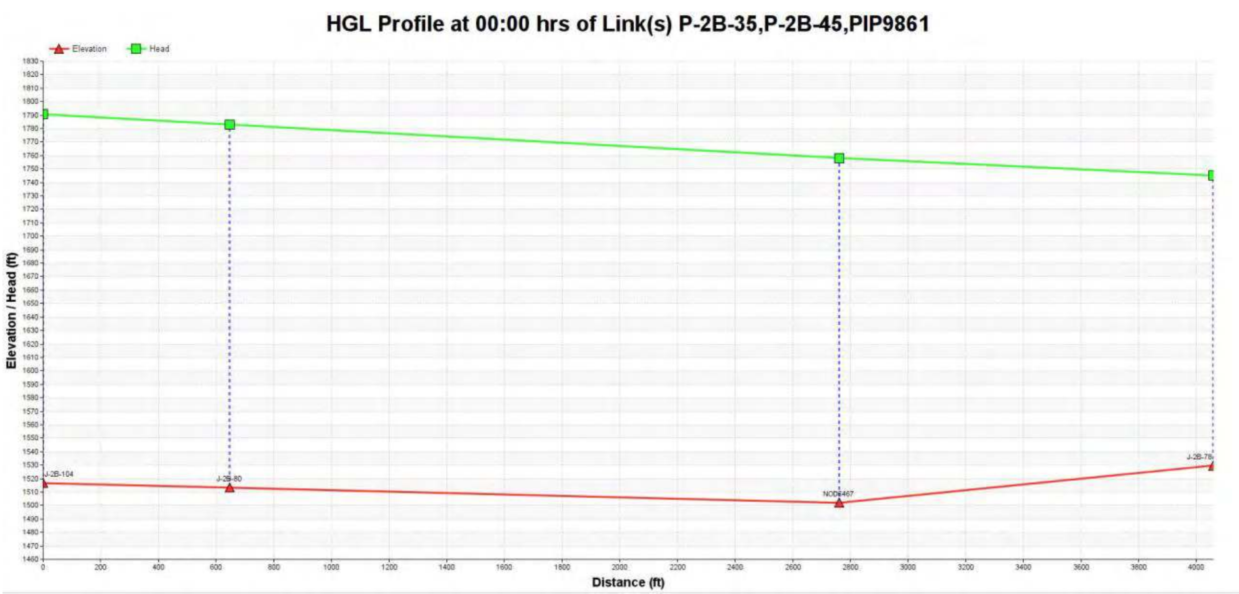


Figure E3 – HGL Profile 2

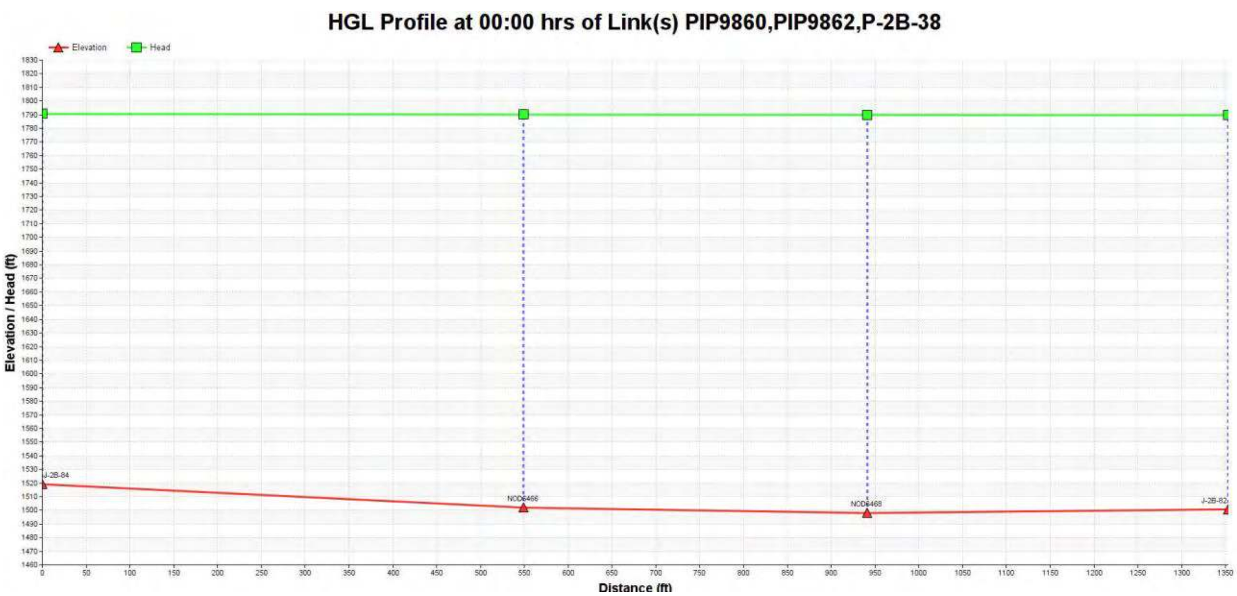


Figure E4 – HGL Profile 3

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Attachment F - Phase 2C and 2D Results

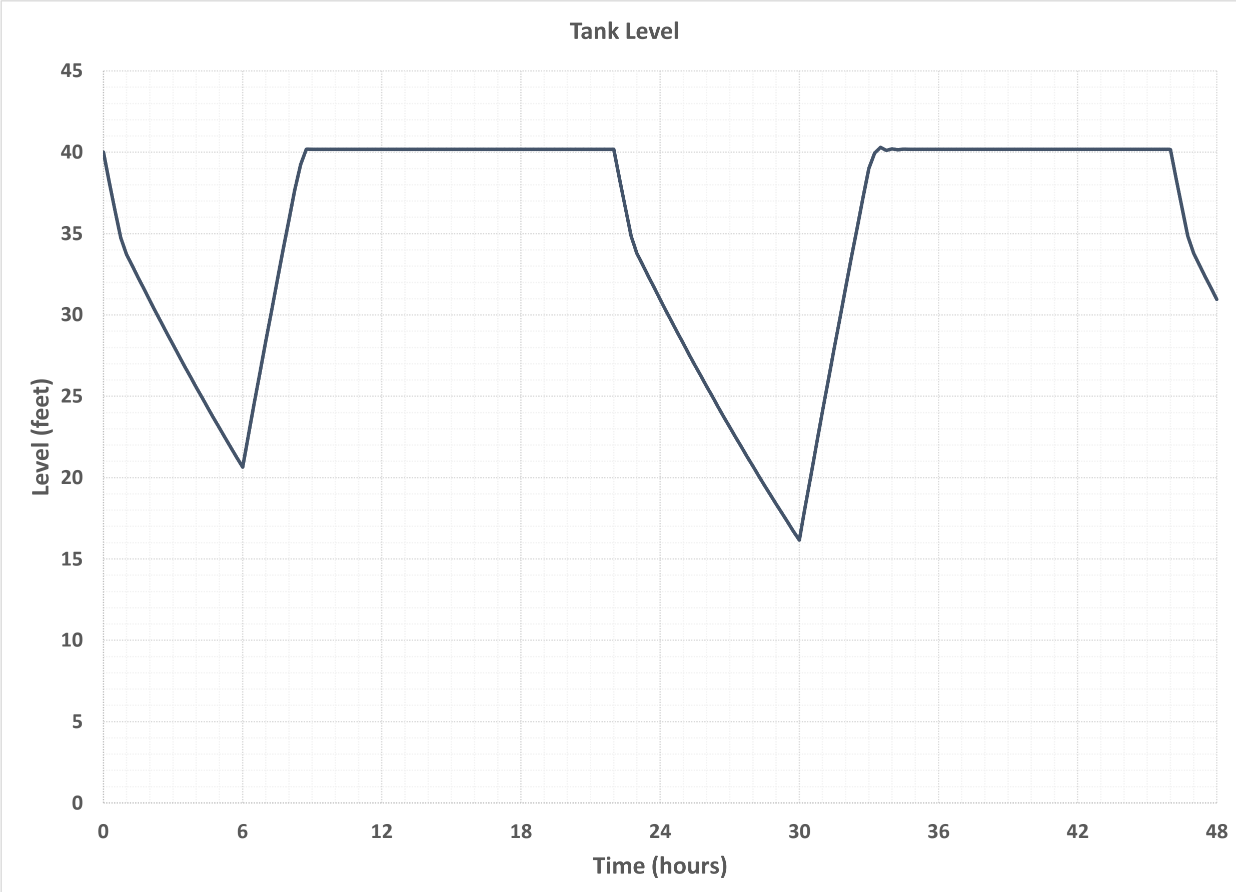


Figure F1 – Existing Tank Level

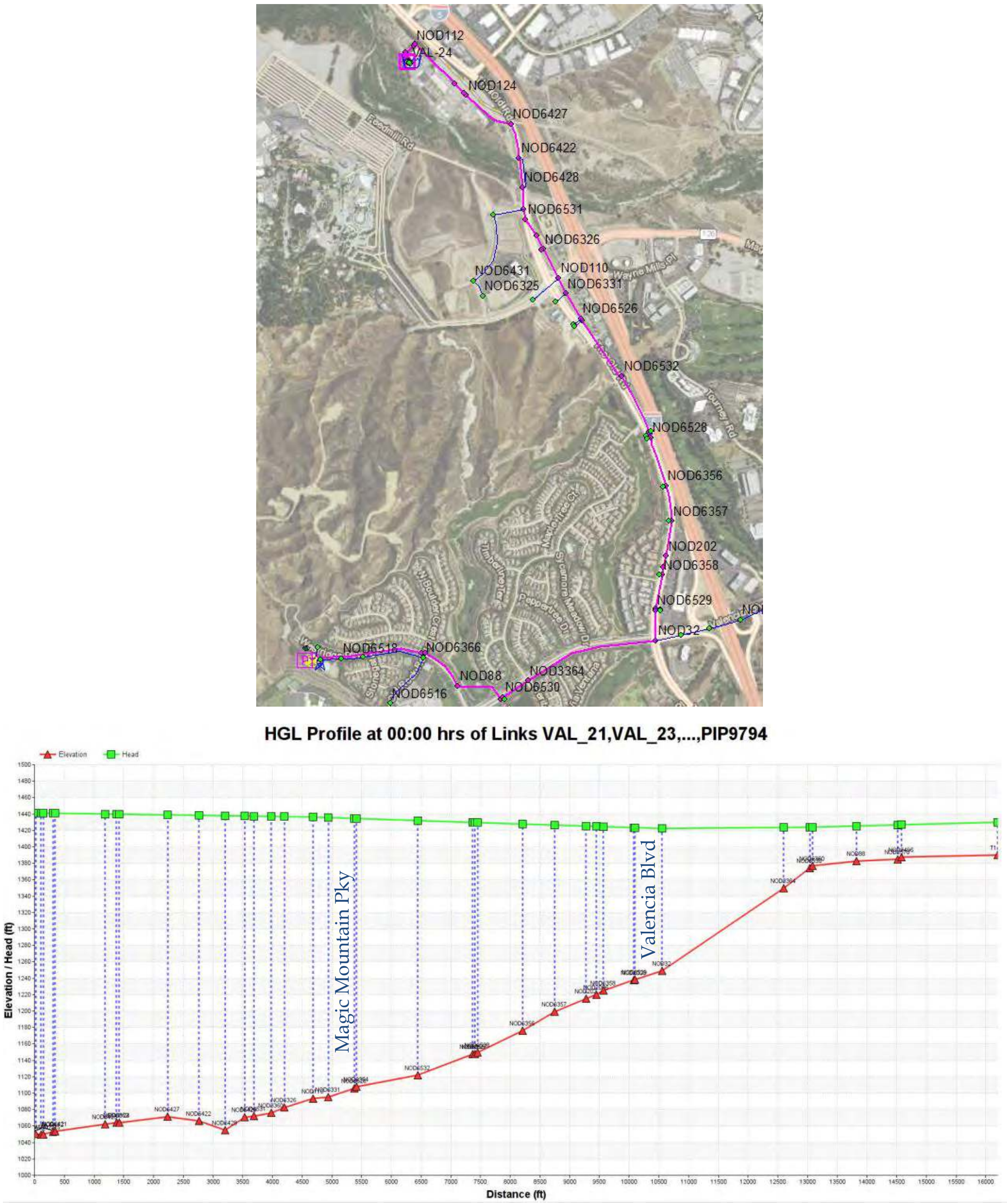


Figure F2 – HGL Profile 1

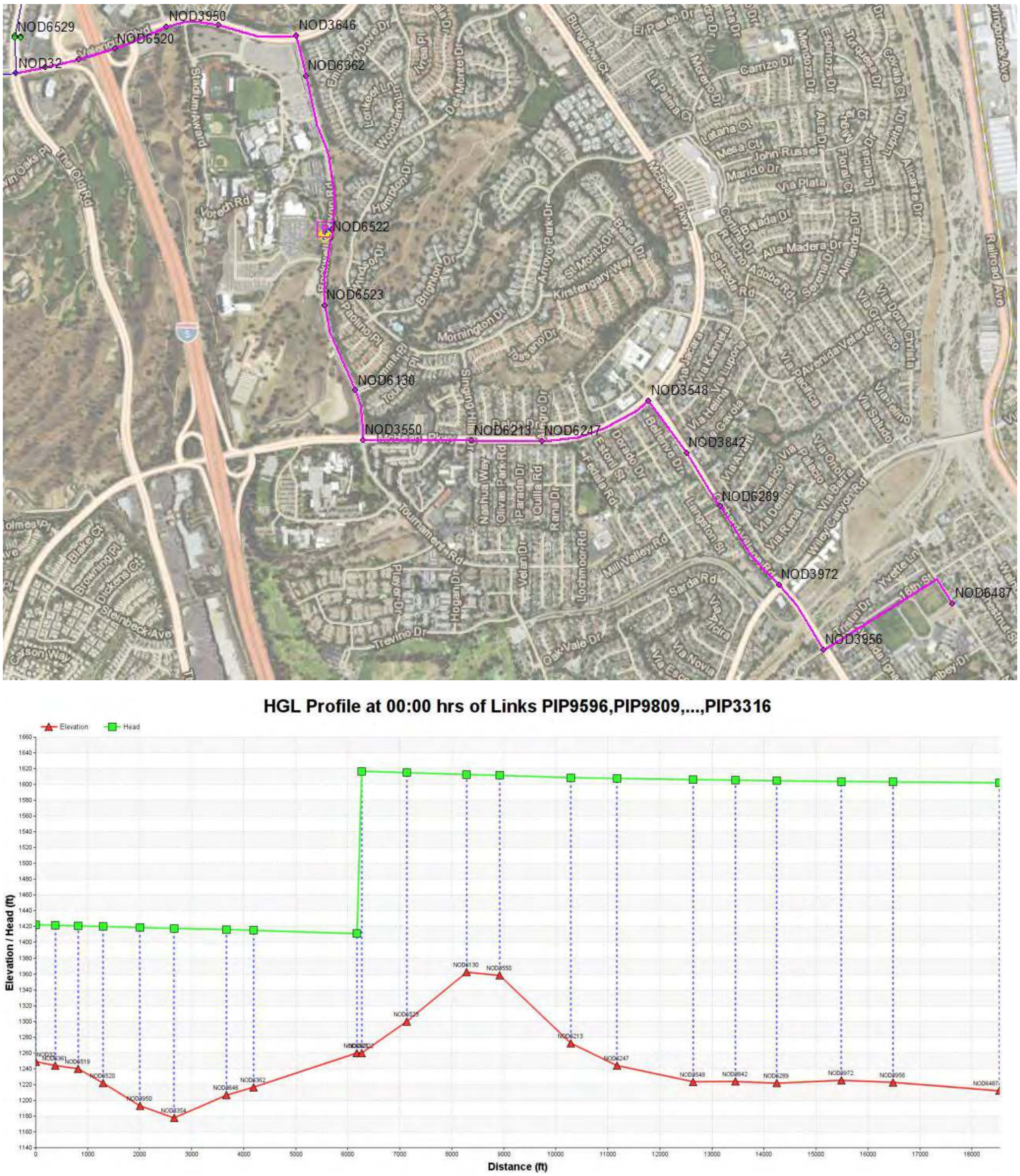
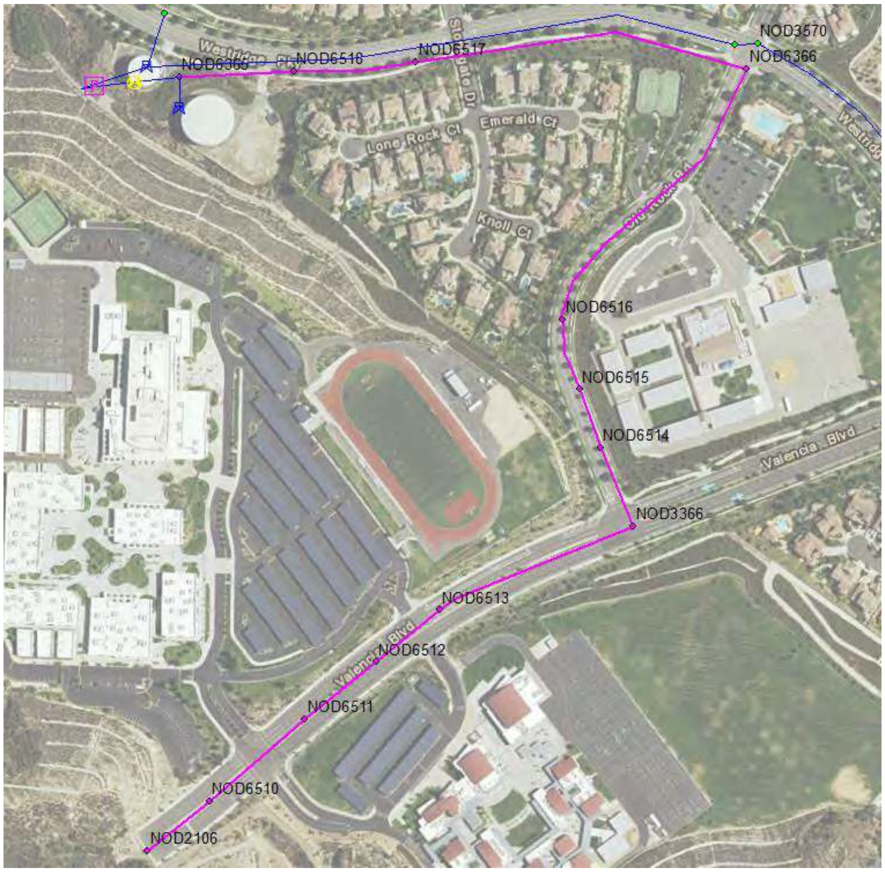


Figure F3 – HGL Profile 2



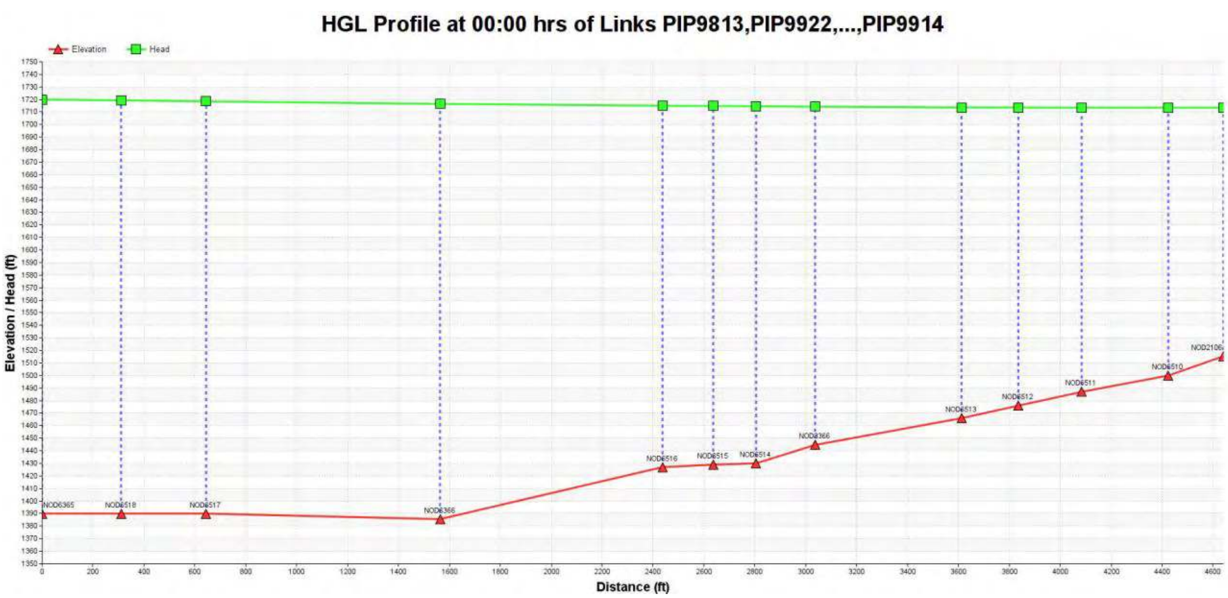


Figure F4 – HGL Profile 3

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Attachment G - IPR Scenario 1 Results

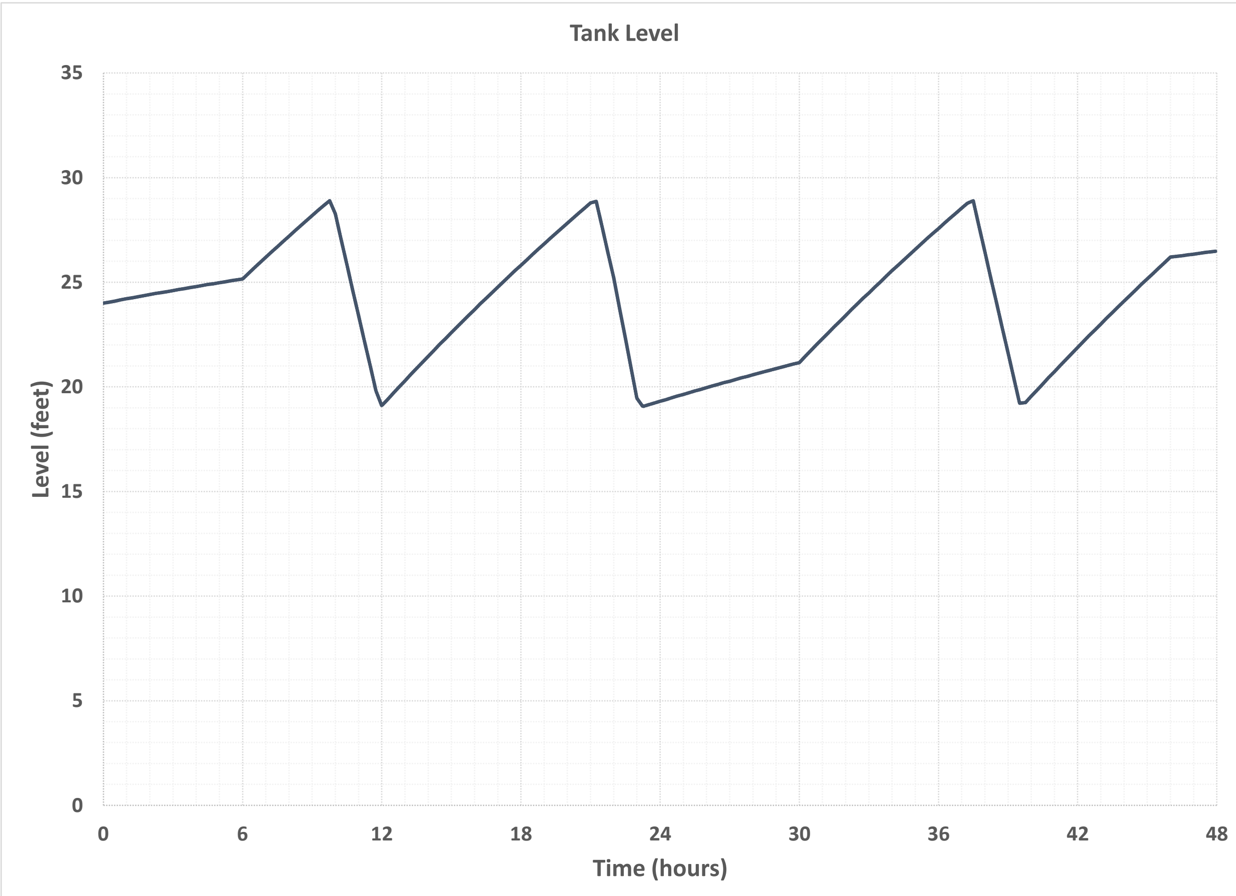

Figure G1 – Proposed Tank Level

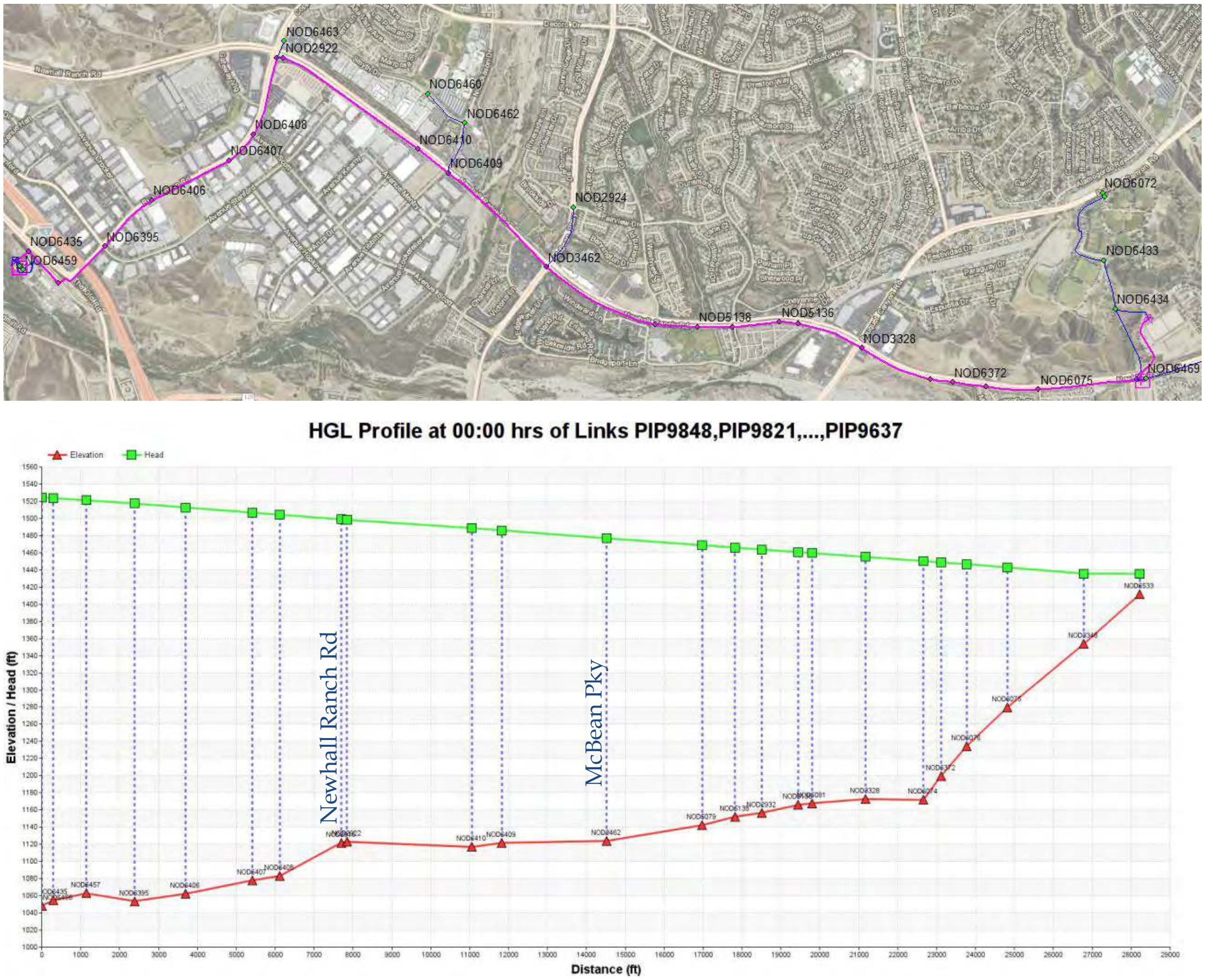

Figure G2 – HGL Profile 1

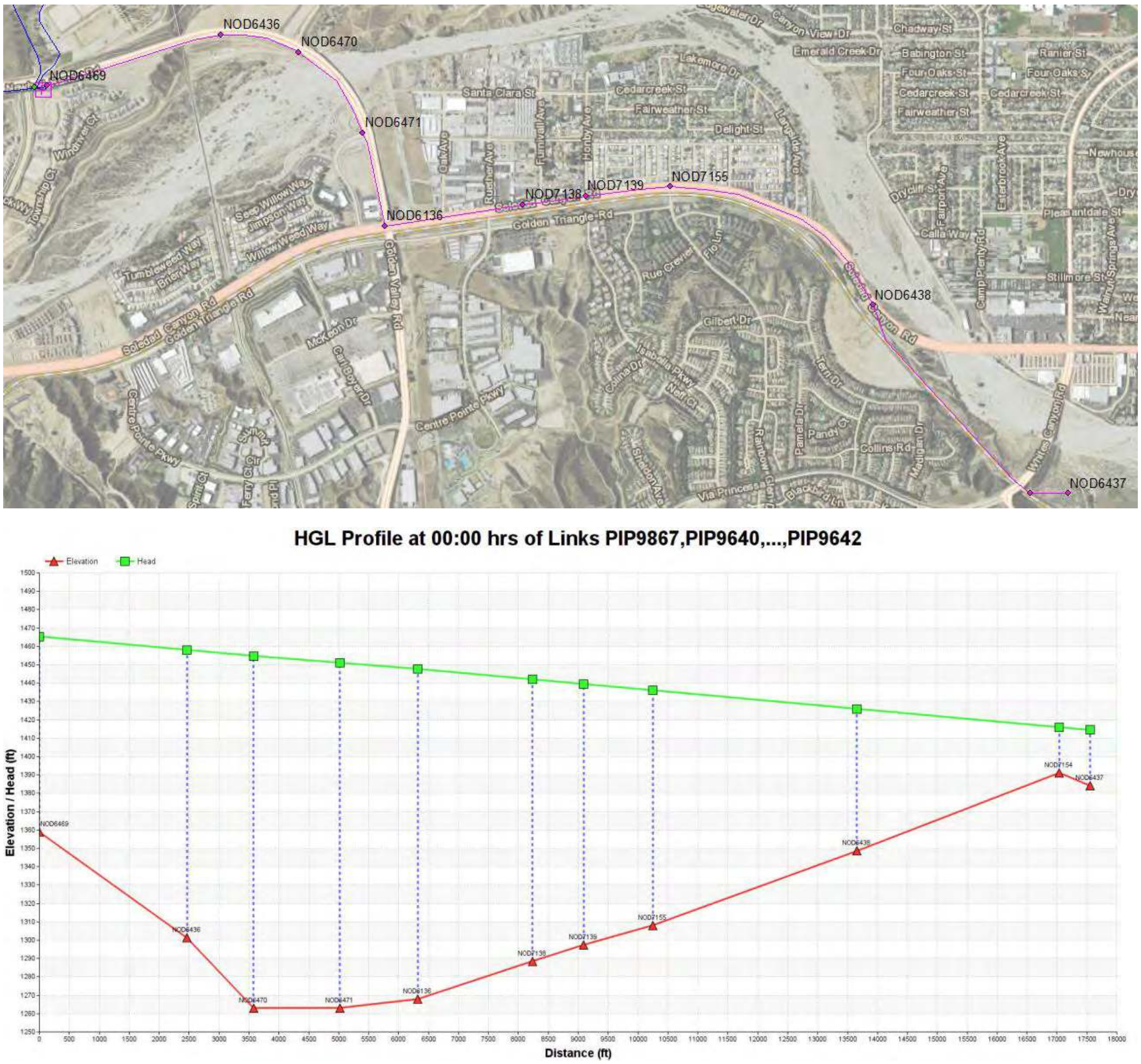

Figure G3 – HGL Profile 2

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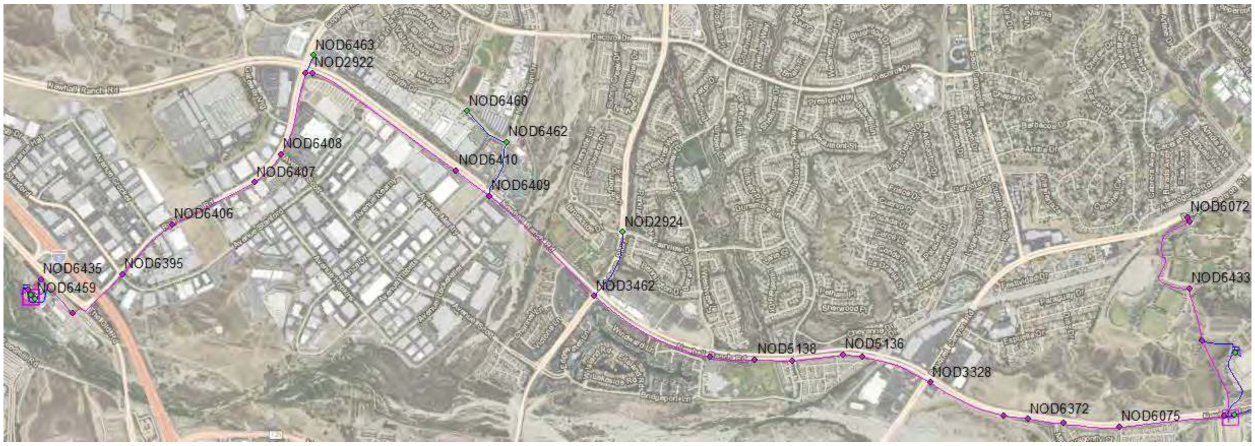
Attachment H - IPR Scenario 2 Results



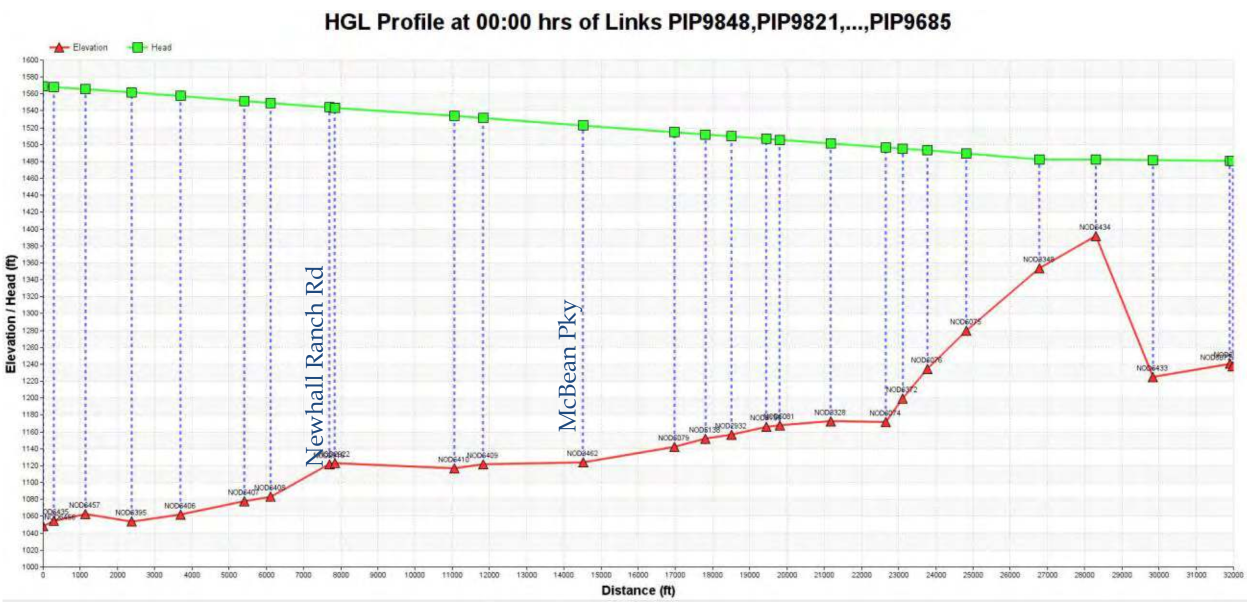


Figure H1 – HGL Profile 1

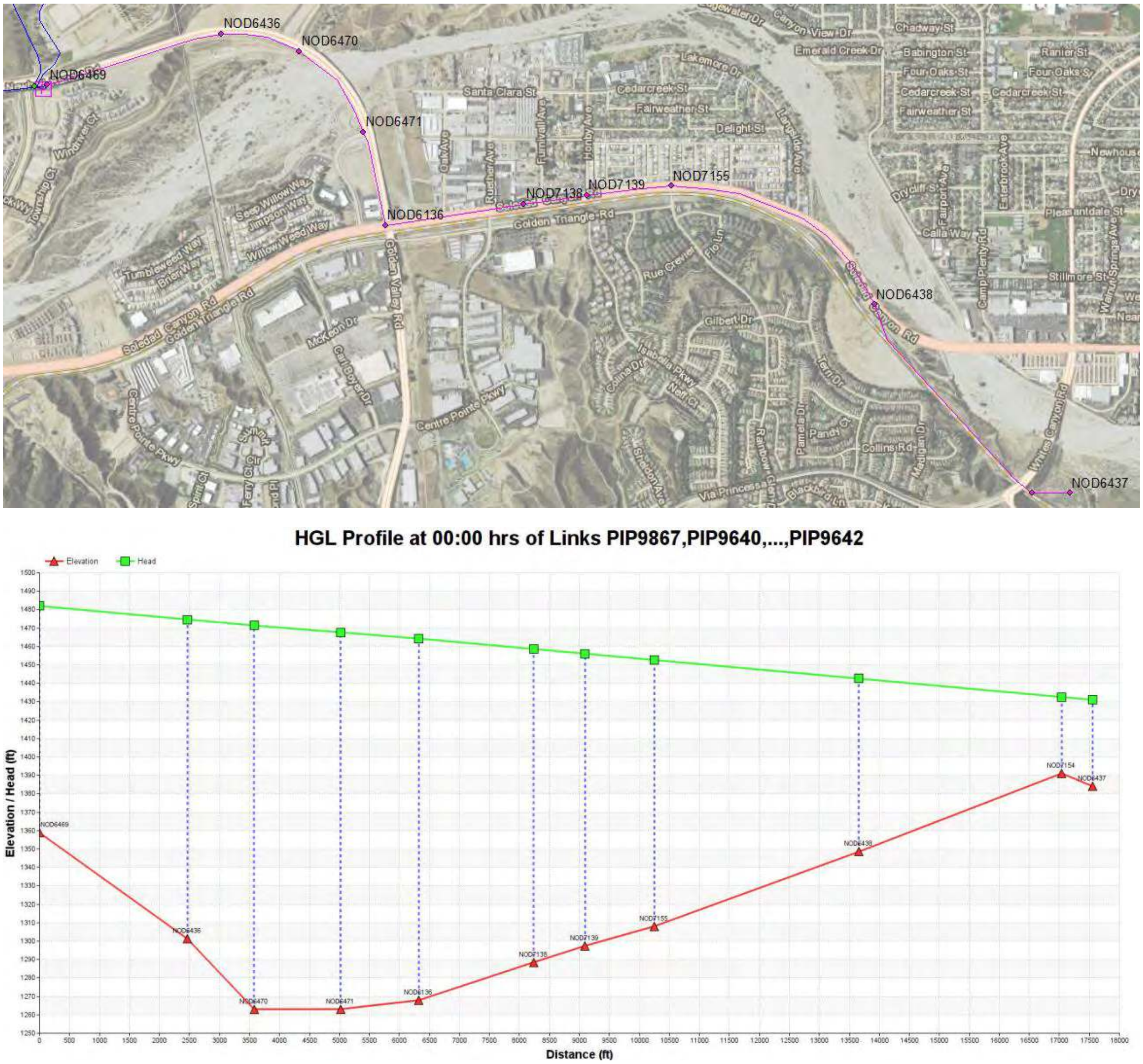


Figure H2 – HGL Profile 2

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Attachment I -
IPR Scenario 3 Results

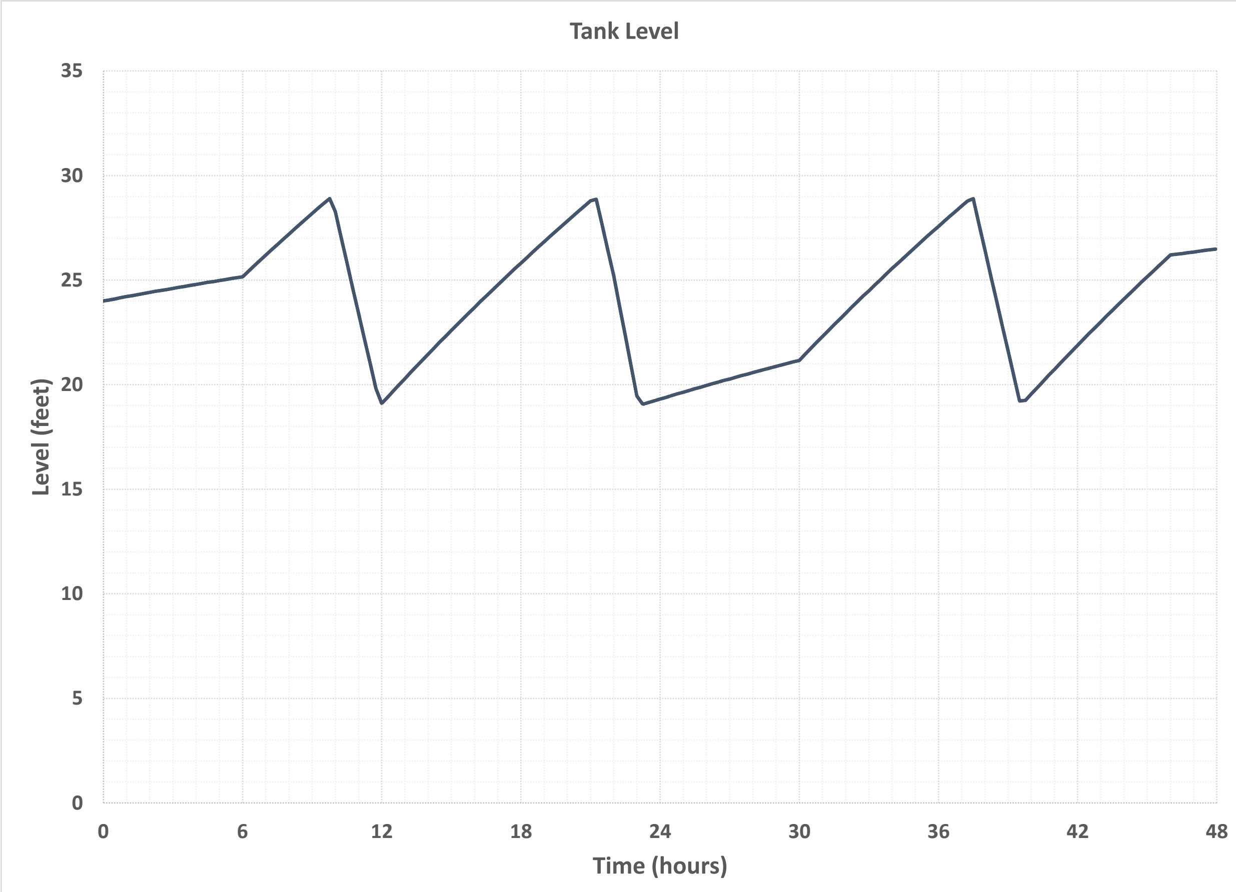

Figure I1 – Proposed Tank Level

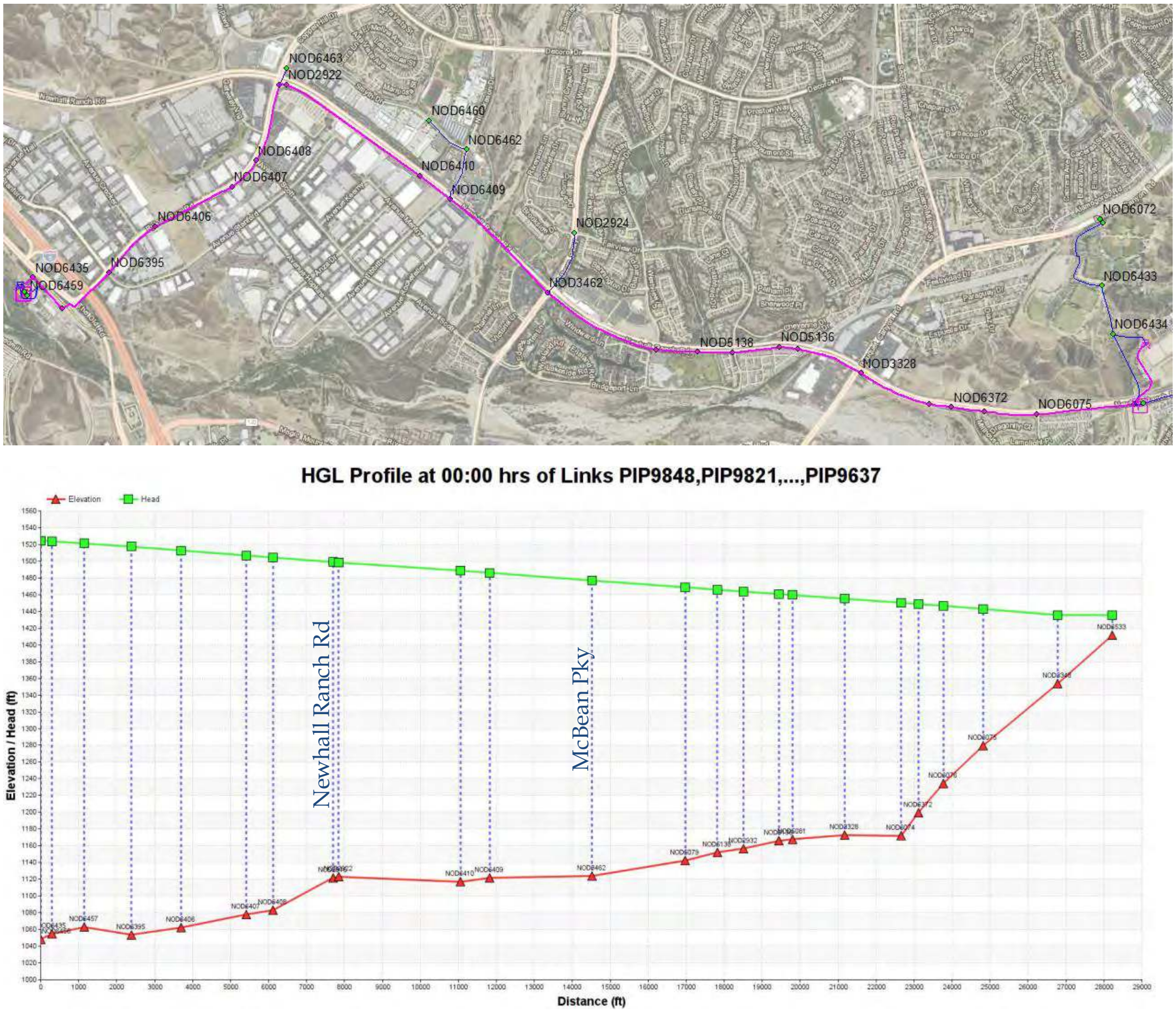

Figure I2 – HGL Profile 1

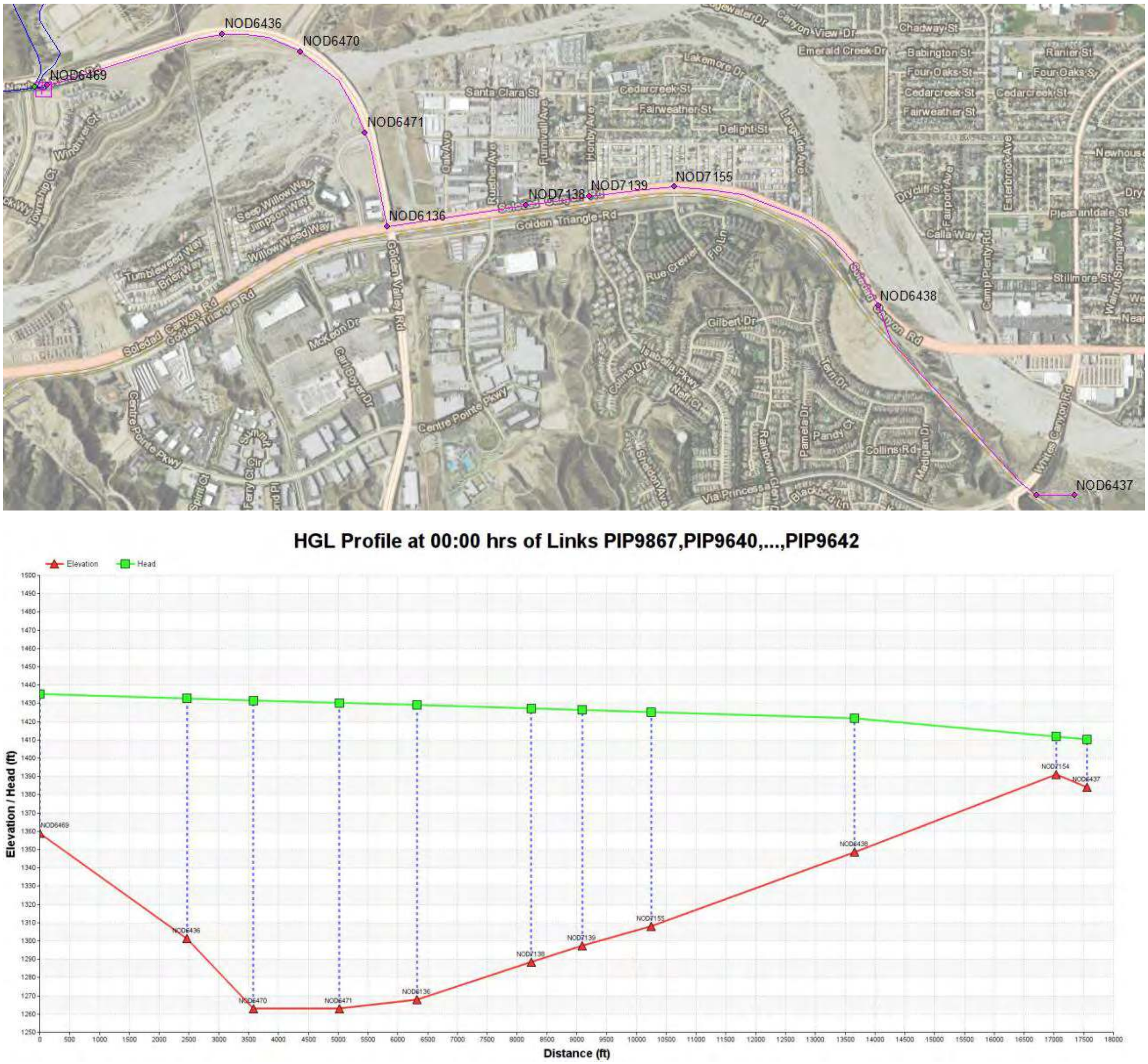

Figure I3 – HGL Profile 2

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Attachment J - IPR Scenario 4 Results

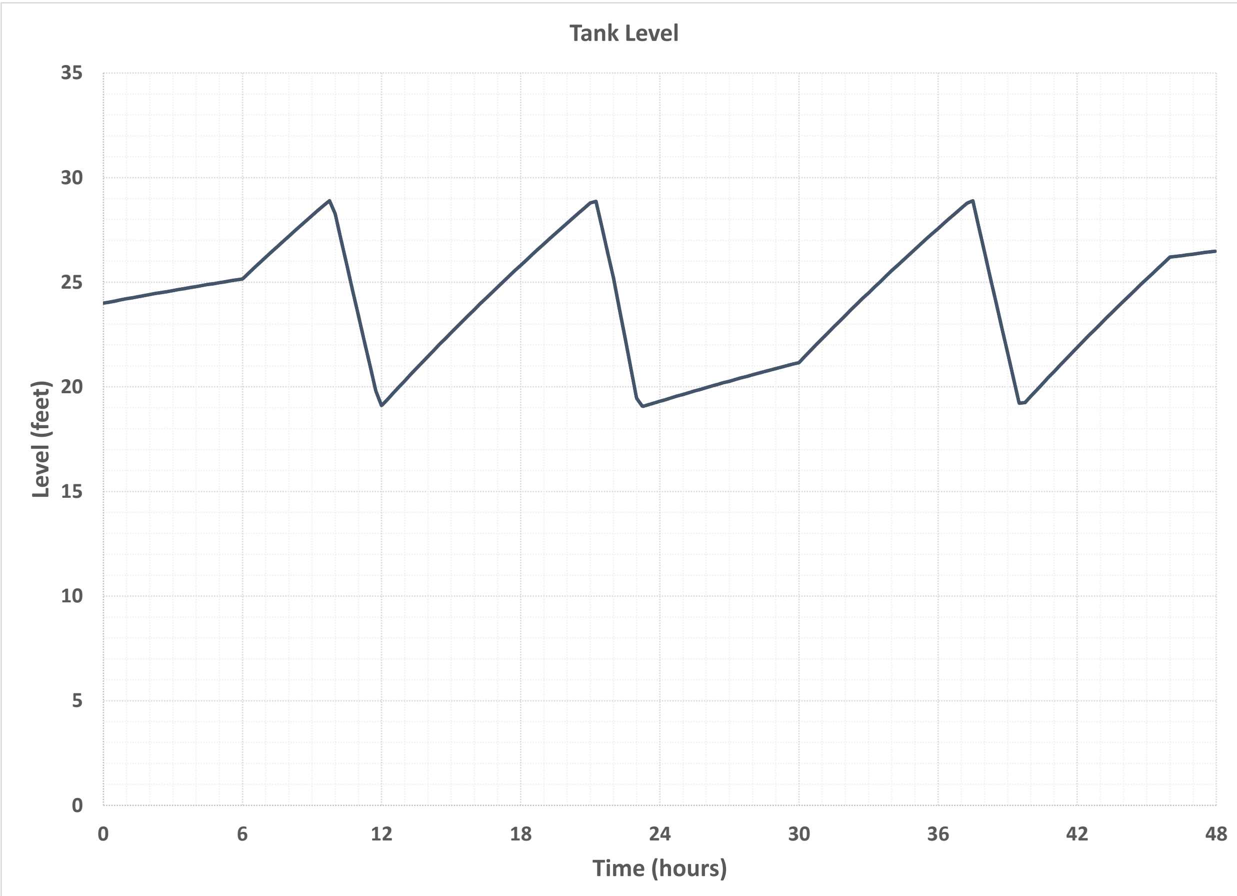

Figure J1 – Proposed Tank Level

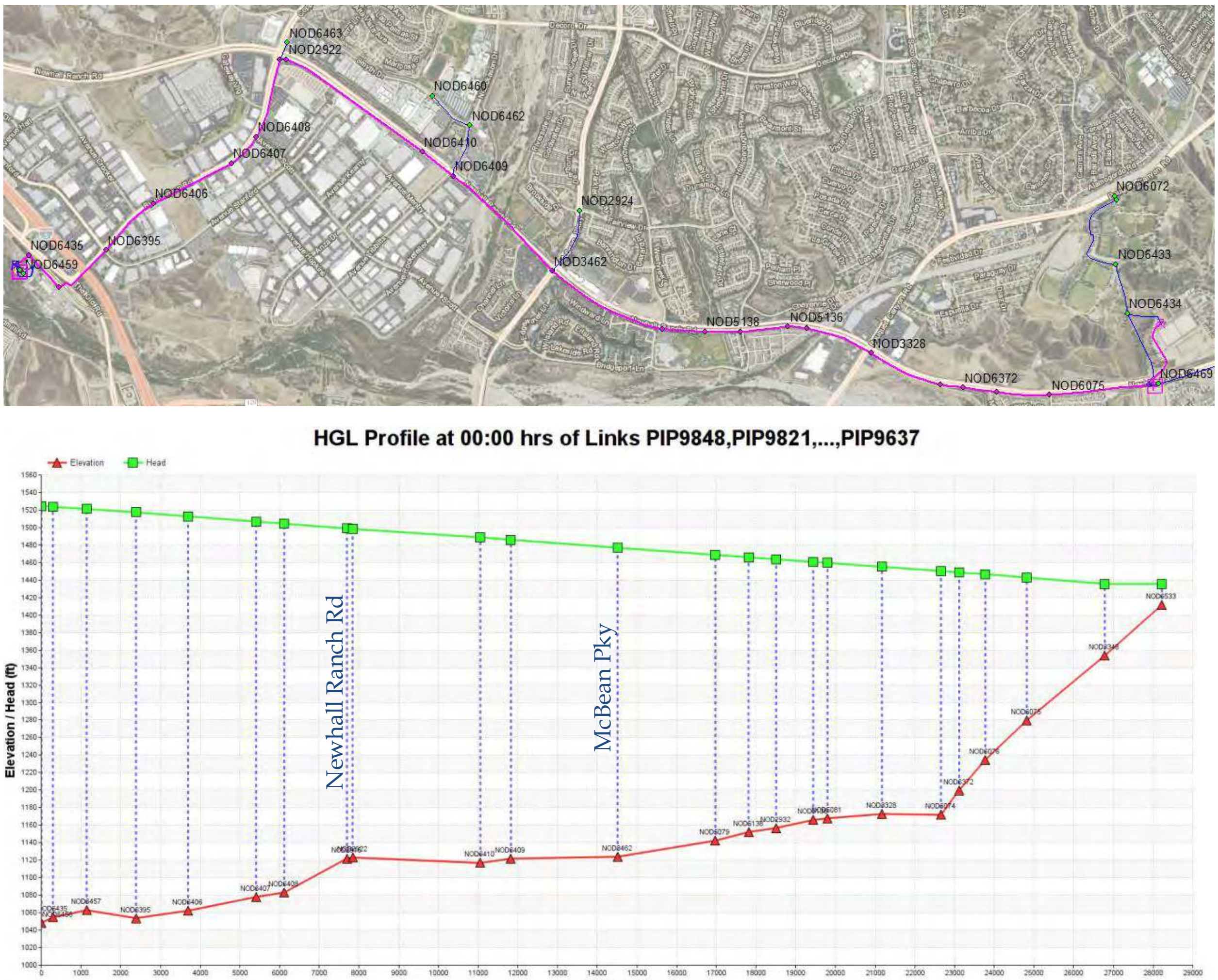

Figure J2 – HGL Profile 1

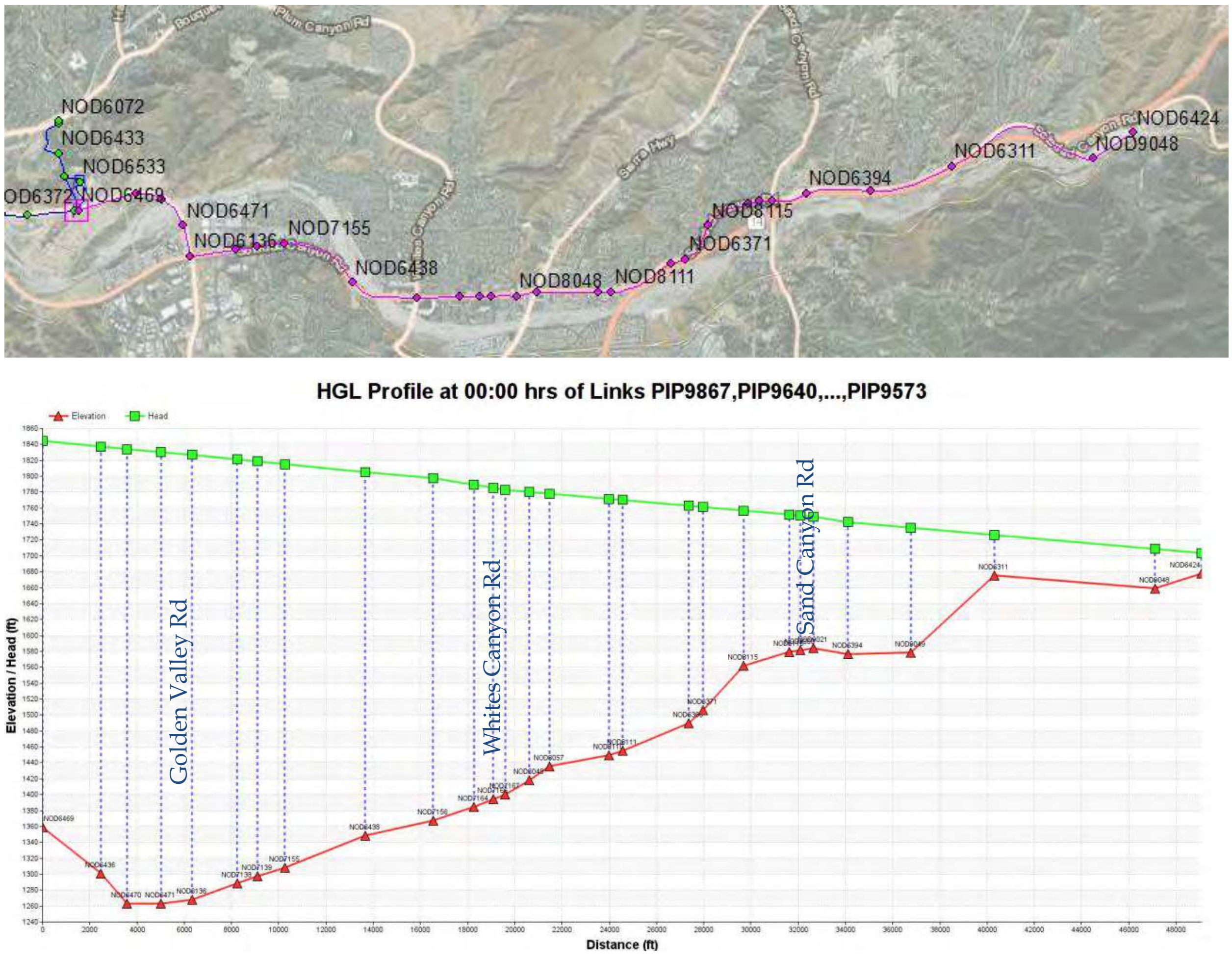

Figure J3 – HGL Profile 2

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Attachment K - Phase 2A + Future Expansion North Results

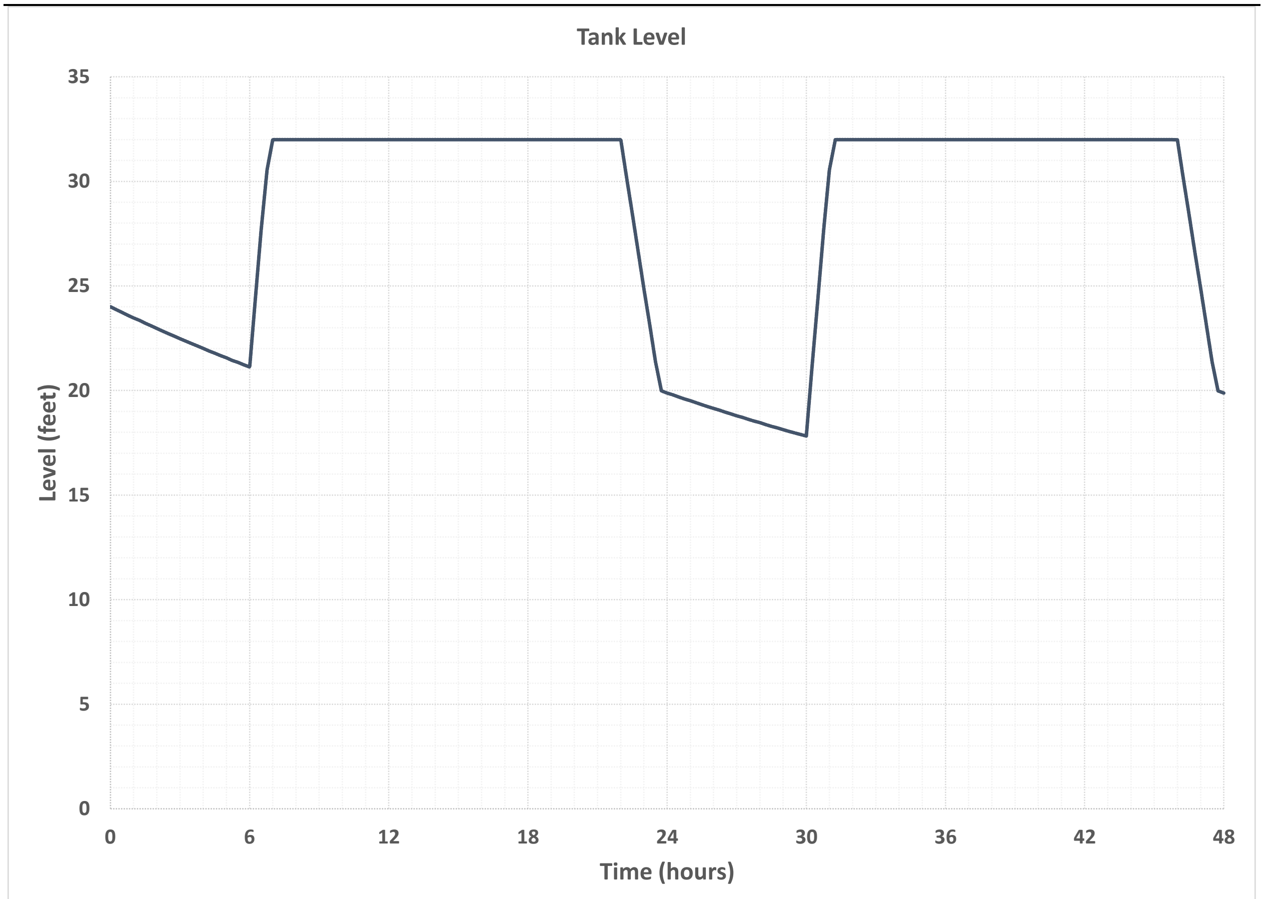
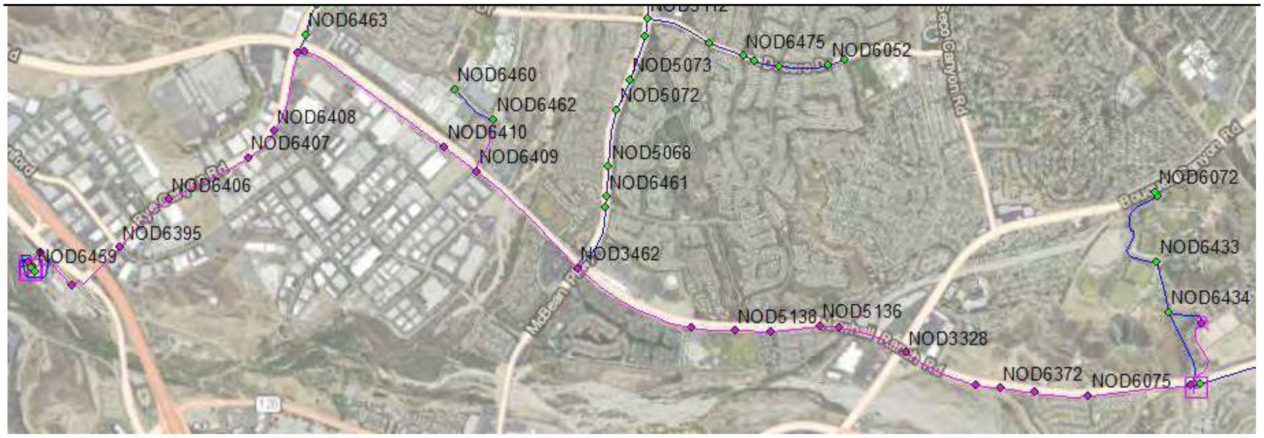


Figure K1 – Proposed Tank Level



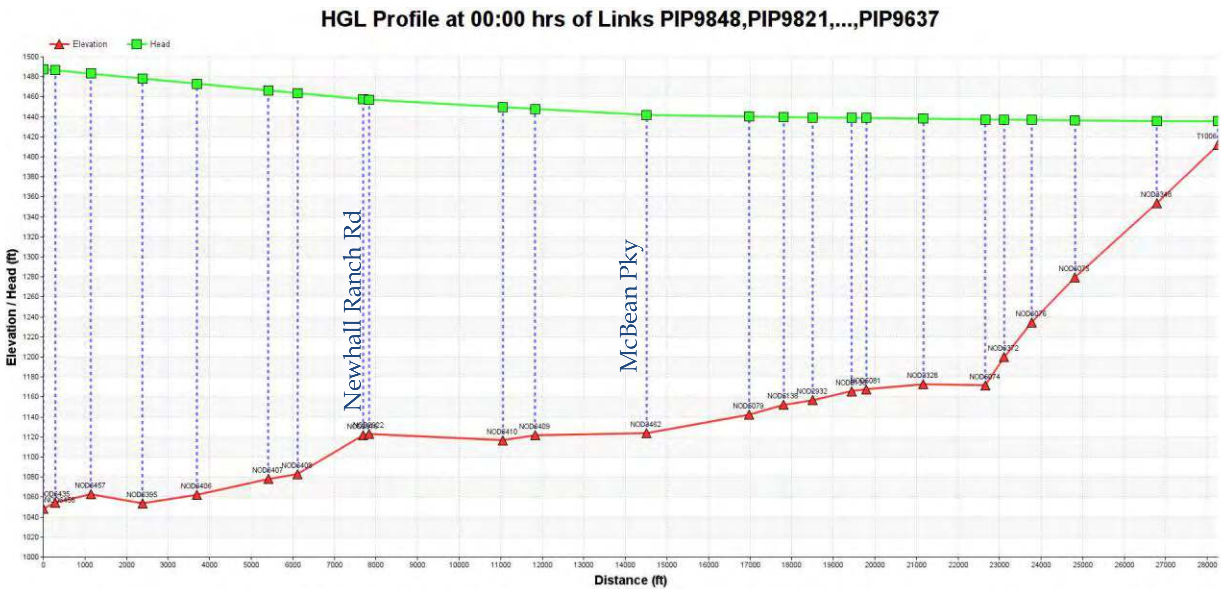


Figure K2 – HGL Profile 1

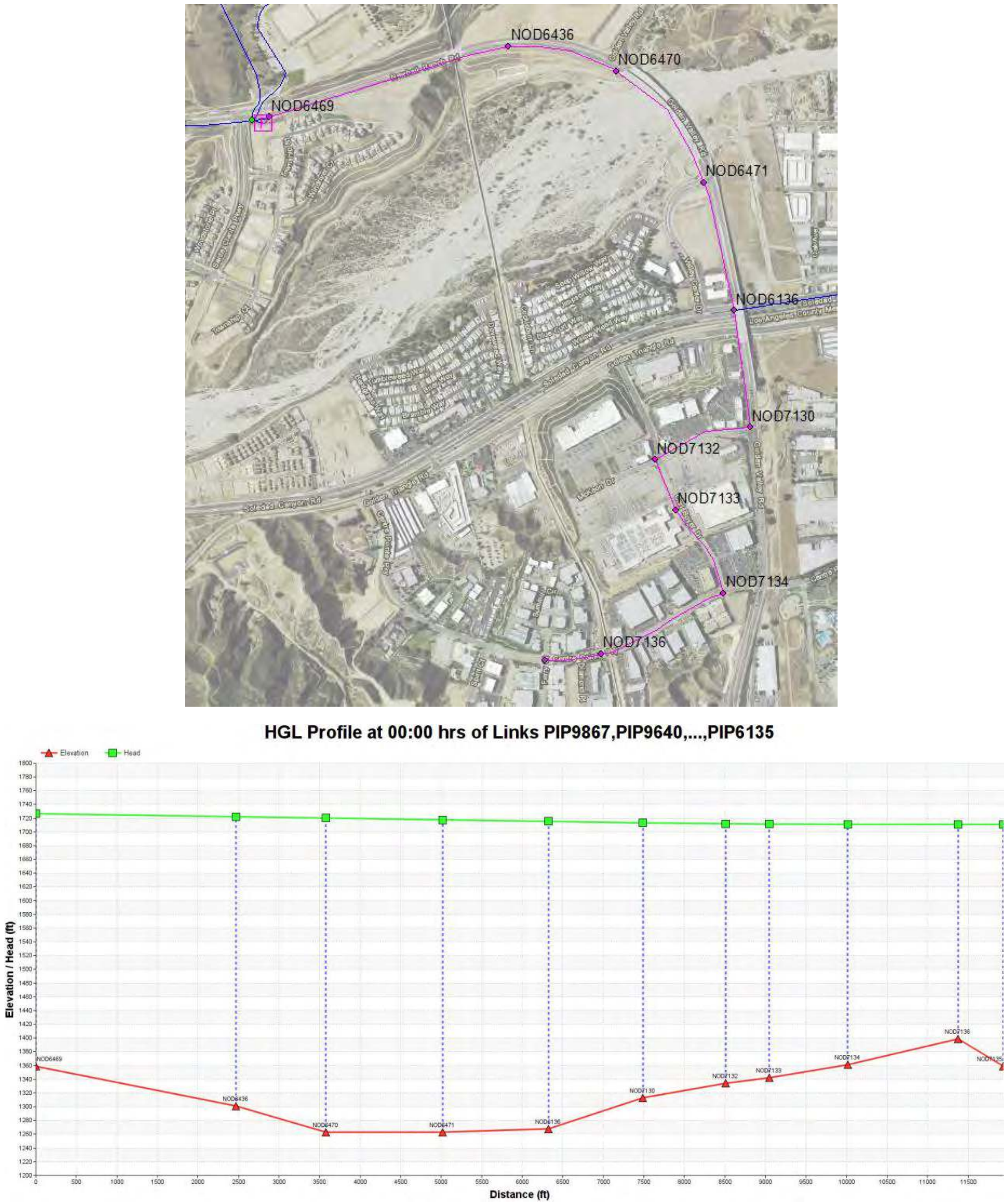
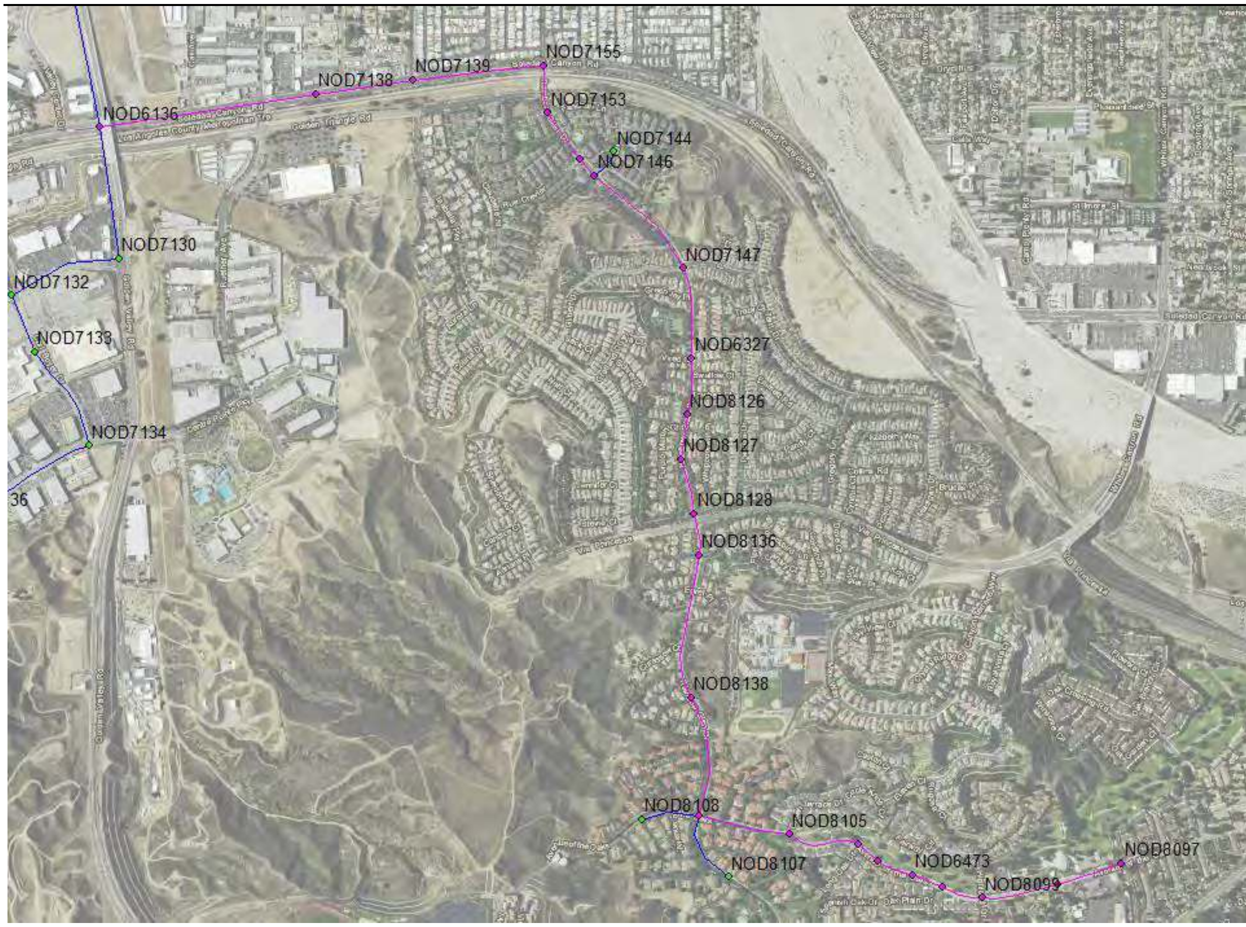


Figure K3 – HGL Profile 2



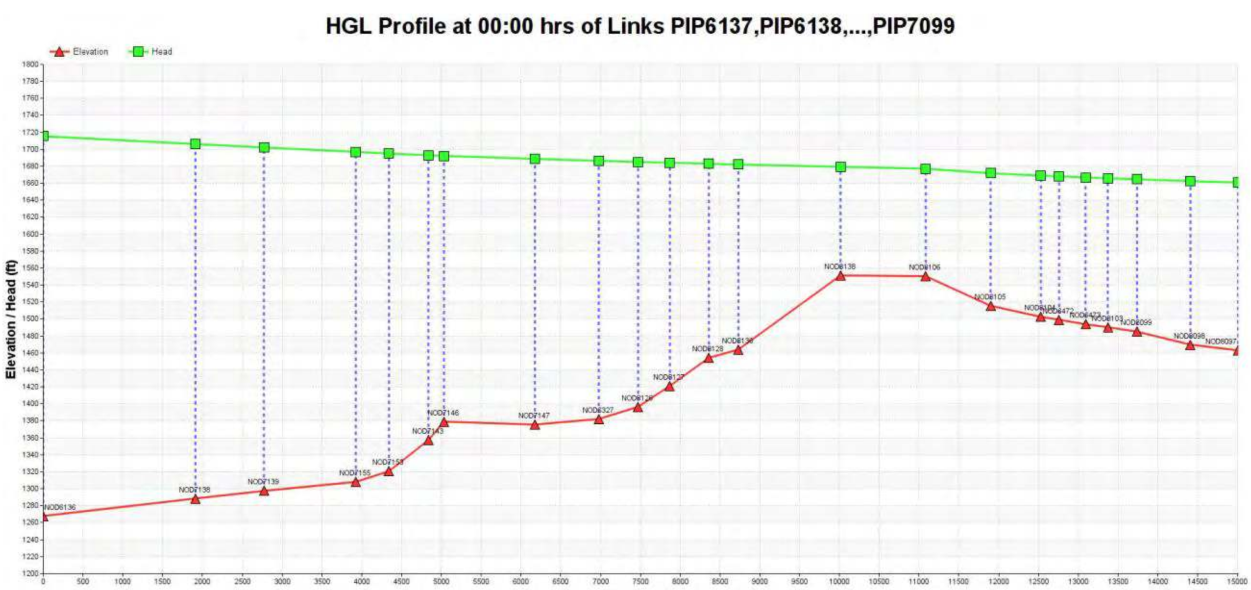
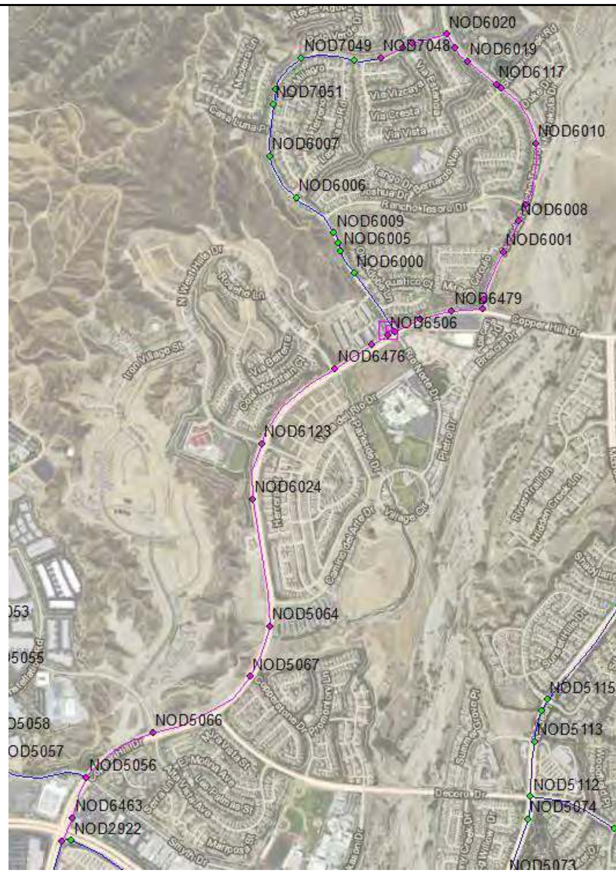


Figure K4 – HGL Profile 3



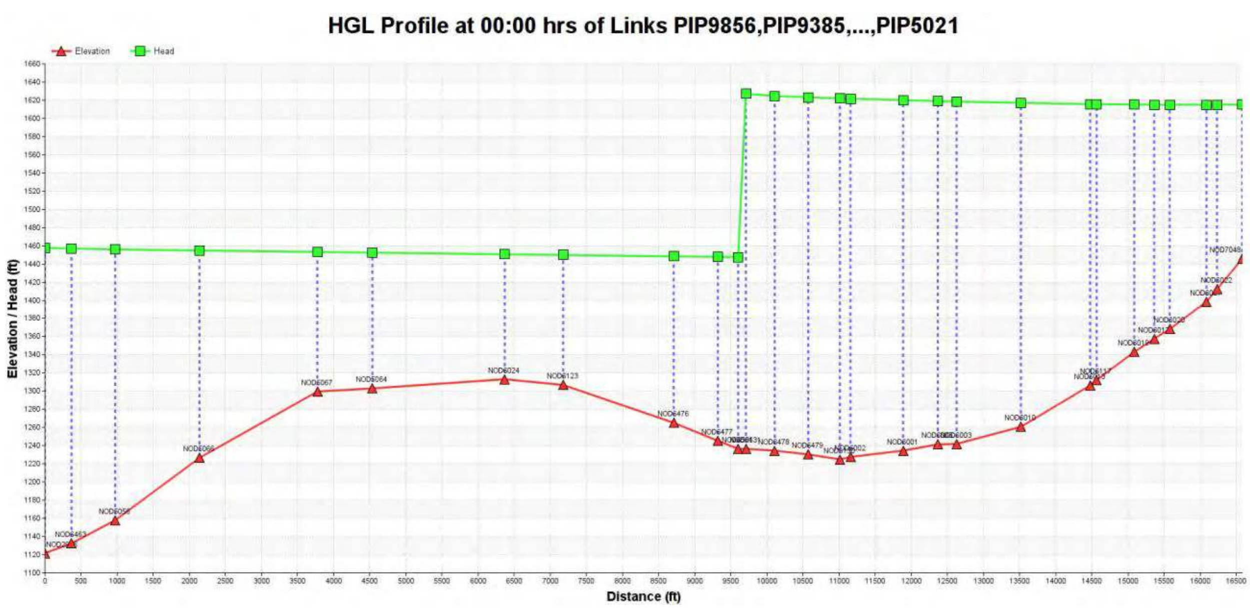


Figure K5 – HGL Profile 4

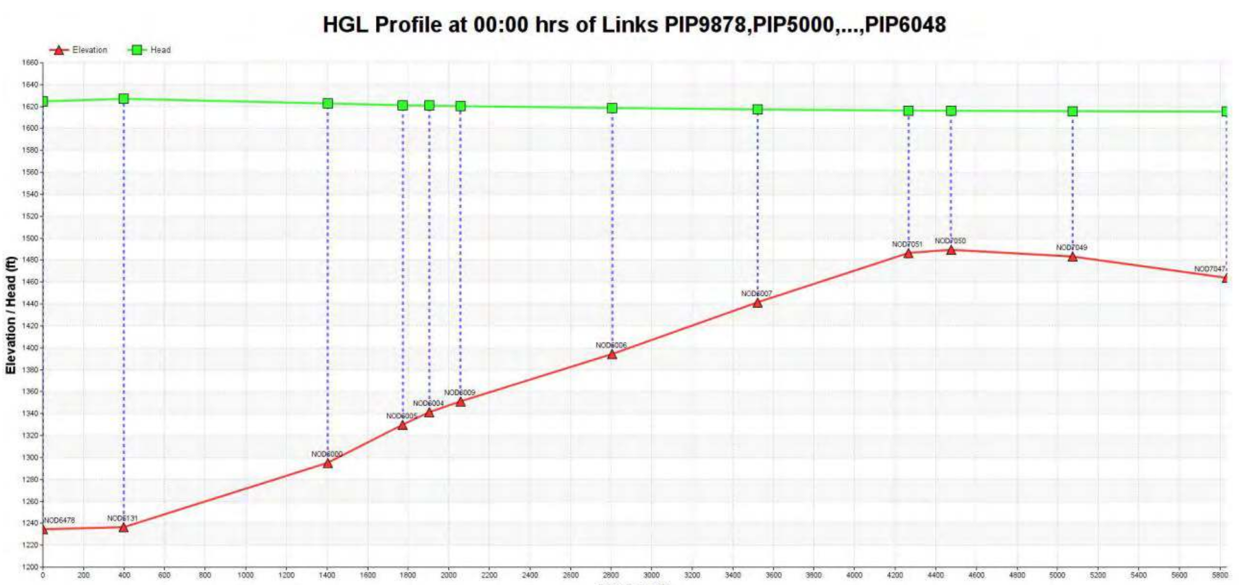


Figure K6 – HGL Profile 5

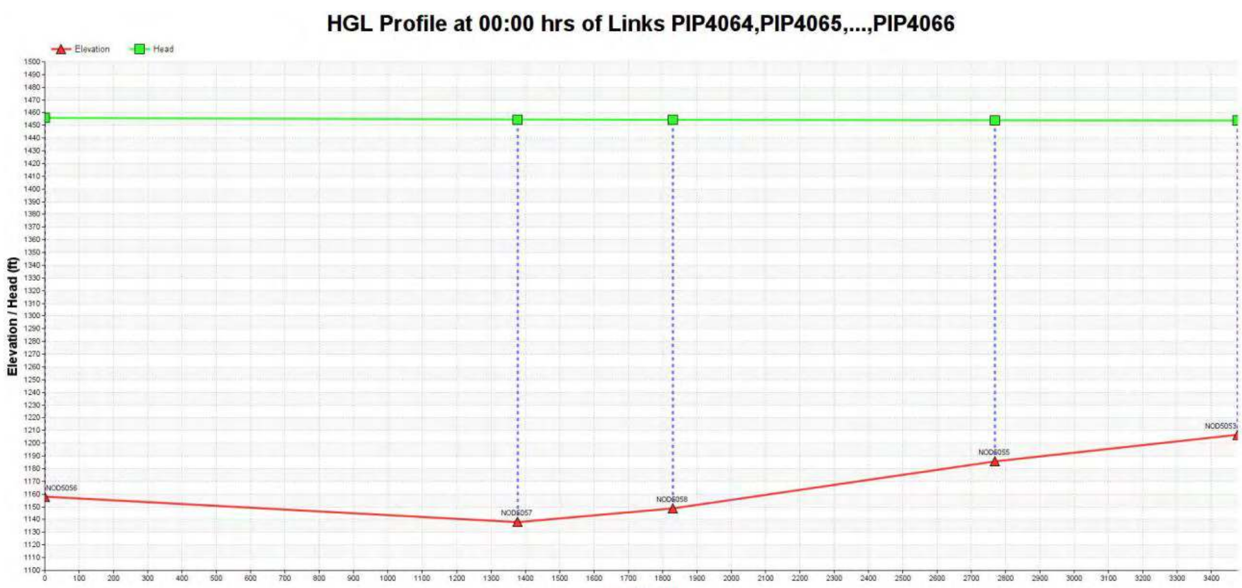
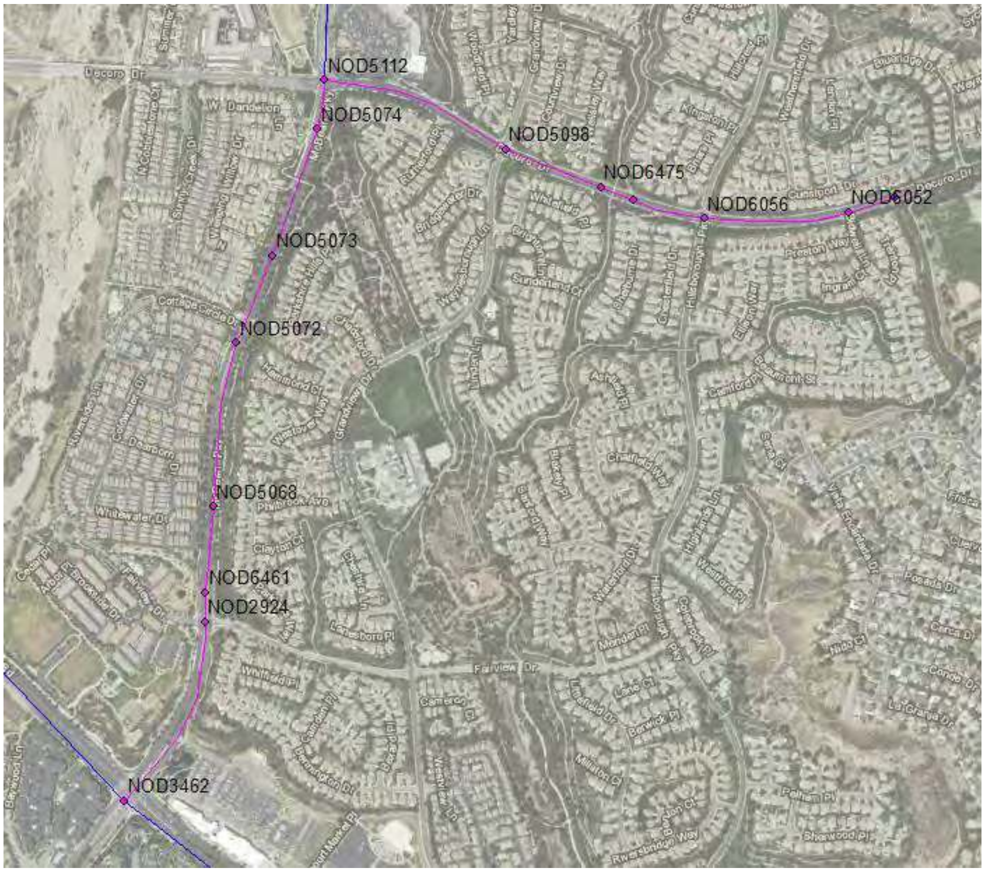


Figure K7 – HGL Profile 6



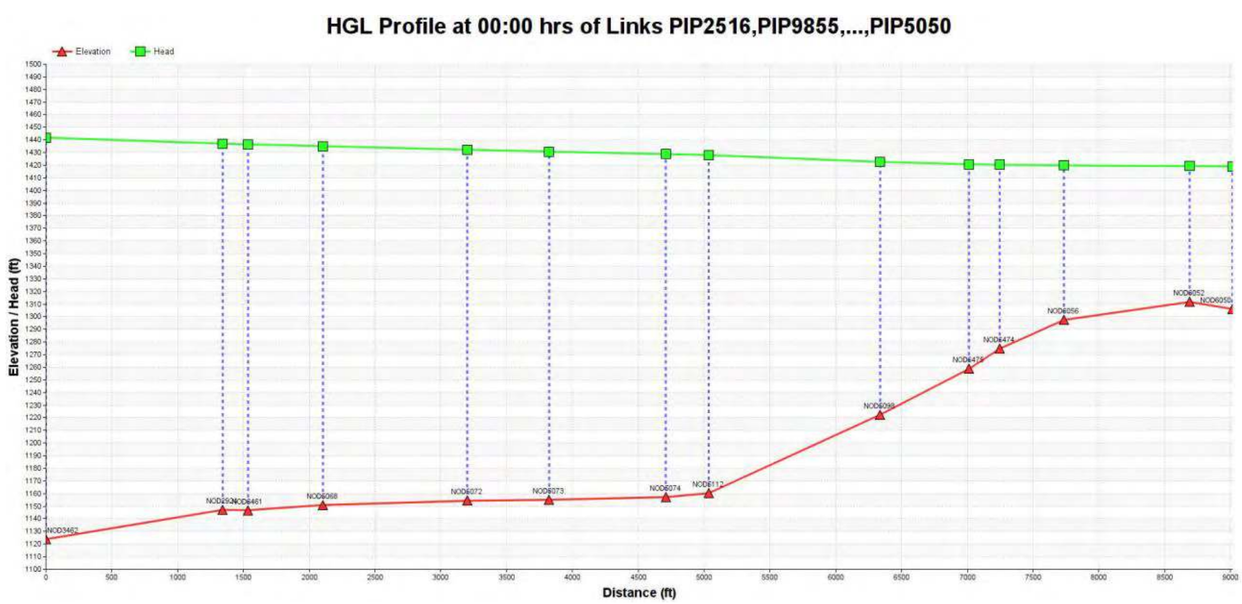


Figure K8 – HGL Profile 7

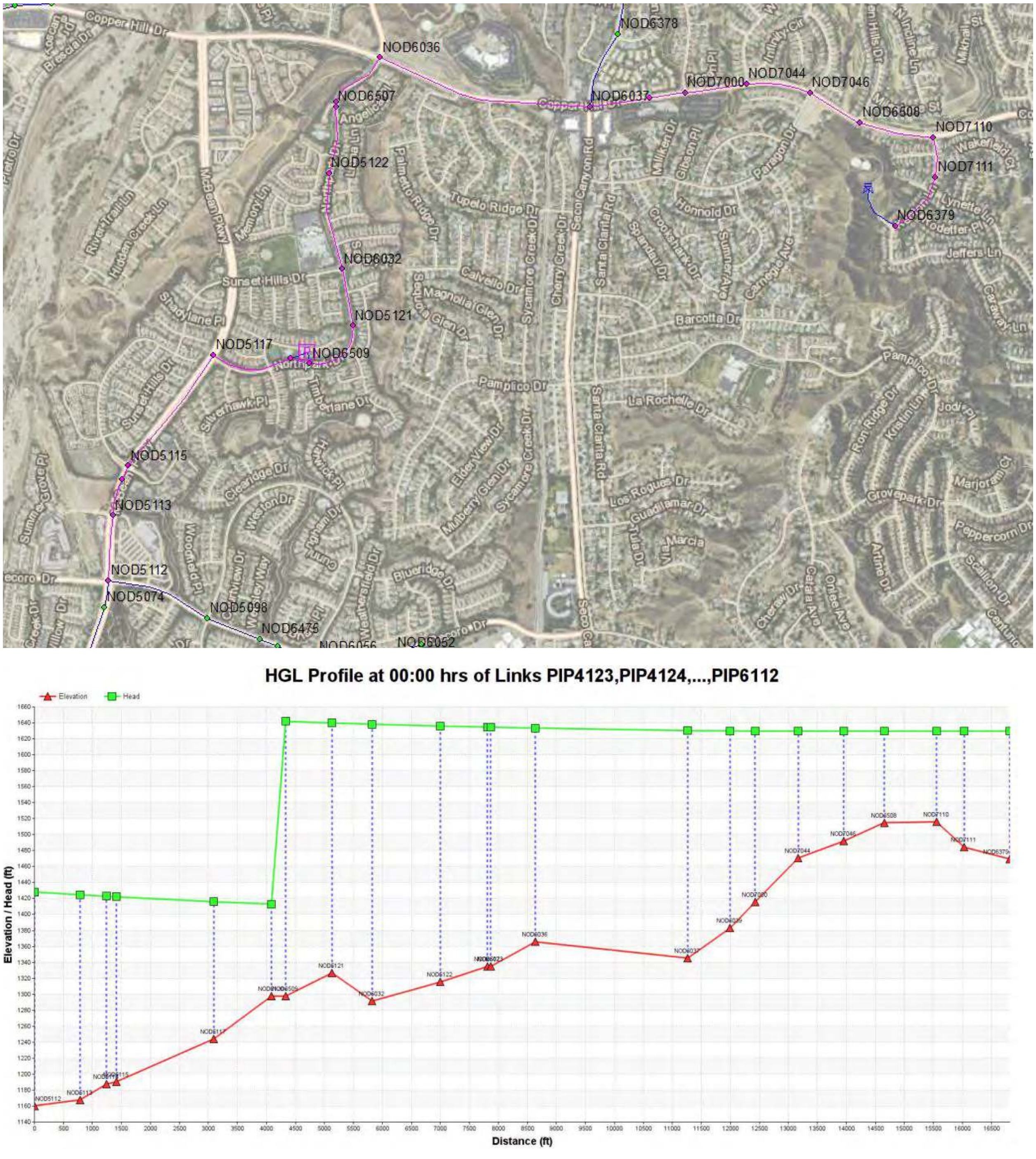


Figure K9 – HGL Profile 8

HGL Profile at 00:00 hrs of Links PIP5036,PIP9397,...,PIP9530

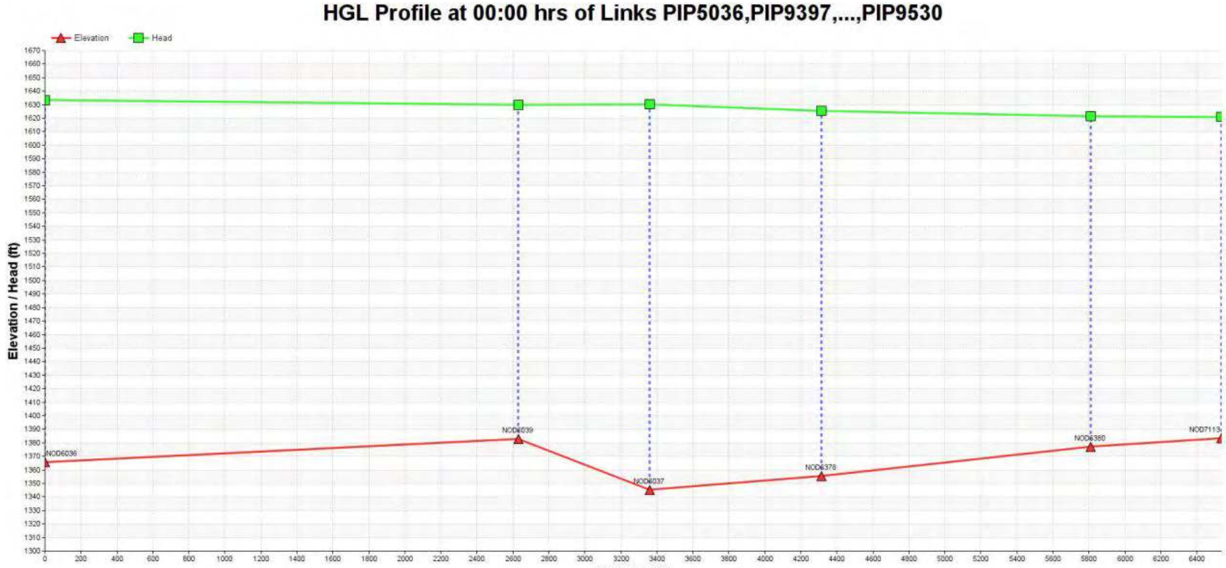


Figure K10 – HGL Profile 9

Castaic Lake Water Agency

Attachment L -
Phase 2C + Future Expansion South Results

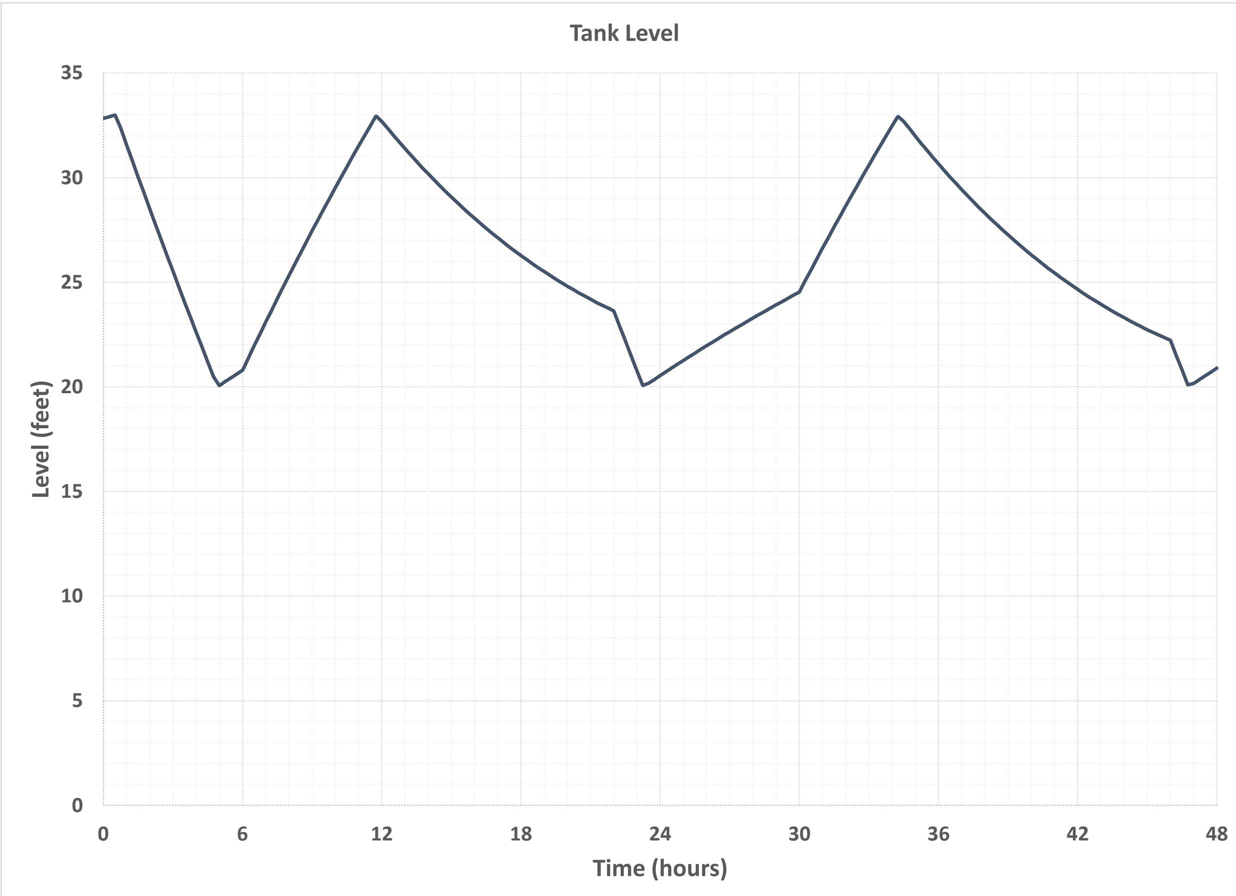


Figure L1 – Existing Tank Level

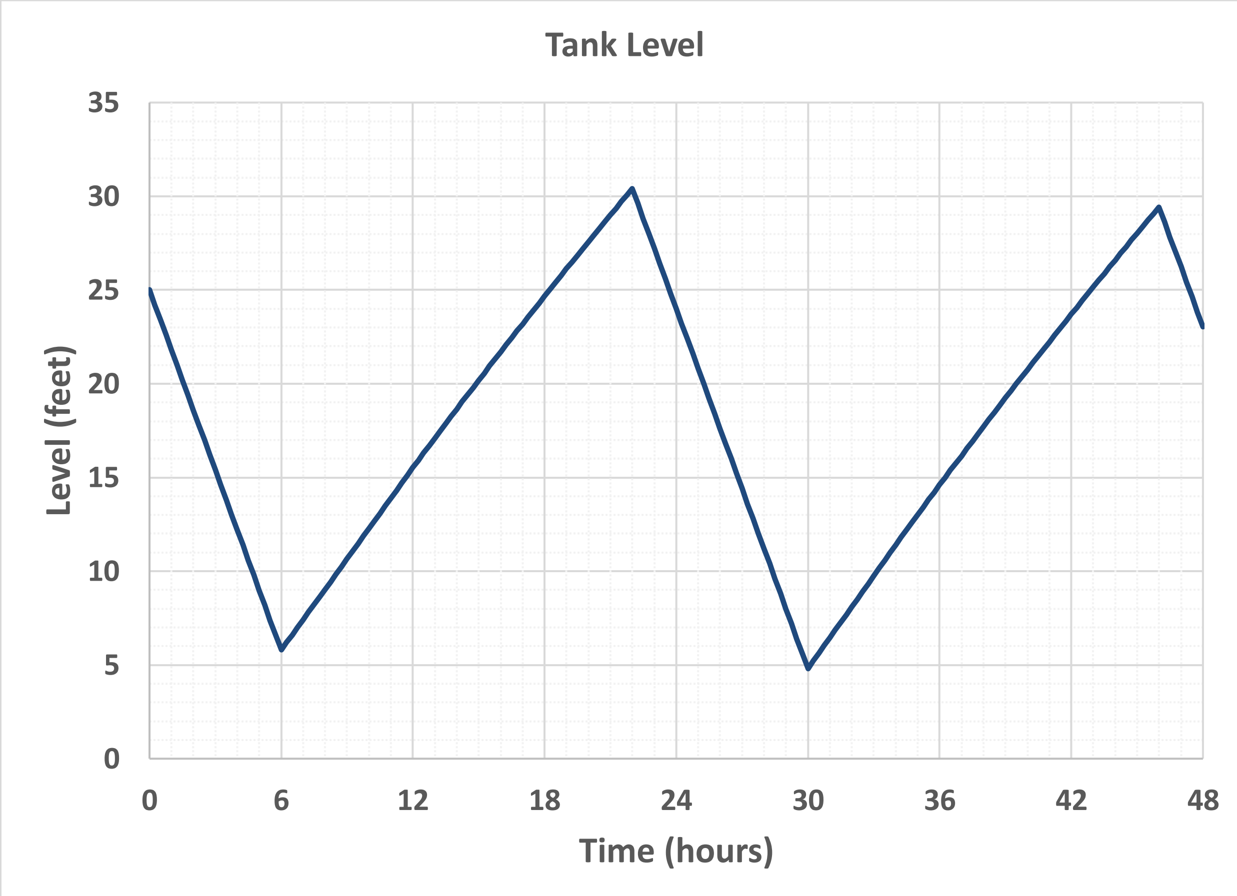


Figure L2 – Proposed Tank Level

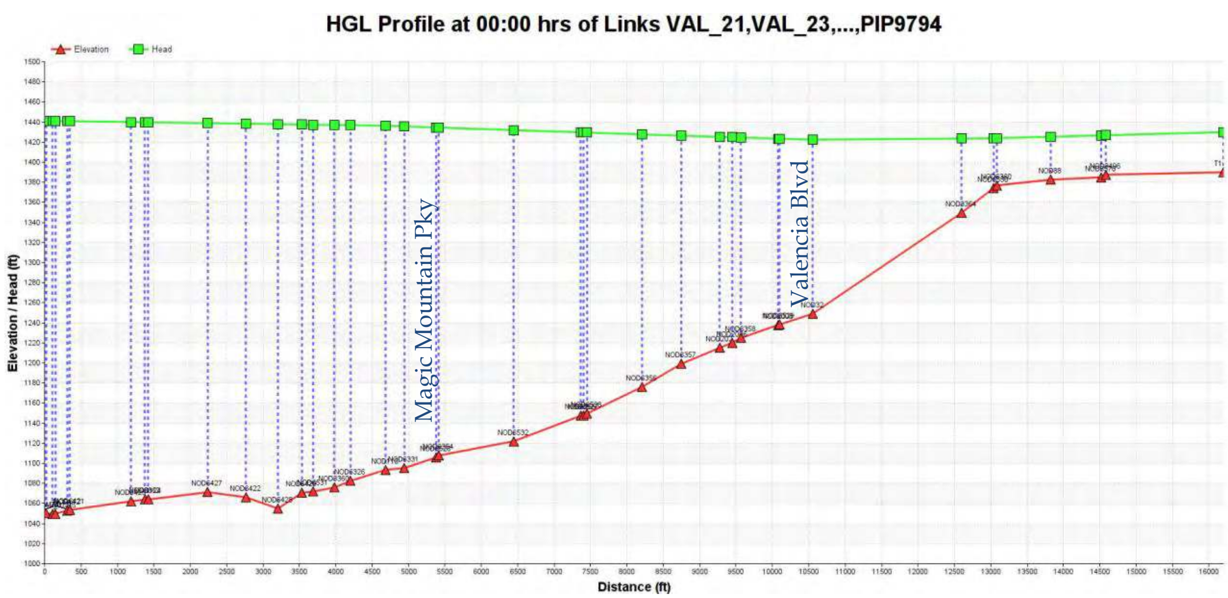


Figure L3 – HGL Profile 1

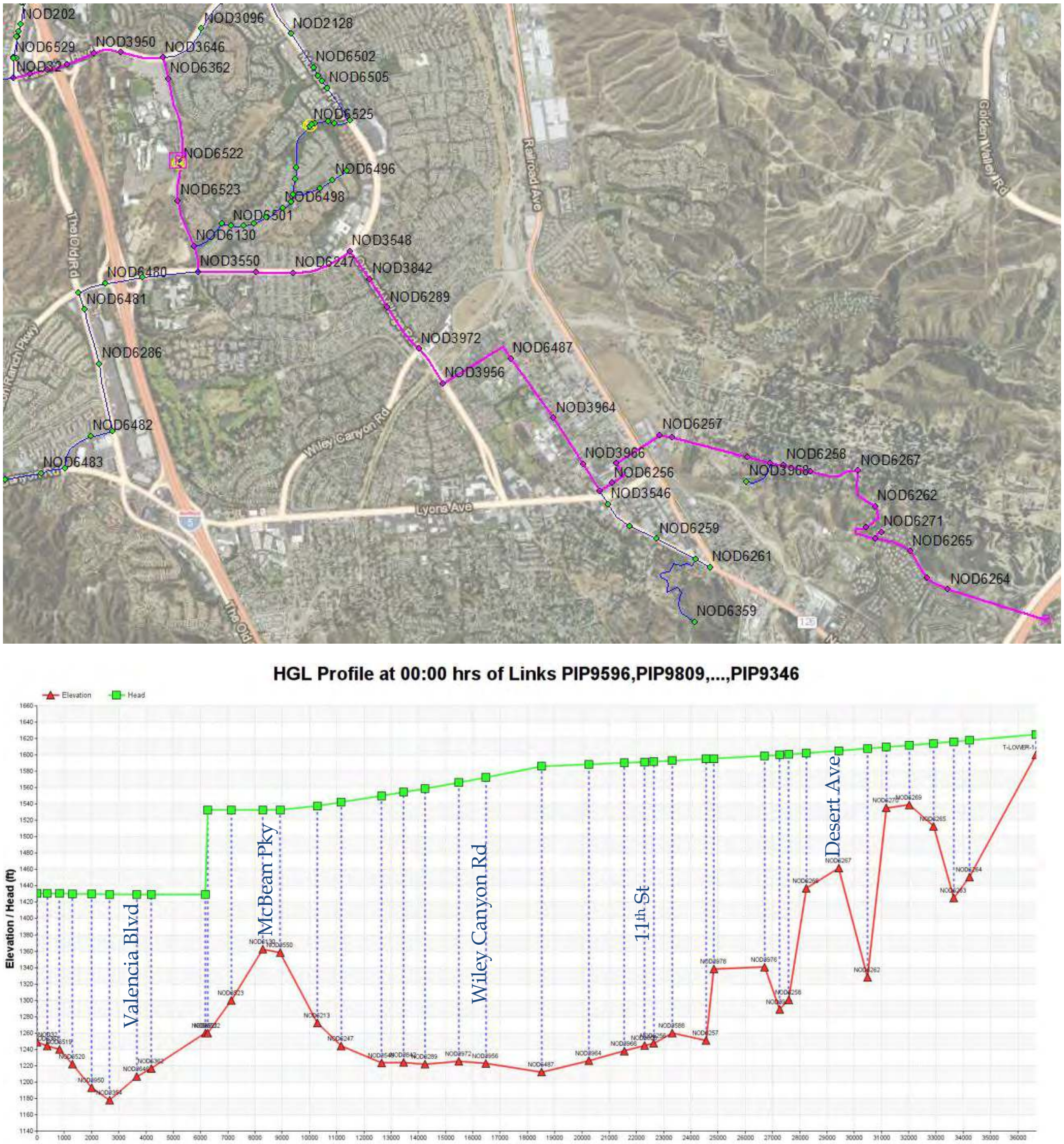


Figure L4 – HGL Profile 2

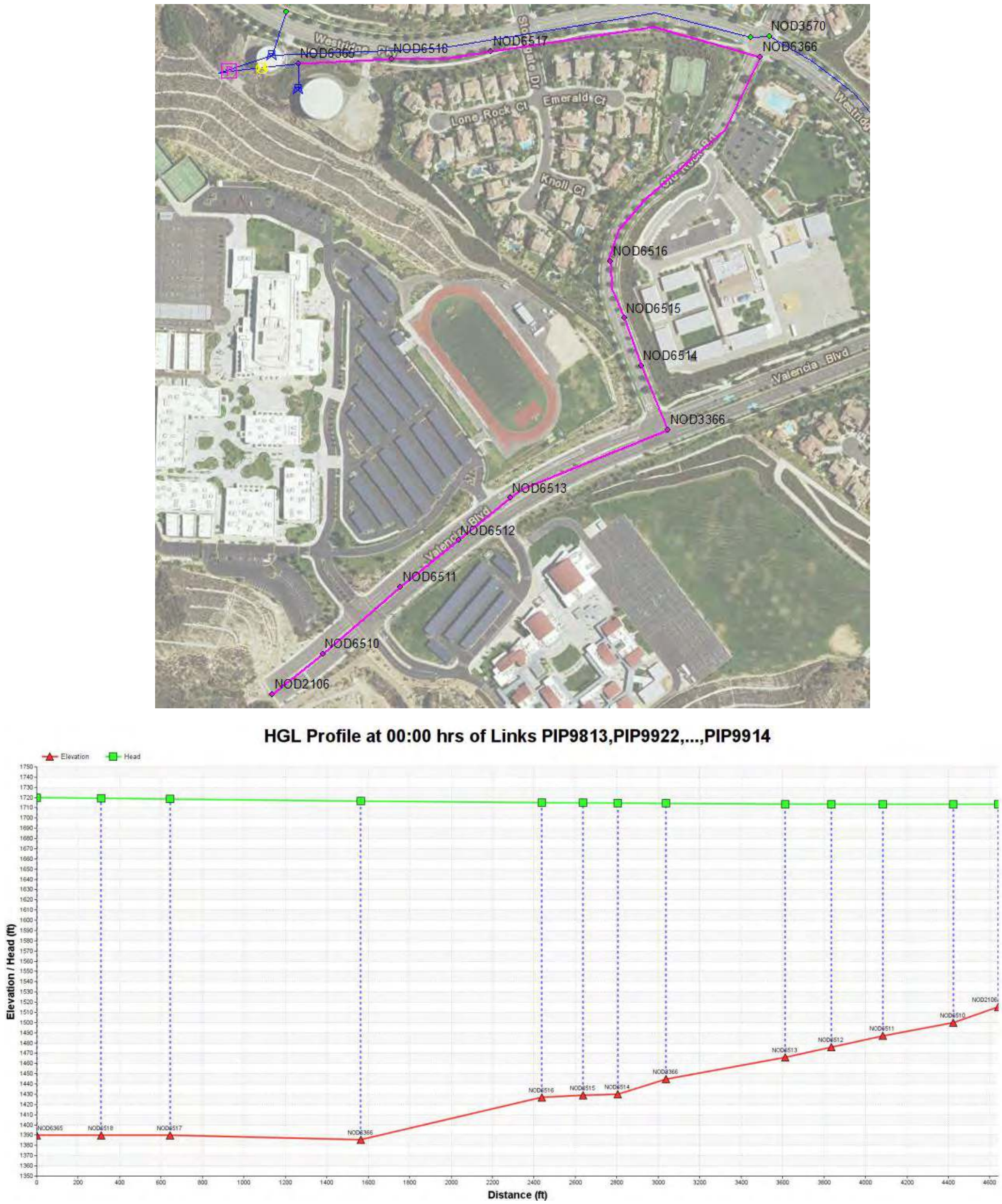


Figure L5 – HGL Profile 3

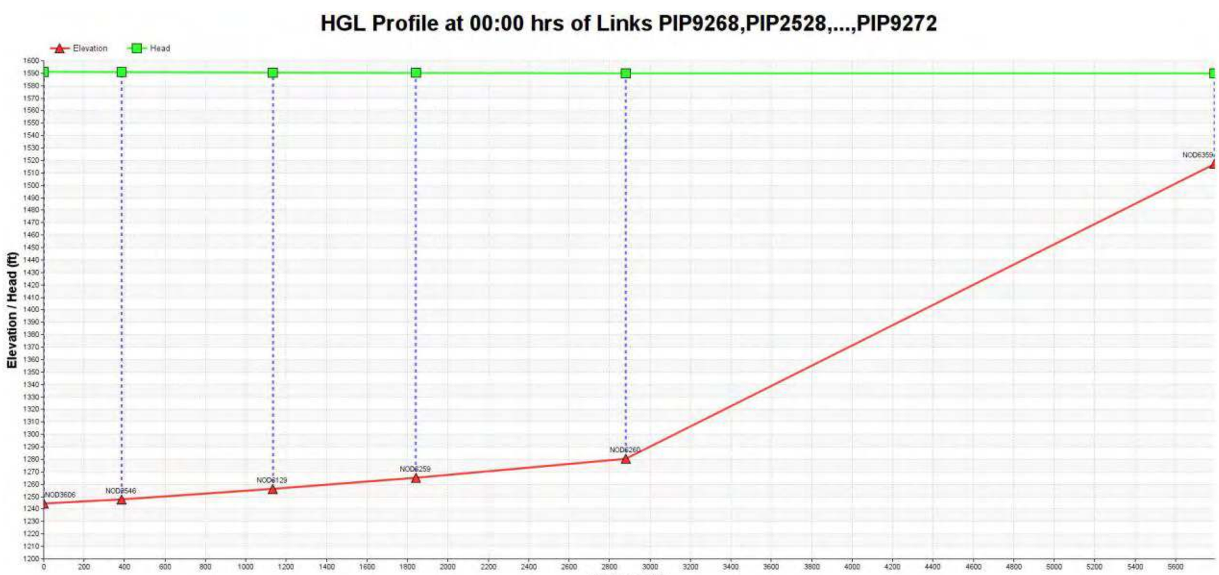
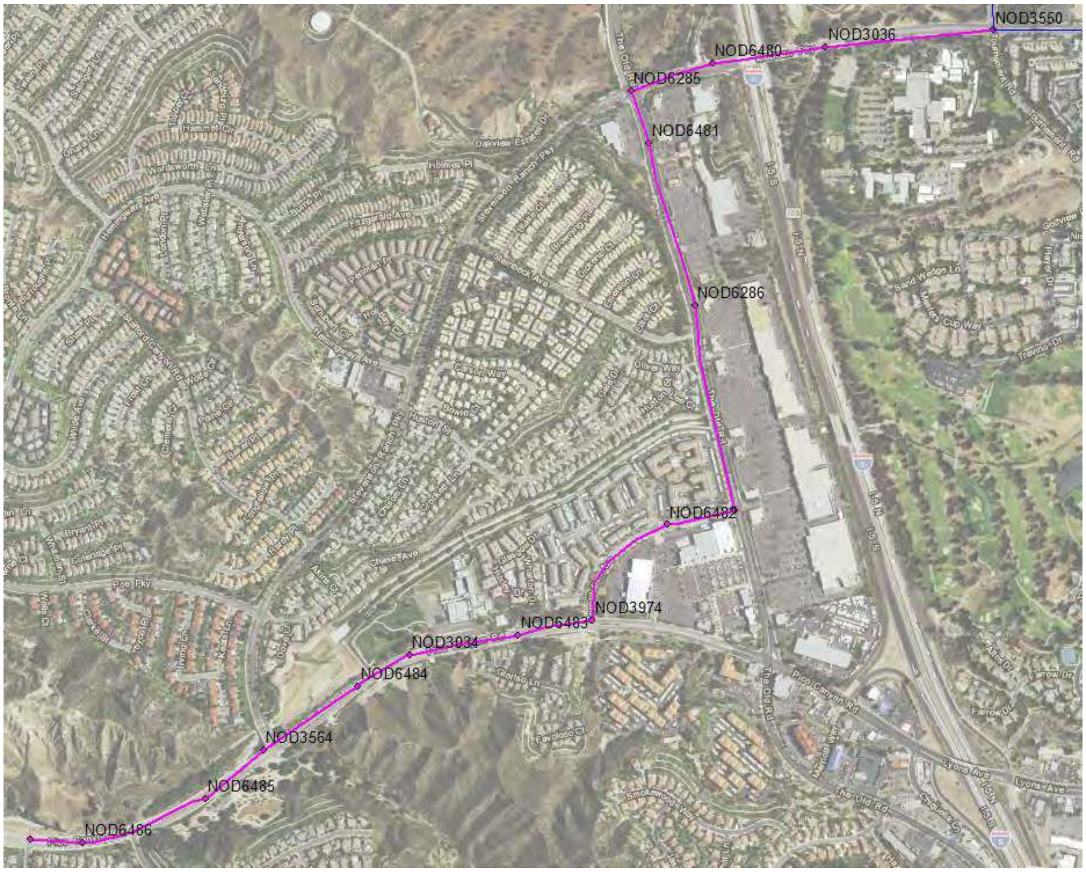


Figure L6 – HGL Profile 4



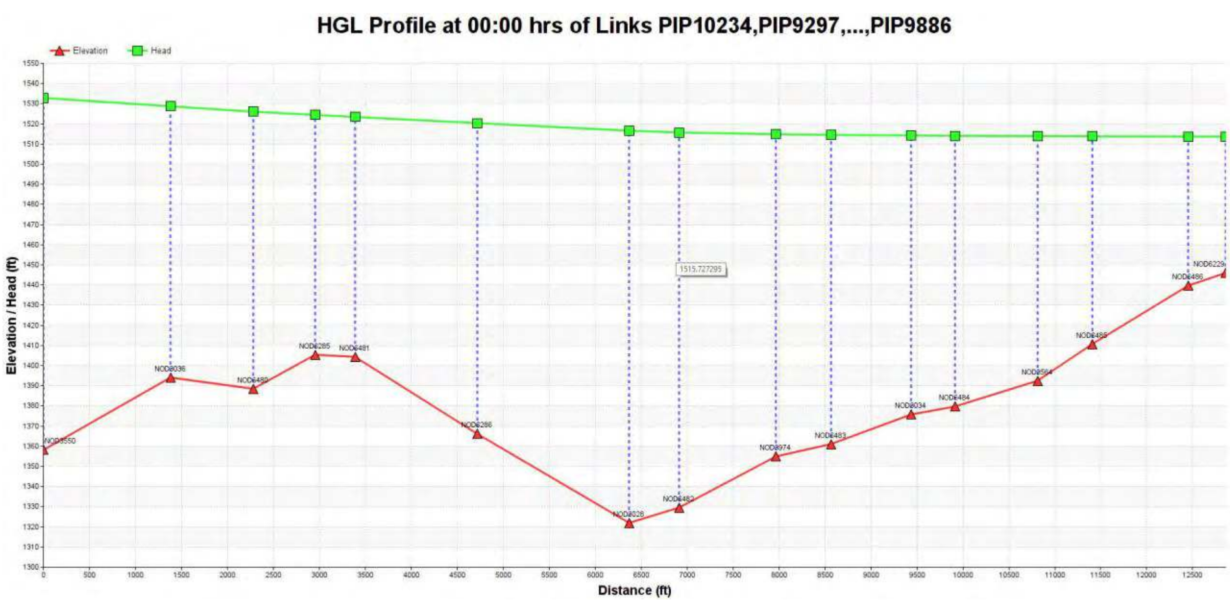
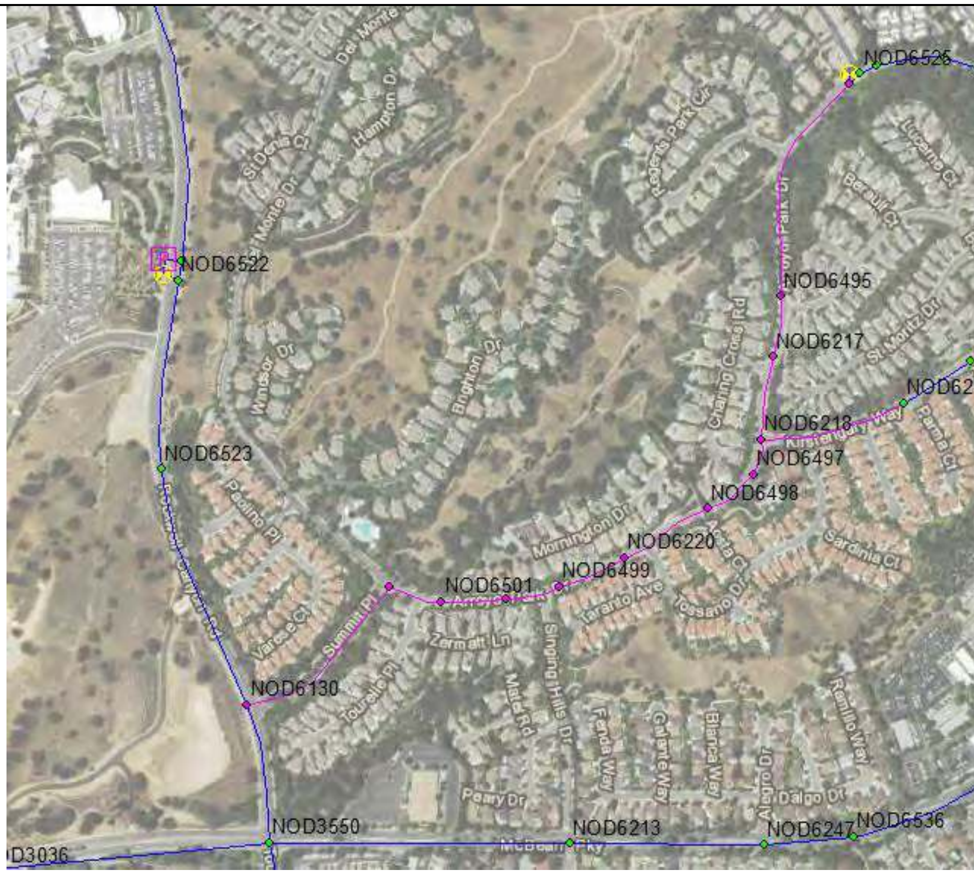


Figure L7 – HGL Profile 5



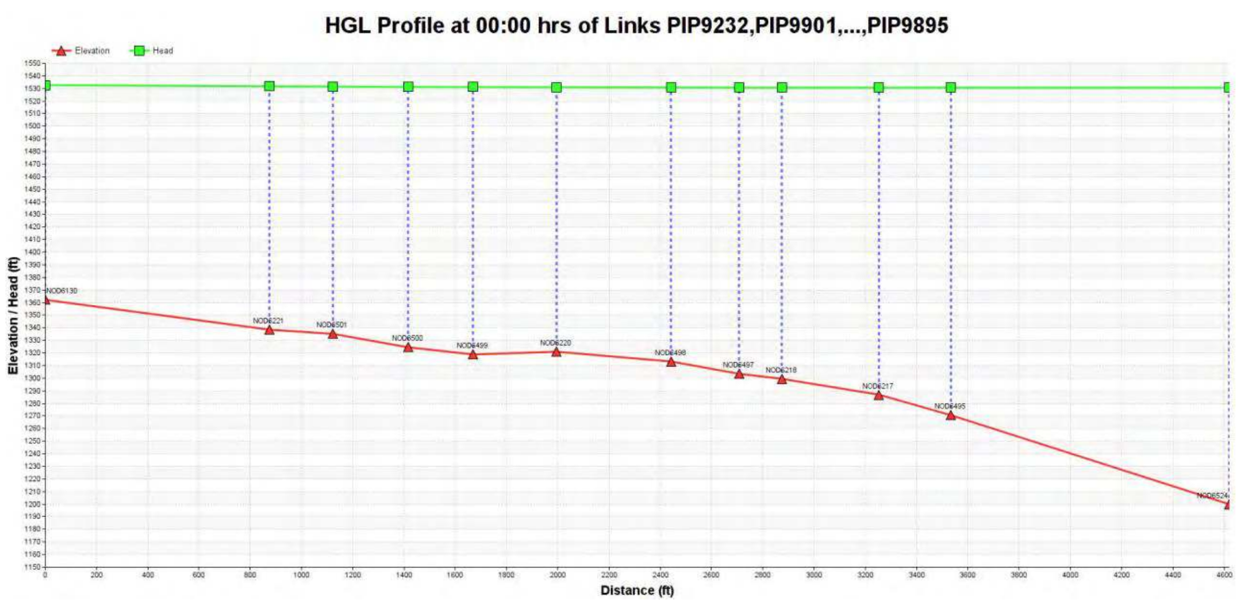


Figure L8 – HGL Profile 6

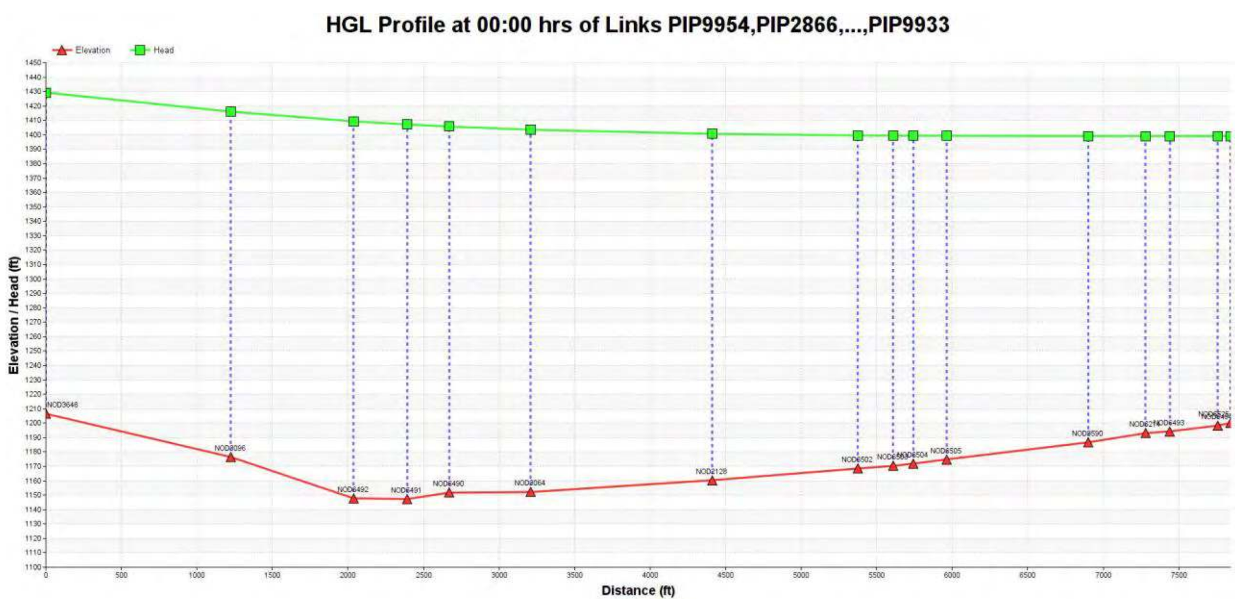


Figure L9 – HGL Profile 7

Appendix E: Engineers Opinion of Probable Costs

This appendix includes detailed cost sheets for the following alternatives and projects:

Alternative 1 - Non-Potable Reuse Expansion (Phase 2): Phase 2A - Bouquet Canyon Road
Alternative 1 - Non-Potable Reuse Expansion (Phase 2): Phase 2A - Central Park South w/o Tank
Alternative 1 - Non-Potable Reuse Expansion (Phase 2): Phase 2A - Central Park South w/ Tank
Alternative 1 - Non-Potable Reuse Expansion (Phase 2): Phase 2B
Alternative 1 - Non-Potable Reuse Expansion (Phase 2): Phase 2C
Alternative 1 - Non-Potable Reuse Expansion (Phase 2): Phase 2D

Alternative 2 - Non-Potable Reuse Expansion (Future Phases): Phase 2A + Future Expansion North
Alternative 2 - Non-Potable Reuse Expansion (Future Phases): Phase 2C + Future Expansion South
Alternative 2 - Non-Potable Reuse Expansion (Future Phases): Westside Communities

Alternative 3 - Groundwater Recharge (Surface Spreading): Phase 2A + Spreading Site #1
Alternative 3 - Groundwater Recharge (Surface Spreading): Phase 2A + Spreading Site #3a
Alternative 3 - Groundwater Recharge (Surface Spreading): Phase 2A + Spreading Site #3b
Alternative 3 - Groundwater Recharge (Surface Spreading): Phase 2A + Spreading Site #3b (Repurpose Infrastructure)
Alternative 3 - Groundwater Recharge (Surface Spreading): Phase 2A + Spreading Sites #1 & #3b (Repurpose Infrastructure)

Alternative 4 - Advanced Treatment for Potable Reuse: Direct Injection
Alternative 4 - Advanced Treatment for Potable Reuse: Surface Water Augmentation
Alternative 4 - Advanced Treatment for Potable Reuse: Direct Potable Reuse + Phase 2A

Engineers Opinion of Probable Cost
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	0.4 mgd
Project:	Phase 2a Boquent Canyon Alignment	**Date Prepared:**	Feb-2016	**RW Delivered:**	482 Annual Irrigation Demand (AFY)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Design Capacity:**	681 Max Day Demand (gpm)
Estimate:	Conceptual-Level	**ENR**	11,155		2,044 Peak Hourly Demand (gpm)

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	8 inch-dia pipeline segments	9,800	lf	112	1,097,600	8 in-diameter $14 per inch-dia-lf
2.2	12 inch-dia pipeline segments	7,000	lf	180	1,260,000	12 in-diameter $15 per inch-dia-lf
2.3	16 inch-dia pipeline segments	14,600	lf	240	3,504,000	16 in-diameter $15 per inch-dia-lf
2.4	Special Crossings (estimated)					
	Bore and Jack Pipe Laying	700	lf	2,640	1,848,000	16 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major Intersections	950	lf	475	451,412	12 in-diameter $40 per inch-dia-lf
3.0	**Pump Stations**					
	Booster PS	1	LS	1,300,000	1,300,000	2,200 total flow (gpm) 450 ft (TDH)
4.0	**Storage**					
	Hydropneumatic Tank	1	LS	200,000	200,000	Recent project experience
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	42	sites	26,000	1,092,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Williams.
	Subtotal Facility Costs				**$10,823,012**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		75,000	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		75,000	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		375,000	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$525,000**	
					$11,348,012	
	Taxes	@	9%		389,628	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		567,401	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		1,702,202	% of Facility Direct Costs
	Estimate Contingency	@	30%		3,404,404	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$17,411,647**	
	Escalation to Midpoint of Construction	@	16%		2,785,863	assume 2% percent over 8
						contrustion start = 2023 end = 2025
	Project Capital Cost Total				**$20,197,510**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 2191 hours operated per year
Energy (conveyance to beneficial use)	510,693	KWh	0.12	61,283	Pump Station Hp = 313 Total Motor HP Required
Energy (other)	26,000	KWh	0.12	3,120	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	0.5	staff	100,000	50,000	full time staff at $100,000 salary per year
Maintenance: Other	482	AF	24	11,697	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		12,610	% of above O&M costs
Recycled Water Purchase (tertiary)	482	AF	200	96,400	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$235,111**	
Annual Unit O&M Costs ($/AFY)				**$488**	

Engineers Opinion of Probable Cost
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	0.5 mgd
Project:	Phase 2a Central Park Alignment without Tank	**Date Prepared:**	Feb-2016	**RW Delivered:**	560 Annual Irrigation Demand (AFY)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Design Capacity:**	792 Max Day Demand (gpm)
Estimate:	Conceptual-Level	**ENR**	11,155		2,376 Peak Hourly Demand (gpm)

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	8 inch-dia pipeline segments	8,900	lf	112	996,800	8 in-diameter $14 per inch-dia-lf
2.2	12 inch-dia pipeline segments	14,800	lf	180	2,664,000	12 in-diameter $15 per inch-dia-lf
2.3	16 inch-dia pipeline segments	14,700	lf	240	3,528,000	16 in-diameter $15 per inch-dia-lf
2.4	Special Crossings (estimate)					
	Bore and Jack Pipe Laying	700	lf	2,640	1,848,000	16 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major Intersections	950	lf	475	451,412	12 in-diameter $40 per inch-dia-lf
3.0	**Pump Stations**					
	Booster PS	1	LS	1,540,000	1,540,000	2,500 total flow (gpm) 490 ft (TDH)
4.0	**Storage**					
	Hydropneumatic Tank	1	LS	200,000	200,000	Recent project experience
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Williams.
	Subtotal Facility Costs				**$12,624,212**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		87,000	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		87,000	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		435,000	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$609,000**	
					$13,233,212	
	Taxes	@	9%		454,472	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		661,661	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		1,984,982	% of Facility Direct Costs
	Estimate Contingency	@	30%		3,969,964	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$20,304,290**	
	Escalation to Midpoint of Construction	@	16%		3,248,686	assume 2% percent over 8
						contrustion start = 2023 end = 2025
	Project Capital Cost Total				**$23,552,976**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 2191 hours operated per year
Energy (conveyance to beneficial use)	631,918	KWh	0.12	75,830	Pump Station Hp = 387 Total Motor HP Required
Energy (other)	32,000	KWh	0.12	3,840	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	0.5	staff	100,000	50,000	full time staff at $100,000 salary per year
Maintenance: Other	560	AF	24	13,595	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		14,327	% of above O&M costs
Recycled Water Purchase (tertiary)	560	AF	200	112,038	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$269,630**	
Annual Unit O&M Costs ($/AFY)				**$481**	

Engineers Opinion of Probable Cost
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	0.5 mgd
Project:	Phase 2a Central Park Alignment with Tank	**Date Prepared:**	Feb-2016	**RW Delivered:**	560 Annual Irrigation Demand (AFY)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Design Capacity:**	792 Max Day Demand (gpm)
Estimate:	Conceptual-Level	**ENR**	11,155		2,376 Peak Hourly Demand (gpm)

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	8 inch-dia pipeline segments	8,900	lf	112	996,800	8 in-diameter $14 per inch-dia-lf
2.2	12 inch-dia pipeline segments	14,900	lf	180	2,682,000	12 in-diameter $15 per inch-dia-lf
2.3	16 inch-dia pipeline segments	14,600	lf	240	3,504,000	16 in-diameter $15 per inch-dia-lf
2.4	Special Crossings (estimate)					
	Bore and Jack Pipe Laying	700	lf	2,640	1,848,000	16 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major Intersections	950	lf	475	451,412	12 in-diameter $40 per inch-dia-lf
3.0	**Pump Stations**					
	Booster PS	1	LS	1,540,000	1,540,000	2,500 total flow (gpm) 490 ft (TDH)
4.0	**Storage**					
	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$13,143,712**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		113,275	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		113,275	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		566,375	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$792,925**	
					$13,936,637	
	Taxes	@	9%		473,174	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		696,832	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		2,090,496	% of Facility Direct Costs
	Estimate Contingency	@	30%		4,180,991	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$21,378,129**	
	Escalation to Midpoint of Construction	@	16%		3,420,501	assume 2% percent over 8
						contrustion start = 2023 end = 2025
	Project Capital Cost Total				**$24,798,630**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3286 hours operated per year
Energy (conveyance to beneficial use)	947,877	KWh	0.12	113,745	Pump Station Hp = 387 Total Motor HP Required
Energy (other)	47,000	KWh	0.12	5,640	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	0.5	staff	100,000	50,000	full time staff at $100,000 salary per year
Maintenance: Other	560	AF	24	13,595	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		18,298	% of above O&M costs
Recycled Water Purchase (tertiary)	560	AF	200	112,038	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$313,316**	
Annual Unit O&M Costs ($/AFY)				**$559**	

APPENDIX E

Engineers Opinion of Probable Cost
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	0.3 mgd
Project:	Phase 2B Vista Canyon Development + SCWD	**Date Prepared:**	Feb-2016	**RW Delivered:**	300 Annual Irrigation Demand (AFY)
RW Supply:	Served by Vista Canyon Water Factory	**K/J Proj. No.**	1544241.00	**Design Capacity:**	424 Max Day Demand (gpm)
Estimate:	Conceptual-Level	**ENR**	11,155		1,272 Peak Hourly Demand (gpm)

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	6 inch-dia Pipelines South of Railroad Tracks	6,100	lf	72	439,200	6 in-diameter $12 per inch-dia-lf
2.2	12 inch-dia Pipelines South of Railroad Tracks	6,600	lf	180	1,188,000	12 in-diameter $15 per inch-dia-lf
2.3	8 inch-dia Pipelines North of Railroad Tracks	10,500	lf		*not incl*	Vista Canyon to pay for all onsite distribution pipeline serving the development
3.0	**Pump Stations**					
	Booster PS	1	LS	370,000	370,000	410 total flow (gpm) 348 ft (TDH)
4.0	**Storage**					
4.1	Storage Tank	1	MG	1,150,000	1,150,000	Recent project experience
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	17	sites	27,000	459,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Williams.
	Subtotal Facility Costs				**$3,606,200**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		76,000	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		76,000	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		380,000	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$532,000**	
					$4,138,200	
	Taxes	@	9%		129,823	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		206,910	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		620,730	% of Facility Direct Costs
	Estimate Contingency	@	30%		1,241,460	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$6,337,123**	
	Escalation to Midpoint of Construction	@	6%		380,227	assume 2% percent over 3
						contrustion start = 2018 end = 2020
	Project Capital Cost Total				**$6,717,351**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3286 hours operated per year
Energy (conveyance to beneficial use)	128,175	KWh	0.12	15,381	Pump Station Hp = 52 Total Motor HP Required
Energy (other)	6,000	KWh	0.12	720	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	0.5	staff	100,000	50,000	full time staff at $100,000 salary per year
Maintenance: Other	163	AF	24	3,959	Based on historical costs for parts, materials, outside service/contracting and other needs for SCWD deliveries only
Contingency	@	10.0%		7,006	% of above O&M costs
Recycled Water Purchase (tertiary)	163	AF	200	32,631	Assume Vista Canyon Water Factory RW rate would be comprable to the avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$109,697**	
Annual Unit O&M Costs ($/AFY)				**$259**	

Engineers Opinion of Probable Cost
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	1.2 mgd
Project:	Phase 2C VWC-NCWD Extensions	**Date Prepared:**	Feb-2016	**RW Delivered:**	1,374 Annual Irrigation Demand (AFY)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Design Capacity:**	1,942 Max Day Demand (gpm)
Estimate:	Conceptual-Level	**ENR**	11,155		5,827 Peak Hourly Demand (gpm)

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	8 inch-dia pipeline segments	8,710	LF	112	975,520	8 in-diameter $14 per inch-dia-lf
2.2	12 inch-dia pipeline segments	5,470	LF	180	984,600	12 in-diameter $15 per inch-dia-lf
2.3	16 inch-dia pipeline segments	7,380	LF	240	1,771,200	16 in-diameter $15 per inch-dia-lf
2.4	20 inch-dia pipeline segments	5,250	LF	300	1,575,000	20 in-diameter $15 per inch-dia-lf
2.5	24 inch-dia pipeline segments	4,130	LF	384	1,585,920	24 in-diameter $16 per inch-dia-lf
2.6	Special Crossings					
	Bore & Jack Pipe Laying	550	LF	2,640	1,452,000	16 in-diameter $165 per inch-dia-lf
	Bore & Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major intersections	500	LF	634	316,780	16 in-diameter $40 per inch-dia-lf
2.7	Replace 12" segment of Old Road with New 24" segment	975	LF	384	374,400	24 in-diameter $16 per inch-dia-lf
3.0	**Pump Stations**					
3.1	New PS at Valencia WRP	1	LS	1,050,000	1,050,000	2,000 total flow (gpm) 380 ft (TDH)
3.2	New PS along Phase 2C	1	LS	1,210,000	1,210,000	5,200 total flow (gpm) 175 ft (TDH)
4.0	**Storage**					
	Storage Tank	0	MG	1,500,000	0	Recent project experience
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	66	sites	27,000	1,782,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Williams.
	Subtotal Facility Costs				**$13,147,420**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		113,000	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		113,000	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		565,000	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$791,000**	
					$13,938,420	
	Taxes	@	9%		473,307	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		696,921	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		2,090,763	% of Facility Direct Costs
	Estimate Contingency	@	30%		4,181,526	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$21,380,938**	
	Escalation to Midpoint of Construction	@	10%		2,138,094	assume 2% percent over 5; contrustion start = 2020 end = 2022
	Project Capital Cost Total				**$23,519,032**	
					Annualized Capital Cost	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	Notes/Source
Energy Costs					Pump Operation = 2191 hours operated per year
Energy (conveyance to beneficial use)	392,047	KWh	0.12	47,046	Pump Station Hp = 240 Total Motor HP Required
Energy (other)	20,000	KWh	0.12	2,400	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	1.5	staff	100,000	150,000	full time staff at $100,000 salary per year
Maintenance: Other	1,374	AF	24	33,343	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		23,279	% of above O&M costs
Recycled Water Purchase (tertiary)	1,374	AF	200	274,791	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$530,859**	
Annual Unit O&M Costs ($/AFY)				**$273**	

Engineers Opinion of Probable Cost
Alternative 1 - Non-Potable Reuse Expansion (Phase 2)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	0.2 mgd
Project:	Phase 2D VWC Extension	**Date Prepared:**	Feb-2016	**RW Delivered:**	186 Annual Irrigation Demand (AFY)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Design Capacity:**	263 Max Day Demand (gpm)
Estimate:	Conceptual-Level	**ENR**	11,155		789 Peak Hourly Demand (gpm)

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
	12 inch-dia pipeline segments	5,200	LF	180	936,000	12 in-diameter $15 per inch-dia-lf
3.0	**Pump Stations**					
	Booster PS	1	LS	590,000	590,000	1,000 total flow (gpm) 350 ft (TDH)
4.0	**Storage**					
	Storage Tank	0	MG	1,500,000	0	Recent project experience
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	14	sites	25,000	350,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Williams.
	Subtotal Facility Costs				**$1,876,000**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		29,500	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		29,500	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		147,500	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$206,500**	
					$2,082,500	
	Taxes	@	9%		67,536	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		104,125	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		312,375	% of Facility Direct Costs
	Estimate Contingency	@	30%		624,750	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$3,191,286**	
	Escalation to Midpoint of Construction	@	4%		127,651	assume 2% percent over 2; contrustion start = 2017 end = 2019
	Project Capital Cost Total				**$3,318,937**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 2191 hours operated per year
Energy (conveyance to beneficial use)	180,548	KWh	0.12	21,666	Pump Station Hp = 110 Total Motor HP Required
Energy (other)	9,000	KWh	0.12	1,080	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	0.5	staff	100,000	50,000	full time staff at $100,000 salary per year
Maintenance: Other	186	AF	24	4,514	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		7,726	% of above O&M costs
Recycled Water Purchase (tertiary)	186	AF	200	37,200	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$122,186**	
Annual Unit O&M Costs ($/AFY)				**$656.91**	

Engineers Opinion of Probable Cost
Alternative 2 - Non-Potable Reuse Expansion (Future Phases)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	1.7 mgd
Project:	Includes Phase 2A and Future Expansion (Alignments E-H)North of the Santa Clara River	**Date Prepared:**	Feb-2016	**Phase 2A RW Delivered:**	560 AFY (Irrigation)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Alignment E-H RW Delivered:**	1,344 AFY (Irrigation)
Estimate:	Conceptual-Level	**ENR**	11,155		

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
	Phase 2A Pipelines (Upsized)					
2.1	8 inch-dia pipeline segments	7,150	lf	112	800,800	8 in-diameter $14 per inch-dia-lf
2.2	12 inch-dia pipeline segments	870	lf	180	156,600	12 in-diameter $15 per inch-dia-lf
2.3	16 inch-dia pipeline segments	1,710	lf	240	410,400	16 in-diameter $15 per inch-dia-lf
2.4	24 inch-dia pipeline segments	28,600	lf	384	10,982,400	24 in-diameter $16 per inch-dia-lf
2.5						
2.6	Special Crossings (estimate)					
	Bore and Jack Pipe Laying	700	lf	3,960	2,772,000	24 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major Intersections	950	lf	950	902,824	24 in-diameter $40 per inch-dia-lf
	Future Alignments E-H					
2.7	Alignment E - Rio Norte Jr High, Tesoro Del Valle Recreation Center	16,070	lf	112	1,799,840	8 in-diameter $14 per inch-dia-lf
		330	lf	180	59,400	12 in-diameter $15 per inch-dia-lf
		9,230	lf	240	2,215,200	16 in-diameter $15 per inch-dia-lf
2.8	Alignment F - Arroyo Secco Middle School	3,980	lf	112	445,760	8 in-diameter $14 per inch-dia-lf
		3,690	lf	240	885,600	16 in-diameter $15 per inch-dia-lf
2.9	Alignment G - Northpark Elementary School, Mountain View Park	9,330	lf	112	1,044,960	8 in-diameter $14 per inch-dia-lf
		11,620	lf	180	2,091,600	12 in-diameter $15 per inch-dia-lf
3.0	Alignment H - SCWD Office, La Mesa Middle School, Friendly Valley Golf Course	10,990	lf	112	1,230,880	8 in-diameter $14 per inch-dia-lf
		11,080	lf	180	1,994,400	12 in-diameter $15 per inch-dia-lf
		6,500	lf	240	1,560,000	16 in-diameter $15 per inch-dia-lf
		210	lf	384	80,640	24 in-diameter $16 per inch-dia-lf
3.0	**Pump Stations**					
3.1	New PS at Valencia WRP	1	LS	3,700,000	3,700,000	8,000 total flow (gpm) 430 ft (TDH)
3.2	New PS along Alignment E	1	LS	410,000	410,000	1,100 total flow (gpm) 180 ft (TDH)
3.3	New PS along Alignment G	1	LS	450,000	450,000	1,000 total flow (gpm) 230 ft (TDH)
3.4	New PS along Alignment H	1	LS	810,000	810,000	1,900 total flow (gpm) 285 ft (TDH)
4.0	**Storage**					
	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
5.0	**Site Retrofit Costs**					
	Phase 2A - Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by
	Alignments E-H - Based on number and size of sites	161	sites	25,400	4,089,400	Dexter Williams.
	Subtotal Facility Costs				**$41,014,204**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		304,775	% of Subtotal treatment, pump station, storage and discharge costs
						(Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		304,775	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		1,523,875	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$2,133,425**	
					$43,147,629	
	Taxes	@	9%		1,476,511	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		2,157,381	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		6,472,144	% of Facility Direct Costs
	Estimate Contingency	@	30%		12,944,289	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$66,197,955**	
	Escalation to Midpoint of Construction	@	16%		10,591,673	assume 2% percent over 8
						contrustion start = 2023 end = 2025
	Project Capital Cost Total				**$76,789,628**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3286 hours operated per year
Energy: New PS at Valencia WRP	2,661,794	KWh	0.12	319,415	Pump Station Hp = 1086 Total Motor HP Required
Energy: New PS along Alignment E	925,438	KWh	0.12	111,053	Pump Station Hp = 378 Total Motor HP Required
Energy: New PS along Alignment H	0	KWh	0.12	0	Pump Station Hp = 0 Total Motor HP Required
Energy (other)	179,000	KWh	0.12	21,480	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	1.5	staff	100,000	150,000	full time staff at $100,000 salary per year
Maintenance: Other	1,904	AF	24	46,209	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		64,816	% of above O&M costs
Recycled Water Purchase (tertiary)	1,904	AF	200	380,818	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$1,093,790**	
Annual Unit O&M Costs ($/AFY)				**$574**	

Engineers Opinion of Probable Cost
Alternative 2 - Non-Potable Reuse Expansion (Future Phases)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	2.1 mgd
Project:	Includes Phase 2C and Future Expansion (Alignments A-D)South of the Santa Clara River	**Date Prepared:**	Feb-2016	**Phase 2C RW Delivered:**	1,374 AFY (Irrigation)
RW Supply:	Served by Valencia WRP	**K/J Proj. No.**	1544241.00	**Alignment A-D RW Delivered:**	1,017 AFY (Irrigation)
Estimate:	Conceptual-Level	**ENR**	11,155		

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
	Existing Pipelines (no change)					The increased demand may require more capacity in the existing Phase 1 pipelines then presently available, but additional costs are not added at this time
	Phase 2C Pipelines (assume same as for Alt 1 - Phase 2C)					
2.1	8 inch-dia pipeline segments	8,710	LF	112	975,520	8 in-diameter $14 per inch-dia-lf
2.2	16 inch-dia pipeline segments	8,710	LF	240	2,090,400	16 in-diameter $15 per inch-dia-lf
2.3	16 inch-dia pipeline segments	7,380	LF	240	1,771,200	16 in-diameter $15 per inch-dia-lf
2.4	20 inch-dia pipeline segments	5,250	LF	300	1,575,000	20 in-diameter $15 per inch-dia-lf
2.5	24 inch-dia pipeline segments	4,130	LF	384	1,585,920	24 in-diameter $16 per inch-dia-lf
2.6	Special Crossings					
	Bore & Jack Pipe Laying	550	LF	3,960	2,178,000	24 in-diameter $165 per inch-dia-lf
	Bore & Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major intersections	500	LF	950	475,171	24 in-diameter $40 per inch-dia-lf
2.7	Replace 12" segment of Old Road with New 24" segment	975	LF	384	374,400	24 in-diameter $16 per inch-dia-lf
	Future Alignments A-D					
2.8	Alignment A - The Master's College	820	lf	180	147,600	12 in-diameter $15 per inch-dia-lf
		220	lf	240	52,800	16 in-diameter $15 per inch-dia-lf
		15,120	lf	384	5,806,080	24 in-diameter $16 per inch-dia-lf
2.9	Alignment B - William S Hart Park	6,190	lf	180	1,114,200	12 in-diameter $15 per inch-dia-lf
3.0	Alignment C - Pico Canyon Park, Pico Canyon Elementary School, Valer	12,860	lf	112	1,440,320	8 in-diameter $14 per inch-dia-lf
3.1	Alignment D - Santa Clarita City Hall	11,920	lf	112	1,335,040	8 in-diameter $14 per inch-dia-lf
		5,730	lf	180	1,031,400	12 in-diameter $15 per inch-dia-lf
3.0	**Pump Stations**					
3.1	Phase 2C Booster PS with increased flow for A-D	1	LS	3,700,000	3,700,000	2,000 total flow (gpm) 380 ft (TDH)
3.2	New PS along Phase 2C	1	LS	1,510,000	1,510,000	5,200 total flow (gpm) 230 ft (TDH)
4.0	**Storage**					
4.1	Lower Storage Tank 1 (1600 ft elevation)	5	MG	1,000,000	5,000,000	RS Means 2015 Water Storage Tank Construction Cost
5.0	**Site Retrofit Costs**					
	Phase 2C - Based on number and size of sites	66	sites	27,000	1,782,000	Unit cost based on retrofit cost curve developed from VWC study by
	Alignments A-D - Based on number and size of sites	93	sites	27,500	2,557,500	Dexter Williams.
	Subtotal Facility Costs				**$36,572,551**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		510,500	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		510,500	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		2,552,500	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$3,573,500**	
					$40,146,051	
	Taxes	@	9%		1,316,612	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		2,007,303	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		6,021,908	% of Facility Direct Costs
	Estimate Contingency	@	30%		12,043,815	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$61,535,688**	
	Escalation to Midpoint of Construction	@	16%		9,845,710	assume 2% percent over 8
						contrustion start = 2023 end = 2025
	Project Capital Cost Total				**$71,381,398**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3286 hours operated per year
Energy: Phase 2C Booster PS with increased flow for A-D	2,451,326	KWh	0.12	294,159	Pump Station Hp = 1000 Total Motor HP Required
Energy: New PS along Phase 2C	925,438	KWh	0.12	111,053	Pump Station Hp = 378 Total Motor HP Required
Energy:	0	KWh	0.12	0	Pump Station Hp = 0 Total Motor HP Required
Energy (other)	169,000	KWh	0.12	20,280	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	1.5	staff	100,000	150,000	full time staff at $100,000 salary per year
Maintenance: Other	2,391	AF	24	58,036	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		63,353	% of above O&M costs
Recycled Water Purchase (tertiary)	2,391	AF	200	478,289	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$1,175,170**	
Annual Unit O&M Costs ($/AFY)				**$491**	

Engineers Opinion of Probable Cost
Alternative 2 - Non-Potable Reuse Expansion (Future Phases)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	6.4 mgd
Project:	Westside Communities	**Date Prepared:**	Feb-2016	**RW Delivered:**	7,184 AFY (Irrigation)
RW Supply:	Served by Valencia WRP and Newhall WRP	**K/J Proj. No.**	1544241.00		
Estimate:	Conceptual-Level	**ENR**	11,155		

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						**Source of Facility Sizing Info = Nov 2015 RWMP Revision for Westside Communities**
1.0	**Treatment Facility (not included)**					
	Approx 50% of demand met by Valencia WRP					Assume purchase of tertiary RW at same rate as other alts
	Approx 50% of demand met by Newhall Ranch WRP					Assume purchase of tertiary RW at same rate as other alts
2.0	**Pipelines**					**Source: Nov 2015 RWMP Revision for Westside Communities (App A Peak Demand Output)**
2.1	8 inch-dia pipeline segments	91	LF	510	46,466	30 in-diameter $17 per inch-dia-lf
2.2	12 inch-dia pipeline segments	59,919	LF	384	23,008,858	24 in-diameter $16 per inch-dia-lf
2.3	16 inch-dia pipeline segments	19,382	LF	270	5,233,216	18 in-diameter $15 per inch-dia-lf
2.4	Total inch-dia pipeline segments	81,859	LF	180	14,734,687	12 in-diameter $15 per inch-dia-lf
2.4	Special Crossings					
	assumed as a percent of pipeline cost	1	LS	4,302,323	4,302,323	10% of pipeline costs assumed to respresent special crossings
3.0	**Pump Stations**					**Source: Nov 2015 RWMP Revision for Westside Communities (App A Pump Report)**
3.1	Zone 1 Pump Station (PZ1)	1	LS	1,510,000	1,510,000	5,000 total flow (gpm) 239 ft (TDH)
3.2	Zone 1 Pump Station (PZ11)	1	LS	2,160,000	2,160,000	4,400 total flow (gpm) 417 ft (TDH)
3.3	Zone 2 Pump Station (PZ2)	1	LS	1,590,000	1,590,000	3,000 total flow (gpm) 424 ft (TDH)
3.4	Zone 2 Pump Station (PZ22)	1	LS	2,180,000	2,180,000	7,600 total flow (gpm) 244 ft (TDH)
3.5	Zone 3 Pump Station (PZ3)	1	LS	690,000	690,000	2,200 total flow (gpm) 199 ft (TDH)
3.6	Zone 4 Pump Station (PZ4)	1	LS	200,000	200,000	2,000 total flow (gpm) 18 ft (TDH)
3.7	Zone 5 Pump Station (PZ5)	1	LS	230,000	230,000	300 total flow (gpm) 184 ft (TDH)
4.0	**Storage Tank**					**Source: Nov 2015 RWMP Revision for Westside Communities (Table 4-1)**
4.1	Zone 1	0.3	MG	1,500,000	450,000	Unit cost based on recent project experience
	Zone 2	3.8	MG	1,000,000	3,800,000	
	Zone 3	2.5	MG	1,000,000	2,500,000	
	Zone 4	0.9	MG	1,250,000	1,062,500	
	Zone 5	0.8	MG	1,250,000	1,025,000	
5.0	**Site Retrofit Costs**					
	Based on number and size of sites	54	sites	27,000	1,458,000	Unit cost based on retrofit cost curve developed from VWC study by
						Dexter Williams. Number of Sites based on App A Demand Table IDs.
	Subtotal Facility Costs				**$66,181,048**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		869,875	% of Subtotal treatment, pump station, storage and discharge costs
						(Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		869,875	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and Remote (low-tech) Control	@	25%		4,349,375	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$6,089,125**	
					$72,270,173	
	Taxes	@	9%		2,382,518	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		3,613,509	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		10,840,526	% of Facility Direct Costs
	Estimate Contingency	@	30%		21,681,052	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$110,787,778**	
	Escalation to Midpoint of Construction	@	11%		12,186,656	assume 2% percent over 6
						contrustion start = 2019 end = 2024
	Project Capital Cost Total				**$122,974,434**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	Notes
Energy Costs					Pump Operation = 3286 hours operated per year
Energy: Zone 1 Pump Station (PZ1)	926,173	KWh	0.12	111,141	Pump Station Hp = 378 Total Motor HP Required
Energy: Zone 1 Pump Station (PZ11)	163,192	KWh	0.12	19,583	Pump Station Hp = 579 Total Motor HP Required
Energy: Zone 2 Pump Station (PZ2)	173,416	KWh	0.12	20,810	Pump Station Hp = 401 Total Motor HP Required
Energy: Zone 2 Pump Station (PZ22)	175,308	KWh	0.12	21,037	Pump Station Hp = 585 Total Motor HP Required
Energy: Zone 3 Pump Station (PZ3)	60,229	KWh	0.12	7,227	Pump Station Hp = 138 Total Motor HP Required
Energy: Zone 4 Pump Station (PZ4)	1,163	KWh	0.12	140	Pump Station Hp = 11 Total Motor HP Required
Energy: Zone 5 Pump Station (PZ5)	156	KWh	0.12	19	Pump Station Hp = 19 Total Motor HP Required
Energy (other)	75,000	KWh	0.12	9,000	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, customer service)	3.0	staff	100,000	300,000	full time staff at $100,000 salary per year
Maintenance: Other	7,184	AF	24	174,343	Based on historical costs for parts, materials, outside service/contracting and other needs
Contingency	@	10.0%		66,330	% of above O&M costs
Recycled Water Purchase (tertiary)	7,184	AF	200	1,436,800	Based on avereage LACSD RW purchase rate from 2013 to 2015
Annual O&M Costs ($/year)				**$2,166,429**	
Annual Unit O&M Costs ($/AFY)				**$302**	

Engineers Opinion of Probable Cost **KENNEDY/JENKS CONSULTANTS**

Alternative 3 - Groundwater Recharge (Surface Spreading)

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	3.3 mgd
Project:	Includes Phase 2A costs and maximizes deliveries to Off-Stream Spreading Site #1	**Date Prepared:**	Feb-2016	**Phase 2A RW Delivered:**	560 AFY (Irrigation)
RW Supply:	Served by Valencia WRP (Tertiary + Demineralized Blend)	**K/J Proj. No.**	1544241.00	**RW Recharged:**	3,700 AFY (Spreading)
Estimate:	Conceptual-Level	**ENR**	11,155	**Design Capacity:**	9.7 mgd

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	Phase 2A Pipelines to meet irrigation and IPR flows					
	8 inch-dia pipeline segments	7,400	LF	112	828,800	8 in-diameter $14 per inch-dia-lf
	12 inch-dia pipeline segments	800	LF	180	144,000	12 in-diameter $15 per inch-dia-lf
	24 inch-dia pipeline segments	30,200	LF	384	11,596,800	24 in-diameter $16 per inch-dia-lf
2.2	Phase 2A to Spreading Basin #1	17,900	LF	384	6,873,600	24 in-diameter $16 per inch-dia-lf
2.3	Special Crossings					
	Bore and Jack Pipe Laying	1,250	LF	3,960	4,950,000	24 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Construction	2	EA	35,000	70,000	based on jacking and receiving pit costs
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Central Park	1	LS	3,690,000	3,690,000	7,000 total flow (gpm) 490 ft (TDH)
						assume sufficient to meet Phase 2A irrigation Demands
3.2	Booster from Central Park to Basin #1	1	LS	420,000	420,000	7,000 total flow (gpm) 30 ft (TDH)
4.0	**Storage and Spreading Basin**					
4.1	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
	Spreading Basin #1					
4.2	Construct 20 acre basin	100	AF	30,000	3,000,000	Recent storage pond construction bid
4.3	Construct 1 acre settling basin	5	AF	60,000	300,000	
4.4	Diversion Structure	600	LF	6,000	3,600,000	Inflatable rubber dam for stormwater flow diversions, includes foundation
4.5	Hydraulic control structures	3	LS	50,000	150,000	** possibility to have LACSD pay for rubber dam*
4.6	Pipelines btw basins	1,000	LF	240	240,000	16 in-diameter $15 per inch-dia-lf
5.0	**Monitoring Wells**					
5.1	Monitroing Wells	3	LS	160,000	480,000	
	Extraction Wells					Assume use of existing wells
6.0	**Site Retrofit Costs (Phase 2A)**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$38,394,700**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		630,275	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		630,275	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and SCADA Control	@	25%		2,085,000	% of Subtotal treatment, pump station, storage and discharge costs (not icnluding spreading basin or pipelines)
	Subtotal Additional Facility Costs				**$3,345,550**	
					$41,740,250	
	Taxes	@	9%		1,382,209	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		2,087,013	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		6,261,038	% of Facility Direct Costs
	Estimate Contingency	@	30%		12,522,075	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$63,992,584**	
	Escalation to Midpoint of Construction	@	18%		11,518,665	assume 2% percent over 9
						contrustion start = 2024 end = 2026
	Project Capital Cost Total				**$75,511,249**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3861 hours operated per year
Energy: PS from Valencia WRP to Central Park	3,118,513	KWh	0.12	374,222	Pump Station Hp = 1083 Total Motor HP Required
Energy (other)	156,000	KWh	0.12	18,720	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, monitoring)	2.0	staff	100,000	200,000	full time staff at $100,000 salary per year
					** may require additional LACFCD staff time to operate diversion/ponds*
Maintenance: Recharge Ponds	@	0.5%		40,078	Includes seasonal weed and erosion control, cleaning hydraulic structures, sediment removal, etc (% of direct facility costs)
Maintenance: Pump Station, Monitoring Wells, Diversion	@	1.0%		41,700	% of above direct facility costs for these components
Contingency	@	10%		67,472	% of above O&M costs
Recycled Water Purchases					
Tertiary for irrigation in summer	237	AF	200	47,400	Unit Cost based on avereage LACSD RW purchase rate from 2013 to 2015
					Based on summer months when no excess RW is available for spreading
Tertiary-Valencia Blend non-summer irrigation	323	AF	385	124,267	Based on 50:50 mix of tertiary:Blend at costs below
Tertiary for spreading (50% of source water)	1,850	AF	200	370,000	Based on avereage LACSD RW purchase rate from 2013 to 2015
Valencia Blend for spreading (50% of source water))	1,850	AF	569	1,052,650	Based on preliminary estimate from LACSD at 70:30 belnd of tertiary:RO
Annual O&M Costs ($/year)				**$2,336,508**	
Annual Unit O&M Costs ($/AFY)				**$548**	

Engineers Opinion of Probable Cost
Alternative 3 - Groundwater Recharge (Surface Spreading)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	3.3 mgd
Project:	Includes Phase 2A costs and maximizes deliveries to In-Stream Spreading Site #3a	**Date Prepared:**	Feb-2016	**Phase 2A RW Delivered:**	560 AFY (Irrigation)
RW Supply:	Served by Valencia WRP (Demineralized Blend)	**K/J Proj. No.**	1544241.00	**RW Recharged:**	3,700 AFY (Spreading)
Estimate:	Conceptual-Level	**ENR**	11,155	**Design Capacity:**	9.7 mgd

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	Phase 2A Pipelines to meet irrigation and IPR flows					
	8 inch-dia pipeline segments	7,400	LF	112	828,800	8 in-diameter $14 per inch-dia-lf
	12 inch-dia pipeline segments	800	LF	180	144,000	12 in-diameter $15 per inch-dia-lf
	24 inch-dia pipeline segments	30,200	LF	384	11,596,800	24 in-diameter $16 per inch-dia-lf
2.2	Phase 2A to Spreading Basin #3a	49,500	LF	384	19,008,000	24 in-diameter $16 per inch-dia-lf
2.3	Special Crossings					
	Bore and Jack Pipe Laying	1,250	LF	3,960	4,950,000	24 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Construction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major Intersections	500	LF	960	480,000	24 in-diameter $40 per inch-dia-lf
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Central Park	1	LS	3,690,000	3,690,000	7,000 total flow (gpm) 490 ft (TDH)
						**assume sufficient to meet Phase 2A irrigation Demands*
3.2	Booster PS from Central Park to Spreading Basin #3	1	LS	3,100,000	3,100,000	7,000 total flow (gpm) 400 ft (TDH)
4.0	**Storage and Spreading Basin**					
4.1	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
	Spreading Basin #3a					
4.2	Construct Levees for In-River Basin	2,000	LF	0	0	assume levee along south side of SCR
4.3	Diversion Structure	400	LF	6,000	2,400,000	Inflatable rubber dam for recharge basin creation
4.4	Hydraulic control structure	1	LS	50,000	50,000	
5.0	**Monitoring Wells**					
5.1	Monitroing Wells	3	LS	160,000	480,000	
	Extraction Wells					Assume use of existing wells
6.0	**Site Retrofit Costs (Phase 2A)**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$48,849,100**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		522,275	% of Subtotal treatment, pump station, storage and monitoring wells
						(Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		522,275	% of Subtotal treatment, pump station, storage and monitoring wells
8.0	Electrical, I&C, and SCADA Control	@	25%		2,611,375	% of Subtotal treatment, pump station, storage and discharge costs
						(not icnluding levee or pipelines)
	Subtotal Additional Facility Costs				**$3,655,925**	
					$52,505,025	
	Taxes	@	9%		1,758,568	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		2,625,251	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		7,875,754	% of Facility Direct Costs
	Estimate Contingency	@	30%		15,751,508	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$80,516,105**	
	Escalation to Midpoint of Construction	@	18%		14,492,899	assume 2% percent over 9
						contrustion start = 2024 end = 2026
	Project Capital Cost Total				**$95,009,004**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3861 hours operated per year
Energy: PS from Valencia WRP to Central Park	3,118,513	KWh	0.12	374,222	Pump Station Hp = 1083 Total Motor HP Required
Energy: Booster PS from Central Park to Spreading Basin #3	713,872	KWh	0.12	85,665	Pump Station Hp = 884 Total Motor HP Required
Energy (other)	192,000	KWh	0.12	23,040	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, monitoring)	2.0	staff	100,000	200,000	full time staff at $100,000 salary per year
					** may require additional LACFCD staff time to operate diversion/ponds*
Maintenance: Recharge Ponds	@	0.5%		15,878	Includes seasonal weed and erosion control, cleaning hydraulic structures, sediment removal, etc (% of direct facility costs)
Maintenance: Pump Station, Monitoring Wells, Diversion	@	1.0%		4,800	% of above direct facility costs for these components
Contingency	@	10%		70,360	% of above O&M costs
Recycled Water Purchases					
Tertiary for irrigation in summer	237	AF	200	47,400	Unit Cost based on avereage LACSD RW purchase rate from 2013 to 2015
					Based on summer months when no excess RW is available for spreading
Valencia Blend for non-summer irrigation	323	AF	569	183,895	Shift to conveying Valencia Blend for spreading
Valencia Blend for spreading (100% of source water)	3,700	AF	569	2,105,300	Based on preliminary estimate from LACSD at 70:30 belnd of tertiary:RO
Annual O&M Costs ($/year)				**$3,110,560**	
Annual Unit O&M Costs ($/AFY)				**$730.15**	

Engineers Opinion of Probable Cost
Alternative 3 - Groundwater Recharge (Surface Spreading)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	3.3	mgd
Project:	Includes Phase 2A costs and maximizes deliveries to Off-Stream Spreading Site #3b	**Date Prepared:**	Feb-2016	**Phase 2A RW Delivered:**	560	AFY (Irrigation)
RW Supply:	Served by Valencia WRP (Demineralized Blend)	**K/J Proj. No.**	1544241.00	**RW Recharged:**	3,700	AFY (Spreading)
Estimate:	Conceptual-Level	**ENR**	11,155	**Design Capacity:**	9.7	mgd

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**Pipelines**					
2.1	Phase 2A Pipelines to meet irrigation and IPR flows					
	8 inch-dia pipeline segments	7,400	LF	112	828,800	8 in-diameter $14 per inch-dia-lf
	12 inch-dia pipeline segments	800	LF	180	144,000	12 in-diameter $15 per inch-dia-lf
	24 inch-dia pipeline segments	30,200	LF	384	11,596,800	24 in-diameter $16 per inch-dia-lf
2.2	Phase 2A to Spreading Basin #3a	49,500	LF	384	19,008,000	24 in-diameter $16 per inch-dia-lf
2.3	Extension from #3a to #3b	4,400	LF	384	1,689,600	24 in-diameter $16 per inch-dia-lf
2.4	Pipeline from SCR diversion to Basin (for stormwater)	1,200	LF	240	288,000	16 in-diameter $15 per inch-dia-lf
2.4	Special Crossings					
	Bore and Jack Pipe Laying	1,250	LF	3,960	4,950,000	24 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Construction	2	EA	35,000	70,000	based on jacking and receiving pit costs
	Major Intersections	500	LF	960	480,000	24 in-diameter $40 per inch-dia-lf
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Central Park	1	LS	3,690,000	3,690,000	7,000 total flow (gpm) 490 ft (TDH)
						**assume sufficient to meet Phase 2A irrigation Demands*
3.2	Booster PS from Central Park to Spreading Basin #3	1	LS	3,100,000	3,100,000	7,000 total flow (gpm) 400 ft (TDH)
3.3	Stormwater pump station to Spreading Basin	1	LS	560,000	560,000	6,800 total flow (gpm) 48 ft (TDH)
4.0	**Storage and Spreading Basin**					
4.1	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
	Spreading Basin #3b					
4.2	Construct 28 acre basin	140	AF	30,000	4,200,000	Recent storage pond construction bid
4.3	Diversion Structure	200	LF	6,000	1,200,000	Inflatable rubber dam for stormwater diversion
4.4	Hydraulic control structure	2	LS	50,000	100,000	One at RW inlet and one at stormwater inlet
5.0	**Monitoring Wells**					
5.1	Monitroing Wells	3	LS	160,000	480,000	
	Extraction Wells					Assume use of existing wells
6.0	**Site Retrofit Costs (Phase 2A)**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$54,436,700**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		702,775	% of Subtotal treatment, pump station, storage and monitoring wells (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		702,775	% of Subtotal treatment, pump station, storage and monitoring wells
8.0	Electrical, I&C, and SCADA Control	@	25%		3,513,875	% of Subtotal treatment, pump station, storage and discharge costs (not icnluding levee or pipelines)
	Subtotal Additional Facility Costs				**$4,919,425**	
					$59,356,125	
	Land Purchase	40	acres	7,500	300,000	Est cost to purchase privately owned piece of land
	Taxes	@	9%		1,959,721	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		2,967,806	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		8,903,419	% of Facility Direct Costs
	Estimate Contingency	@	30%		17,806,838	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$91,293,909**	
	Escalation to Midpoint of Construction	@	18%		16,432,904	assume 2% percent over 9
						contrustion start = 2024 end = 2026
				Project Capital Cost Total	**$107,726,812**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	Notes
Energy Costs					Pump Operation = 3861 hours operated per year
Energy: PS from Valencia WRP to Central Park	3,118,513	KWh	0.12	374,222	Pump Station Hp = 1083 Total Motor HP Required
Energy: Booster PS from Central Park to Spreading Basin #3	713,872	KWh	0.12	85,665	Pump Station Hp = 884 Total Motor HP Required
Energy: Stormwater pump station to Spreading Basin	67,932	KWh	0.12	8,152	Pump Station Hp = 103 Total Motor HP Required
Energy (other)	195,000	KWh	0.12	23,400	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, monitoring)	2.0	staff	100,000	200,000	full time staff at $100,000 salary per year
					** may require additional LACFCD staff time to operate diversion/ponds*
Maintenance: Recharge Ponds	@	0.5%		31,128	Includes seasonal weed and erosion control, cleaning hydraulic structures, sediment removal, etc (% of direct facility costs)
Maintenance: Pump Station, Monitoring Wells, Diversion	@	1.0%		4,800	% of above direct facility costs for these components
Contingency	@	10%		72,737	% of above O&M costs
Recycled Water Purchases					
Tertiary for irrigation in summer	237	AF	200	47,400	Unit Cost based on avereage LACSD RW purchase rate from 2013 to 2015
					Based on summer months when no excess RW is available for spreading
Valencia Blend for non-summer irrigation	323	AF	569	183,895	Shift to conveying Valencia Blend for spreading
Valencia Blend for spreading (100% of source water)	3,700	AF	569	2,105,300	Based on preliminary estimate from LACSD at 70:30 belnd of tertiary:RO
			Annual O&M Costs ($/year)	**$3,136,698**	
			Annual Unit O&M Costs ($/AFY)	**$736.28**	

Engineers Opinion of Probable Cost — **KENNEDY/JENKS CONSULTANTS**

Alternative 3 - Groundwater Recharge (Surface Spreading)

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT
Project:	Includes Phase 2A costs and reuses Honby lateral and Honby pipeline to deliver to In-Stream Spreading Site #3b	**Date Prepared:**	Feb-2016
RW Supply:	Served by Valencia WRP (Demineralized Blend)	**K/J Proj. No.**	1544241.00
Estimate:	Conceptual-Level	**ENR**	11,155

Average Annual Product Flow:	1.0	mgd
Phase 2A RW Delivered:	560	AFY (Irrigation)
RW Recharged:	1,100	AFY (Spreading)
Design Capacity:	3.0	mgd

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**New Pipelines (west of Honby)**					
2.1	Phase 2A Pipelines to meet irrigation and IPR flows (limited by Honby Capacity)					
	8 inch-dia pipeline segments	8,900	LF	112	996,800	8 in-diameter; $14 per inch-dia-lf
	14 inch-dia pipeline segments	14,900	LF	210	3,129,000	14 in-diameter; $15 per inch-dia-lf
	16 inch-dia pipeline segments	14,600	LF	240	3,504,000	16 in-diameter; $15 per inch-dia-lf
2.2	Phase 2A to old Honby Lateral	2,000	LF	210	420,000	14 in-diameter; $15 per inch-dia-lf
	Repurpose Existing Pipelines					**assumes cost to rehab pipeline for reuse*
2.3	Repurpose Honby Lateral to get to Honby PS Pad	1	LS	200,000	200,000	Reuse ~ 6,000 LF of Honby Lateral, assume 600 LF to connect from east side to site of Honby PS
2.4	Rehab Honby Pipeline from Honby PS Pad to near Sand Canyon Rd	25,000	LF	140	3,500,000	12 in-diameter; $12 per inch-dia-lf
2.5	Jacking/Receiving Pits for sliplining	25	LS	18,000	450,000	assume a jacking and receiving pit every 1,000 LF for sliplilning
	New Pipelines (east of Honby)					
2.6	Extension from Honby Pipeline near Sand Canyon to Site #3b	20,000	LF	210	4,200,000	14 in-diameter; $15 per inch-dia-lf
2.7	Pipeline from SCR diversion to Basin (for stormwater)	1,200	LF	240	288,000	16 in-diameter; $15 per inch-dia-lf
2.8	Special Crossings					
	Bore and Jack Pipe Laying	0	LF	2,310	0	14 in-diameter; $165 per inch-dia-lf
	Bore and Jack Pit Construction	0	EA	35,000	0	based on jacking and receiving pit costs
	Major Intersections	150	LF	560	84,000	14 in-diameter; $40 per inch-dia-lf
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Central Park	1	LS	2,380,000	2,380,000	4,200 total flow (gpm); 490 ft (TDH)
						**assume sufficient to meet Phase 2A irrigation Demands*
3.2	Booster PS from Old Honby PS Pad to Spreading Basin #3	1	LS	2,010,000	2,010,000	4,200 total flow (gpm); 400 ft (TDH)
3.3	Stormwater pump station to Spreading Basin	1	LS	560,000	560,000	6,800 total flow (gpm); 48 ft (TDH)
4.0	**Storage and Spreading Basin**					
4.1	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
	Spreading Basin #3b					
4.2	Construct 28 acre basin	140	AF	30,000	4,200,000	Recent storage pond construction bid
4.3	Diversion Structure	200	LF	6,000	1,200,000	Inflatable rubber dam for stormwater diversion
4.4	Hydraulic control structure	2	LS	50,000	100,000	One at RW inlet and one at stormwater inlet
5.0	**Monitoring Wells**					
5.1	Monitroing Wells	3	LS	160,000	480,000	
	Extraction Wells					Assume use of existing wells
6.0	**Site Retrofit Costs (Phase 2A)**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$29,753,300**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		582,775	% of Subtotal treatment, pump station, storage and monitoring wells
						(Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		582,775	% of Subtotal treatment, pump station, storage and monitoring wells
8.0	Electrical, I&C, and SCADA Control	@	25%		2,913,875	% of Subtotal treatment, pump station, storage and discharge costs
						(not icnluding levee or pipelines)
	Subtotal Additional Facility Costs				**$4,079,425**	
					$33,832,725	
	Land Purchase	40	acres	7,500	300,000	Est cost to purchase privately owned piece of land
	Taxes	@	9%		1,071,119	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		1,691,636	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		5,074,909	% of Facility Direct Costs
	Estimate Contingency	@	30%		10,149,818	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$52,120,206**	
	Escalation to Midpoint of Construction	@	18%		9,381,637	assume 2% percent over 9
						contrustion start = 2024; end = 2026
	Project Capital Cost Total				**$61,501,843**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3861 hours operated per year
Energy: PS from Valencia WRP to Central Park	1,871,108	KWh	0.12	224,533	Pump Station Hp = 650 Total Motor HP Required
Energy: Booster PS from Old Honby PS Pad to Spreading Basin #3	256,994	KWh	0.12	30,839	Pump Station Hp = 530 Total Motor HP Required
Energy: Stormwater pump station to Spreading Basin	49,930	KWh	0.12	5,992	Pump Station Hp = 103 Total Motor HP Required
Energy (other)	109,000	KWh	0.12	13,080	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, monitoring)	2.0	staff	100,000	200,000	full time staff at $100,000 salary per year
					** may require additional LACFCD staff time to operate diversion/ponds*
Maintenance: Recharge Ponds	@	0.5%		31,128	Includes seasonal weed and erosion control, cleaning hydraulic structures, sediment removal, etc (% of direct facility costs)
Maintenance: Pump Station, Monitoring Wells, Diversion	@	1.0%		4,800	% of above direct facility costs for these components
Contingency	@	10%		51,037	% of above O&M costs
Recycled Water Purchases					
Tertiary for irrigation in summer	237	AF	200	47,400	Unit Cost based on avereage LACSD RW purchase rate from 2013 to 2015
					Based on summer months when no excess RW is available for spreading
Valencia Blend for non-summer irrigation	323	AF	569	183,895	Shift to conveying Valencia Blend for spreading
Valencia Blend for spreading (100% of source water)	1,100	AF	569	625,900	Based on preliminary estimate from LACSD at 70:30 belnd of tertiary:RO
Annual O&M Costs ($/year)				**$1,418,604**	
Annual Unit O&M Costs ($/AFY)				**$854.48**	

Engineers Opinion of Probable Cost
Alternative 3 - Groundwater Recharge (Surface Spreading)

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	3.3 mgd
Project:	Includes Phase 2A costs, splits deliveries between Spreading Sites #1 & #3b, and reuses Honby lateral and Honby pipeline	**Date Prepared:**	Feb-2016	**Phase 2A RW Delivered:**	560 AFY (Irrigation)
RW Supply:	Served by Valencia WRP (Demineralized Blend)	**K/J Proj. No.**	1544241.00	**RW Recharged:**	3,700 AFY (Spreading)
Estimate:	Conceptual-Level	**ENR**	11,155	**Design Capacity:**	3.0 mgd

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (no additional facilities)**					
2.0	**New Pipelines (west of Site #1)**					
2.1	Phase 2A Pipelines to meet irrigation and IPR flows (limited by Honby Capacity)					
	8 inch-dia pipeline segments	8,900	LF	112	996,800	8 in-diameter; $14 per inch-dia-lf
	24 inch-dia pipeline segments	14,900	LF	384	5,721,600	24 in-diameter; $16 per inch-dia-lf
	24 inch-dia pipeline segments	14,600	LF	384	5,606,400	24 in-diameter; $16 per inch-dia-lf
2.2	Phase 2A to old Honby Lateral	2,000	LF	384	768,000	24 in-diameter; $16 per inch-dia-lf
2.3	Honby PS to Spreadng Site #1	10,000	LF	384	3,840,000	24 in-diameter; $16 per inch-dia-lf
	Repurpose Existing Pipelines					*assumes cost to rehab pipeline for reuse*
2.4	Repurpose Honby Lateral to get to Honby PS Pad	1	LS	200,000	200,000	Reuse ~ 6,000 LF of Honby Lateral, assume 600 LF to connect from east side to site of Honby PS
2.5	Rehab Honby Pipeline from Honby PS Pad to near Sand Canyon Rd	25,000	LF	140	3,500,000	12 in-diameter; $12 per inch-dia-lf
2.6	Jacking/Receiving Pits for sliplining	25	LS	10,000	250,000	assume a jacking and receiving pit every 1,000 LF for sliplilning
	New Pipelines (east of Honby)					
2.7	Extension from Honby Pipeline near Sand Canyon to Site #3b	20,000	LF	210	4,200,000	14 in-diameter; $15 per inch-dia-lf
2.8	Pipeline from SCR diversion to Basin (for stormwater)	1,200	LF	240	288,000	16 in-diameter; $15 per inch-dia-lf
2.9	Special Crossings					
	Bore and Jack Pipe Laying	0	LF	2,310	0	14 in-diameter; $165 per inch-dia-lf
	Bore and Jack Pit Construction	0	EA	35,000	0	based on jacking and receiving pit costs
	Major Intersections	150	LF	560	84,000	14 in-diameter; $40 per inch-dia-lf
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Central Park	1	LS	3,690,000	3,690,000	7,000 total flow (gpm); 490 ft (TDH)
						assume sufficient to meet Phase 2A irrigation Demands
3.2	Booster PS from Old Honby PS Pad to Spreading Basin #3	1	LS	2,010,000	2,010,000	4,200 total flow (gpm); 400 ft (TDH)
3.3	Stormwater pump station to Spreading Basin	1	LS	560,000	560,000	6,800 total flow (gpm); 48 ft (TDH)
4.0	**Storage and Spreading Basin**					
	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
4.1	**Spreading Basin #3b**					
4.2	Construct 20 acre basin	100	AF	30,000	3,000,000	Recent storage pond construction bid
4.3	Construct 1 acre settling basin	5	AF	60,000	300,000	
4.4	Diversion Structure	600	LF	6,000	3,600,000	Inflatable rubber dam for stormwater flow diversions, includes foundation
4.5	Hydraulic control structures	3	LS	50,000	150,000	** possibility to have LACSD pay for rubber dam*
4.6	Pipelines btw basins	1,000	LF	240	240,000	16 in-diameter; $15 per inch-dia-lf
	Spreading Basin #3a					
4.7	Construct 28 acre basin	140	AF	30,000	4,200,000	Recent storage pond construction bid
4.8	Diversion Structure	200	LF	6,000	1,200,000	Inflatable rubber dam for stormwater diversion
4.9	Hydraulic control structure	2	LS	50,000	100,000	One at RW inlet and one at stormwater inlet
5.0	**Monitoring Wells**					
5.1	Monitroing Wells	3	LS	160,000	480,000	
	Extraction Wells					Assume use of existing wells
6.0	**Site Retrofit Costs (Phase 2A)**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$47,036,300**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		1,012,775	% of Subtotal treatment, pump station, storage and monitoring wells (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		1,012,775	% of Subtotal treatment, pump station, storage and monitoring wells
8.0	Electrical, I&C, and SCADA Control	@	25%		5,063,875	% of Subtotal treatment, pump station, storage and discharge costs (not icnluding levee or pipelines)
	Subtotal Additional Facility Costs				**$7,089,425**	
					$54,125,725	
	Land Purchase	40	acres	7,500	300,000	Est cost to purchase privately owned piece of land
	Taxes	@	9%		1,693,307	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		2,706,286	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		8,118,859	% of Facility Direct Costs
	Estimate Contingency	@	30%		16,237,718	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$83,181,894**	
	Escalation to Midpoint of Construction	@	18%		14,972,741	assume 2% percent over 9; contrustion start = 2024; end = 2026
				Project Capital Cost Total	**$98,154,635**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs					Pump Operation = 3861 hours operated per year
Energy: PS from Valencia WRP to Central Park	3,118,513	KWh	0.12	374,222	Pump Station Hp = 1083 Total Motor HP Required
Energy: Booster PS from Old Honby PS Pad to Spreading Basin #3	428,323	KWh	0.12	51,399	Pump Station Hp = 530 Total Motor HP Required
Energy: Stormwater pump station to Spreading Basin	83,217	KWh	0.12	9,986	Pump Station Hp = 103 Total Motor HP Required
Energy (other)	182,000	KWh	0.12	21,840	5% of sum of pumping energy requirements
Labor Costs					
Other Labor (pipeline, PS, monitoring)	2.0	staff	100,000	200,000	full time staff at $100,000 salary per year
					** may require additional LACFCD staff time to operate diversion/ponds*
Maintenance: Recharge Ponds	@	0.5%		67,578	Includes seasonal weed and erosion control, cleaning hydraulic structures, sediment removal, etc (% of direct facility costs)
Maintenance: Pump Station, Monitoring Wells, Diversion	@	1.0%		4,800	% of above direct facility costs for these components
Contingency	@	10%		72,982	% of above O&M costs
Recycled Water Purchases					
Tertiary for irrigation in summer	237	AF	200	47,400	Unit Cost based on avereage LACSD RW purchase rate from 2013 to 2015
					Based on summer months when no excess RW is available for spreading
Valencia Blend for non-summer irrigation	323	AF	569	183,895	Shift to conveying Valencia Blend for spreading
Valencia Blend for spreading (100% of source water)	3,700	AF	569	2,105,300	Based on preliminary estimate from LACSD at 70:30 belnd of tertiary:RO
			Annual O&M Costs ($/year)	**$3,139,402**	
			Annual Unit O&M Costs ($/AFY)	**$736.92**	

Engineers Opinion of Probable Cost
Alternative 4 - Advanced Treatment for Potable Reuse

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	4.9 mgd
Project:	Direct Injection Location #1 (Near Valencia WRP)	**Date Prepared:**	Feb-2016	**RW Recharged:**	5,500 AFY
RW Supply:	Valencia WRP with Advanced Treatment	**K/J Proj. No.**	1544241.00	**Design Capacity:**	9.7 mgd
Estimate:	Conceptual-Level	**ENR**	11,155		

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility (AWTF for Peak Flow)**					Source: Trussell TM based on costs from LACSD Chrloride EIR
1.1	Microfiltration	1	LS	18,600,000	18,600,000	
1.2	Enhanced Brine Concentration (NF + IX)	1	LS	36,200,000	36,200,000	
1.3	Reverse Osmosis	1	LS	29,950,000	29,950,000	
1.4	UV AOP	1	LS	4,700,000	4,700,000	
1.5	Other Appurtenances	1	LS	17,550,000	17,550,000	
2.0	**Pipelines**					
2.1	Valencia to Direct Injection Location #1	6,000	lf	384	2,304,000	24 in-diameter; $16 per inch-dia-lf
2.2	Special Crossings (estimate)	100	lf	960	96,000	24 in-diameter; $40 per inch-dia-lf
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Direct Injection Site	1	LS	990,000	990,000	7,000 total flow (gpm); 100 ft (TDH)
4.0	**Storage**					
	None					
5.0	**Groundwater Wells**					
5.1	Injection wells	4	LS	1,070,000	4,280,000	1,000 gpm per well
5.2	Monitroing Wells	3	LS	160,000	480,000	
5.3	Extraction Wells					Assume use of existing wells
	Subtotal Facility Costs				**$115,150,000**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		5,637,500	% of Subtotal treatment, pump station, storage and discharge costs
						(Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		5,637,500	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and SCADA Control	@	25%		28,187,500	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$39,462,500**	
					$154,612,500	
	Taxes	@	9%		4,145,400	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		7,730,625	% of Facility Direct Costs
	Contractor Overhead & Profit	@	#####		23,191,875	% of Facility Direct Costs
	Estimate Contingency	@	#####		46,383,750	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$236,064,150**	
	Escalation to Midpoint of Construction	@	#####		42,491,547	assume 2% percent over 9
						contrustion start = 2024; end = 2026
	Project Capital Cost Total				**$278,555,697**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	Notes
Energy Costs (non-treatment)					Pump Operation = 5150 hours operated per year
Energy (injection wells)	667,980	KWh	0.12	80,158	Pump Station Hp = 174 Total Motor HP Required
Energy (other)	33,000	KWh	0.12	3,960	5% of sum of pumping energy requirements
Labor Costs (non-treatment)					
Other Labor (pipeline, injection wells, monitoring)	1.0	staff	100,000	100,000	full time staff at $100,000 salary per year
Maintenance: Pipeline, Injection and Monitoring Wells	@	1.0%		81,500	% of above direct facility costs for pipelines, injection and monitoring wells
Contingency	@	10%		26,562	% of above O&M costs
Recycled Water Purchase (tertiary)	5,500	AF	200	1,100,000	Based on avereage LACSD RW purchase rate from 2013 to 2015
Advanced Treatment Costs					Source: Trussell Technologies, including energy, labor, chemicals, materials and replacement costs by process type
Microfiltration	4,900,000	gal	0.22	1,097,600	
Enhanced Brine Concentration (NF + IX)	4,900,000	gal	0.45	2,195,200	unit cost based on average operating flow over the year
Reverse Osmosis	4,900,000	gal	0.43	2,095,418	
UV AOP	4,900,000	gal	0.04	199,564	
Other Appurtenances	4,900,000	gal	0.12	598,691	
Annual O&M Costs ($/year)				**$7,578,652**	
Annual Unit O&M Costs ($/AFY)				**$1,378**	

Engineers Opinion of Probable Cost
Alternative 4 - Advanced Treatment for Potable Reuse

KENNEDY/JENKS CONSULTANTS

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	4.9 mgd
Project:	SW Augmentation at Castaic Lake	**Date Prepared:**	Feb-2016	**RW Augmented to Castaic Lake:**	5,500 AFY
RW Supply:	Valencia WRP with Advanced Treatment	**K/J Proj. No.**	1544241.00	**Design Capacity:**	9.7 mgd
Estimate:	Conceptual-Level	**ENR**	11,155		

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility**					Source: Trussell TM based on costs from LACSD Chrloride EIR
1.1	Microfiltration	1	LS	18,600,000	18,600,000	
1.2	Enhanced Brine Concentration (NF + IX)	1	LS	36,200,000	36,200,000	
1.3	Reverse Osmosis	1	LS	29,950,000	29,950,000	
1.4	UV AOP	1	LS	4,700,000	4,700,000	
1.5	Other Appurtenances	1	LS	17,550,000	17,550,000	
2.0	**Pipelines**					
2.1	Valencia WRP to ARWT at Earl Schmidt	36,000	LF	384	13,824,000	24 in-diameter; $16 per inch-dia-lf
2.2	ARWT to Castaic Lake (Boat Ramp Location)	9,000	LF	384	3,456,000	24 in-diameter; $16 per inch-dia-lf
2.3	Special Crossings					
	Major Intersections	400	LF	960	384,000	24 in-diameter; $40 per inch-dia-lf
3.0	**Pump Stations**					
3.1	Valencia WRP to ARWT at Earl Schmidt	1	LS	3,940,000	3,940,000	7,000 total flow (gpm); 530 ft (TDH)
3.2	ARWT to Castaic Lake	1	LS	1,310,000	1,310,000	7,000 total flow (gpm); 140 ft (TDH)
4.0	**Storage**					
	None					
5.0	**Discharge Facility**	4.9	mgd	350,000	1,710,000	Standard bank outfall with erosion protection and energy dissipation.
	Subtotal Facility Costs				**$131,624,000**	
Additional Facility Capital Costs						
6.0	Site Development Costs	@	5%		5,698,000	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
7.0	Yard Piping	@	5%		5,698,000	% of Subtotal treatment, pump station, storage and discharge costs
8.0	Electrical, I&C, and SCADA Control	@	25%		28,490,000	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$39,886,000**	
					$171,510,000	
	Taxes	@	9%		4,738,464	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		8,575,500	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		25,726,500	% of Facility Direct Costs
	Estimate Contingency	@	30%		51,453,000	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$262,003,464**	
	Escalation to Midpoint of Construction	@	28%		73,360,970	assume 2% percent over 14; contrustion start = 2028 end = 2032
	Project Capital Cost Total				**$335,364,434**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	Notes
Energy Costs (non-treatment)					Pump Operation = 5150 hours operated per year
Energy - Valencia WRP to ARWT at Earl Schmidt	4,491,571	KWh	0.12	538,989	Pump Station Hp = 1169 Total Motor HP Required
Energy - ARWT to Castaic Lake	1,214,190	KWh	0.12	145,703	Pump Station Hp = 316 Total Motor HP Required
Energy (other)	285,000	KWh	0.12	34,200	5% of sum of pumping energy requirements
Labor Costs (non-treatment)					
Other Labor (pipeline, pump stations, discharge, monitoring)	1.0	staff	100,000	100,000	full time staff at $100,000 salary per year
Maintenance: Pipeline, Pump Station, discharge	@	1.0%		246,240	% of above direct facility costs for pipelines, injection and monitoring wells
Contingency	@	10%		106,513	% of above O&M costs
Recycled Water Purchase (tertiary)	5,500	AF	200	1,100,000	Based on avereage LACSD RW purchase rate from 2013 to 2015
Advanced Treatment Costs					Source: Trussell Technologies, including energy, labor, chemicals, materials and replacement costs by process type
Microfiltration	4,900,000	gal	0.22	1,097,600	
Enhanced Brine Concentration (NF + IX)	4,900,000	gal	0.45	2,195,200	unit cost based on average operating flow over the year
Reverse Osmosis	4,900,000	gal	0.43	2,095,418	
UV AOP	4,900,000	gal	0.04	199,564	
Other Appurtenances	4,900,000	gal	0.12	598,691	
Annual O&M Costs ($/year)				**$8,458,117**	
Annual Unit O&M Costs ($/AFY)				**$1,538**	

Engineers Opinion of Probable Cost KENNEDY/JENKS CONSULTANTS

Alternative 4 - Advanced Treatment for Potable Reuse

Study:	CLWA Recycled Water Master Plan	**Prepared By:**	DTT	**Average Annual Product Flow:**	4.9 mgd
Project:	Direct Potable Reuse + Phase 2A	**Date Prepared:**	Feb-2016	**Phase 2A RW Delivered:**	560 AFY (Irrigation)
RW Supply:	Valencia WRP with Advanced Treatment	**K/J Proj. No.**	1544241.00	**RW delivered to Rio Vista:**	5,500 AFY (DPR)
Estimate:	Conceptual-Level	**ENR**	11,155	**Design Capacity:**	9.7 mgd

Item No.	Description	Qty	Units	Total Costs $/Unit	Total Capital Cost	Notes/Source
Facility Capital Costs						
1.0	**Treatment Facility**					Source: Trussell TM based on costs from LACSD Chrloride EIR
1.1	Ozone System	1	LS	3,150,000	3,150,000	
1.2	Biologically Active Carbon Filter	1	LS	7,900,000	7,900,000	
1.3	Microfiltration	1	LS	18,600,000	18,600,000	
1.4	Enhanced Brine Concentration (NF + IX)	1	LS	36,200,000	36,200,000	
1.5	Reverse Osmosis	1	LS	29,950,000	29,950,000	
1.6	UV AOP	1	LS	4,700,000	4,700,000	
1.7	Other Appurtenances	1	LS	17,550,000	17,550,000	
2.0	**Pipelines**					
2.1	Phase 2A Pipelines to meet irrigation and IPR flows					
	8 inch-dia pipeline segments	7,400	LF	272	2,012,800	8 in-diameter $14 per inch-dia-lf
	24 inch-dia pipeline segments	14,900	LF	384	5,721,600	24 in-diameter $16 per inch-dia-lf
	24 inch-dia pipeline segments	14,600	LF	384	5,606,400	24 in-diameter $16 per inch-dia-lf
2.2	Phase 2A to Rio Vista	1,000	LF	384	384,000	24 in-diameter $16 per inch-dia-lf
2.3	Special Crossings					
	Major Intersections	950	LF	960	912,000	24 in-diameter $40 per inch-dia-lf
	Bore and Jack Pipe Laying	700	LF	3,960	2,772,000	24 in-diameter $165 per inch-dia-lf
	Bore and Jack Pit Constuction	2	EA	35,000	70,000	based on jacking and receiving pit costs
3.0	**Pump Stations**					
3.1	PS from Valencia WRP to Central Park	1	LS	3,690,000	3,690,000	7,000 total flow (gpm) 490 ft (TDH)
3.1	Suction PS at Rio Vista	1	LS	290,000	290,000	6,000 total flow (gpm) 20 ft (TDH)
4.0	**Storage Tank**					
4.1	Storage Tank at Central Park	1	MG	725,500	725,500	RS Means 2015 Water Storage Tank Construction Cost
	Eng Buffer Storage (at ARWT or Rio Vista)					
4.2	Steel Ground Tank	5	mil gal	350,000	1,750,000	RS Means 2015 Water Storage Tank Construction Cost
6.0	**Site Retrofit Costs (Phase 2A)**					
	Based on number and size of sites	51	sites	26,000	1,326,000	Unit cost based on retrofit cost curve developed from VWC study by Dexter Willliams
	Subtotal Facility Costs				**$143,310,300**	
Additional Facility Capital Costs						
5.0	Site Development Costs	@	5%		6,225,275	% of Subtotal treatment, pump station, storage and discharge costs (Includes grading, erosion control, cut/fill, etc.)
6.0	Yard Piping	@	5%		6,225,275	% of Subtotal treatment, pump station, storage and discharge costs
7.0	Electrical, I&C, and SCADA Control	@	25%		31,126,375	% of Subtotal treatment, pump station, storage and discharge costs
	Subtotal Additional Facility Costs				**$43,576,925**	
					$186,887,225	
	Taxes	@	9%		5,159,171	apply taxes to 40% of the Capital Costs for facilities
	Mobilization/Bonds/Permits	@	5%		9,344,361	% of Facility Direct Costs
	Contractor Overhead & Profit	@	15%		28,033,084	% of Facility Direct Costs
	Estimate Contingency	@	30%		56,066,168	% of Facility Direct Costs
Subtotal with Contractor Markups and Contingency					**$285,490,008**	
	Escalation to Midpoint of Construction	@	38%		108,486,203	assume 2% percent over 19; contrustion start = 2033 end = 2037
	Project Capital Cost Total				**$393,976,211**	

Annual Operations and Maintenance	Qty	Units	Total Annual Costs $/Unit	Total	
Energy Costs (non-treatment)					Pump Operation = 5150 hours operated per year
Energy - Suction PS at Rio Vista	125,478	KWh	0.12	15,057	Pump Station Hp = 33 Total Motor HP Required
Energy (other)	6,000	KWh	0.12	720	5% of sum of pumping energy requirements
Labor Costs (non-treatment)					
Other Labor (pipeline, pump station, storage tank, monitoring)	0.5	staff	100,000	50,000	full time staff at $100,000 salary per year
Maintenance: Pipeline, Pump Station, Tank	@	1.0%		252,603	% of above direct facility costs for pipelines, injection and monitoring wells
Contingency	@	10%		31,838	% of above O&M costs
Recycled Water Purchase (tertiary)	5,500	AF	200	1,100,000	Based on avereage LACSD RW purchase rate from 2013 to 2015
Advanced Treatment Costs					Source: Trussell Technologies, including energy, labor, chemicals, materials and replacement costs by process type
Ozone System	4,900,000	gal	0.04	199,564	
Biologically Active Carbon Filter	4,900,000	gal	0.01	39,913	unit cost based on average operating flow over the year
Microfiltration	4,900,000	gal	0.22	1,097,600	
Enhanced Brine Concentration (NF + IX)	4,900,000	gal	0.45	2,195,200	
Reverse Osmosis	4,900,000	gal	0.43	2,095,418	
UV AOP	4,900,000	gal	0.04	199,564	
Other Appurtenances	4,900,000	gal	0.12	598,691	
Annual O&M Costs ($/year)				**$7,876,167**	
Annual Unit O&M Costs ($/AFY)				**$1,432**	

CASTAIC LAKE WATER AGENCY

NCWD

SCWD

VWC
Valencia Water Company

LACWD

SANITATION DISTRICTS OF LOS ANGELES COUNTY